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	Alame	da, Califorr	nia 94502-6577	00			Alameda County Environmental Health				
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COMME If you hav (510) 420-3	e any qi	ıestions reg	garding the cont	ents of t	his docu	ment, p	lease call Peter Schaefer at				
Copy to:	y) 1627 Vineyard Avenue, Pleasanton, CA										
		Danielle Ste CA 9456		e-Pleasaı	nton Fire	e Depart	tment, 3560 Nevada Street, Pleasanton,				
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Filing: Correspondence File



Denis L. Brown Shell Oil Products US

HSE – Environmental Services 20945 S. Wilmington Ave. Carson, CA 908101039 Tel (707) 865 0251 Fax (707) 865 2542 Email denis.1.brown@shell.com

Jerry Wickham Alameda County Environmental Health 1131 Harbor Bay Parkway, Suite 250 Alameda, California 94502-6577

Re:

Shell-branded Service Station

4212 First Street
Pleasanton, California
SAP Code 135782
Incident No. 98995840
ACEH Case No. RO0000360

Dear Mr. Wickham:

The attached document is provided for your review and comment. Upon information and belief, I declare, under penalty of perjury, that the information contained in the attached document is true and correct.

If you have any questions or concerns, please call me at (707) 865-0251.

Sincerely,

Denis L. Brown

Senior Program Manager



DUAL-PHASE EXTRACTION PILOT TEST WORK PLAN

SHELL-BRANDED SERVICE STATION 4212 FIRST STREET PLEASANTON, CALIFORNIA

SAP CODE

135782

INCIDENT NO.

98995840

AGENCY NO.

RO0000360

APRIL 12, 2012 REF. NO. 240523 (11) This report is printed on recycled paper. Prepared by: Conestoga-Rovers & Associates

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- NOVEMBER 4, 2011

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1.0 INTRODUCTION

Conestoga-Rovers & Associates (CRA) on behalf of Equilon Enterprises LLC dba Shell Oil Products US (Shell) prepared this *Dual-Phase Extraction Pilot Test Work Plan*. CRA's October 24, 2011 *Corrective Action Plan* recommended monitored natural attenuation as the final remediation option for the site; however, Alameda County Environmental Health's (ACEH's) January 5, 2012 letter rejected that proposal and requested a work plan for dual-phase extraction (DPE) and air sparging (AS) pilot tests and for a soil vapor investigation. Per CRA's March 14, 2012 electronic correspondence with ACEH, the due date for submitting this work plan was extended to April 13, 2011. Per CRA's April 11, 2012 telephone conversation with ACEH, we will submit work plans for an AS pilot test and a soil vapor investigation under separate cover.

CRA's January 31, 2011 Data Review and Subsurface Investigation Work Plan included an evaluation of data from Delta Consultants' (Delta's) 2010 AS pilot test. CRA noted that the data was at best inconclusive, and likely demonstrated that AS is infeasible due to the following: first, a lack of groundwater mounding occurred during the test; and second, air sparge flow rates acceptable for successful AS only occurred at flow rates near the maximum acceptable applied pressure. CRA notes that further AS pilot testing would not likely be successful due to the lithology at this site, and still supports the conclusions from this report. As such, no AS pilot test work plan is being submitted with this work plan.

This Shell-branded service station is located on the southeastern corner of the First Street and Vineyard Avenue intersection in a mixed residential and commercial area of Pleasanton (Figures 1 and 2). The site layout includes three current fuel underground storage tanks (USTs), a former fuel UST complex, two fuel dispenser islands, a former waste oil UST, and a station building.

A summary of previous work performed at the site and additional background information is presented in Appendix A.

2.0 SITE BACKGROUND

2.1 <u>SITE GEOLOGY AND HYDROGEOLOGY</u>

2.1.1 REGIONAL GEOLOGY AND HYDROGEOLOGY

According to the *Evaluation of Ground Water Resources: Livermore and Sunol Valleys* (California Department of Water Resources [DWR] Bulletin No. 118-2, June 1974), the site is located in the Bernal sub-basin of the Livermore Valley groundwater basin. Streams draining Livermore Valley merge in the Bernal sub-basin and then leave the valley as Arroyo de la Laguna.

Sediments below the site are inter-layered clays, silts, and sands with occasional gravel lenses to the total depth explored of approximately 108 feet below grade (fbg). DWR Bulletin No. 118-2 indicates that surface soils extend to 110 fbg in the area of the site and that water-bearing materials below these shallow sediments are comprised of the valley-fill materials. These materials are present as a sequence of sandy gravel and sandy clayey gravel aquifers up to 100 feet in thickness. The aquifers are separated by silty clay confining beds up to 30 feet in thickness.

2.1.2 SOIL TYPES

Available exploratory boring logs are included in Appendix B. Well and historical soil boring and soil sample locations are shown on Figures 2 and 3.

Shallow soils consist of interbedded layers of silts and clays from the surface to approximately 15 fbg, underlain by inter-layered sands, silts, and clays to approximately 30 fbg. Shallow groundwater occurs just below this sandy layer within a relatively low permeability zone at a depth of approximately 31 fbg. Deeper soils consist of silts and clays with sand and gravel inter-beds to approximately 95 fbg underlain by sands to the total depth explored of 108 fbg.

