JUDD HULL and ASSOCIATES

Geotechnical Consultants

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582-1880 AREA CODE 415 22654 WATKINS STREET HAYWARD, CALIFORNIA 94541



By W. H. SLAUTTERBACK

Project No. 1189-A28-D4 15 November 1983

Bebco Paving Company 4901 Tidewater Avenue Oakland, California 94601

Subject: Backfill of Tank Excavations

2492 Castro Valley Boulevard Alameda County, California

Gentlemen:

This letter summarizes results of testing services performed by our firm during backfill placement for an 8-foot excavation of three existing tanks.

Our services were requested by Mr. S. Butler of your firm to verify the 90% compaction requirement of the County of Alameda. Personnel from our firm were present on a routine intermittent basis during fill placement and performed compaction tests when appropriate. The observation was made of the removal of the three tanks. A summary containing results of tests performed by our firm is attached in Tables I and II. The test results indicate that the fill was placed and compacted in substantial conformance with the County's requirements.

Some placement and compaction of the fill was completed at the time our personnel visited the site to perform testing services. We did not observe the excavation prior to placement of fill nor did we observe placement or compaction of the material being tested. As a result we do not have knowledge, nor can we express an opinion, concerning the uniformity of the fill or the conditions in which it was placed. Our opinion as the compaction of the fill is based solely on the test results which may or may not be representative of the entire fill. Our opinion is not intended as a warranty or certification of the work performed by others. Responsibility to perform the work in accordance with the plans and specifications rests with the contractor(s).

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COUNTY OF ALAMEDA BUILDING INSPECTION DEPARTMENT

Bebco Paving Co. Project No. 1189-A28-D4 -2-15 November 1983 If you have any questions about the work performed by our firm, or need additional information, please contact this office. Yours very truly, JUDD HULL & ASSOCIATES Dante P. Moresi Staff Technician DPM: ph Copies: 2 to Client

TABLE I SUMMARY OF LABORATORY TEST RESULTS

SOIL TYPE	DESCRIPTION OF SOIL	.8	MAXIMUM DRY DENSITY P.C.F.	OPTIMUM MOISTURE CONTENT %
1	GRAY SAND W/BLACK CLAY W/ROCK		122.5	10.2
2	*TAN FINE SAND W/AB (IMPORT)		121.8	8.6
3	TAN-BROWN GRAVELLY SANDY SILT		127.6	10.4
4	RED-BROWN SAN LEANDRO 3/4" AB		126.7	9.7

Another Import, a very moist brown-black silty clay, was substituted for Sample 2. This was then removed and off-hauled because it was too wet to achieve 90% compaction.

TABLE II
SUMMARY OF FIELD DENSITY TEST RESULTS
NUCLEAR DENSITY METHOD

* Denotes Failing Test (RT Denotes Retest)

TEST NO.	DATE 1983	LOCATION	ELEV/	MOISTURE CONTENT % DRY WEIGHT	SOIL DENSITY P.C.F.	RELATIVE COMPACTION %	SOIL TYPE & REMARKS
1401 1101	10/31	NE Corner	4' BRG	15.2	115.3	94	1
7	10/31	NE Corner	6' BRG	, 15.9	110.9	91	1
2	11	SW Corner	3' BRG	16.0	113.8	93	1
3			2½' BRG	11.0	124.6	98	3
4	11/4	SE Corner N Side, Mid	2' BRG	12.0	119.3	93	3
5	tt		RG	10.7	116.4	92	1
7		NW Corner	.5' BRG	12.9	118.2	93	4

GENERAL NOTES

SAMPLE IDENTIFICATION



All sample classifications reviewed by Geotechnical Engineer in accordance with Unified Soil Classification System (ASTM D-2487)

DESCRIPTIVE TERM (% BY DRY WEIGHT)		PARTICLE SIZE (DIAMETER)		
Trace:	1-10%	Boulders: 8 in and larger		
Little:	11-20%	Cobbles: 3 in to 8 in		
Some:	21-35%	Gravel: coarse- 3/4 to 3 in		
And/Adjactive	36-50%	fine- No. 4 (4.76mm) to 3/4 in		
		Sand: coarse- No. 4 (4.76mm) to No. 10 (2.0mm)		
		medium- No. 10 (2.0mm) to No. 40 (0.42mm)		
		fine- No. 40 (0.42mm) to No. 200 (0.074mm)		
		Silt: No. 200 (0.074mm) and smaller (Non-plastic)		
		Clay: No. 200 (0.074mm) and smaller (Plastic)		

SOIL PROPERTY SYMBOLS

DRILLING AND SAMPLING SYMBOLS

Dd: Dry Density, pcf SS: Split-Spoon

LL: Liquid Limit ST: Shelby Tube - 3" O.D. (except where noted)

PL: Plastic Limit

SL: Shrinkage Limit

LI: Liquidity Index{(w - PL)/PI}

Pl: Plasticity Index (LL-PL)

AU: Auger Sample

DB: Diamond Bit

CB: Carbide Bit

WS: Wash Sample

Gs: Specific Gravity

RB: Rock-Roller Bit

K: Coefficient of Permeability

BS: Bag Sample

w: Moisture Content

qp: Calibrated Penetrometer Resistance, tsf

qs: Vane-Shear Strength, tsf

qu: Unconfined Compressive Strength, tsf

N: Penetration Resistance per foot or fraction thereof for standard 2 inch 0.D., 1 3/8 inch I.D., split spoon sampler driven with a 140 pound weight free-falling 30 inches, in accordance with Standard Penetration Test Specifications (ASTM D-1586)

Nc: Penetration Resistance per foot or fraction thereof for standard Cone Penetrometer driven with a 140 pound weight free-falling 30 inches

 $\underline{\underline{\mathbf{y}}}$: Apparent groundwater level at the time noted after completion

: Depth to which boring caved during water level readings

SOIL STRENGTH CHARACTERISTICS

NON-COHESIVE (GRANULAR) SOILS COHESIVE (CLAYEY) SOILS UNCONFINED COMPARATIVE BLOWS PER BLOWS PER COMPRESSIVE RELATIVE FOOT (N) CONSISTENCY STRENGTH (TSF) DENSITY FOOT (N) Very Soft 0-2 0 - 0.25Very Loose 0-4 Soft 3-4 0.25 - 0.50Loose 5-10 5-8 Medium Stiff 0.50 - 1.00Firm 11-30 Stiff 9-15 1.00 - 2.00 Dense 31~50 2.00 - 4.00 16-30 Very Stiff Very Dense 51+ Hard 31+ 4.00+ DEGREE OF DEGREE OF

PLASTICITY PΙ EXPANSIVE POTENTIAL None to Slight 0 - 40-15 Low Slight 5-10 Medium 15-25 Medium 11-30 High 25+ High to Very High 31+

GEA446/kah