2.1.3 GROUNDWATER DEPTH AND FLOW DIRECTION

The depth to first-encountered groundwater typically ranges between 31 to 34 fbg. A groundwater contour map for the fourth quarter 2011 groundwater monitoring event is included as Figure 4. As illustrated on Figure 4, the groundwater flow direction for the

site is predominantly toward the north-northeast, which is consistent with historical groundwater flow direction.

After further data review, it appears that there may be a confining layer situated between 15 and 20 fbg near the former UST complex, which may have trapped infiltrated surfaced water in the former UST complex and adjoining soil horizon.

2.1.4 GROUNDWATER QUALITY ASSESSMENT

The California State Water Resources Control Board's Geotracker website file for the environmental case at this site states that the groundwater at this site is considered a "drinking water supply"; however, neighboring properties receive potable water from the City of Pleasanton.

2.2 PREFERENTIAL PATHWAY ANALYSIS

Based on the depth to water which typically ranges between 31 to 34 fbg, on- and off-site utilities and infrastructure are not likely to be preferential pathways for groundwater flow.

2.3 SENSITIVE RECEPTORS

2.3.1 WELL SURVEY

In May 2004, Toxichem Management Systems, Inc. (Toxichem) conducted a well survey which identified a municipal well (3S/1E-21B1) and a well of unknown use (3S/1E-21B) located approximately 900 feet northeast of the site and another municipal well (3S/1E-16P1) located approximately 1,200 feet north of the site. The locations of the wells could not be field verified.

In September 2005, Delta conducted a well survey which located an old water tower in the area of the wells identified in Toxichem's 2004 well survey and identified a water supply well (3S/1E-21C1) and an irrigation well (3S/1E-21C4) located approximately 1,000 feet northwest of the site, and another irrigation well in Kottinger Park located approximately 800 feet east of the site.

Given the relative distance from the site to the municipal wells, it is unlikely that petroleum hydrocarbons originating from the site will reach these wells.

2.3.2 SURFACE WATER

Arroyo del Valle, a creek located approximately 1,130 feet north of the site, is the closest potential surface water receptor to the site. Surface water in Arroyo del Valle flows to the west at a depth of approximately 20 feet below the surrounding grade. Based on the typical depth to water which ranges between 31 to 34 fbg and the distance to Arroyo del Valle, it is unlikely that the creek will be impacted from petroleum-based hydrocarbons and oxygenates that may have been released from the site.

3.0 DPE PILOT TEST WORK PLAN

CRA recommends completing a DPE pilot test to confirm the validity of DPE as a viable As noted in the January 31, 2011 Data Review and Subsurface remedial option. Investigation Work Plan, DPE utilizing wells MW-1 and MW-2 produced maximum concentrations of total petroleum hydrocarbons as gasoline (TPHg) of 70 and 120 parts per million by volume (ppmv), respectively. DPE pilot testing utilizing well MW-4 produced a maximum concentration for TPHg of 4,900 ppmv. According to Delta's pilot test data tables, the maximum extracted vapor concentrations occurred after approximately 20 hours of operation. This indicates that the main source of residual TPHg is within the radius of influence, but at the outer edges of this zone. The purpose of this second DPE pilot test is to determine if adequate dewatering can be achieved, the degree of dewatering possible, the vacuum radius of influence, air/water yields necessary to achieve drawdown, and volatile organic compound mass removal rates near the former tank pit in the vicinity of former well S-1/WA-1. In summary, previous DPE pilot testing performed by Delta indicated that DPE can be a viable and effective measure for addressing residual hydrocarbon impacts to soils and groundwater in the shallow soil horizon in the vicinity of the former UST complex. This second pilot test will better define the shallow soil horizon where removable residual hydrocarbon mass is present.

The pilot test target areas are in the vicinity of former monitoring well S-1/WA-1, well MW-4, and boring SB-5. One new 4-inch diameter extraction well, EW-1, will be installed near former well S-1/WA-1 and will be used to evaluate DPE effectiveness in a shallow confined layer. Proposed piezometers P-1 and P-2 will be used for observation wells during pilot testing, as well as existing well SVE-3. Proposed 4-inch diameter

extraction well EW-2 will be installed between well MW-4 and boring SB-5 at a deeper depth. Proposed piezometers P-2 and P-3, as well as wells SVE-4 and MW-4, will be used as observation wells for DPE pilot testing. MW-4 is a 4-inch diameter well screened from 35 to 47 fbg, while SVE-3 and SVE-4 are screened from 20 to 30 fbg. The proposed well layouts can be seen in Figure 2.

When EW-1 is installed and constructed, if no groundwater is encountered in the hypothetical confined perched water zone, then EW-1 will be utilized solely as a soil vapor extraction (SVE) well completed in the shallow soil zone near the remaining residual hydrocarbon mass. All the variables will be the same, except CRA will not include any groundwater extraction monitoring equipment for EW-1. EW-1 is being screened where suspected impacts may remain at the bottom of the former UST complex (approximately 14 fbg). Groundwater was encountered at approximately 14 fbg during the 2002 UST replacement and over-excavation work. EW-2, P-2, and P-3 will be screened in the deeper aquifer zone, and will be constructed for DPE pilot testing and potential remedial use in a new system, as warranted.

3.1 DPE PILOT TEST OBJECTIVES

This DPE pilot test is being proposed to address some of the inconsistencies in the data collected by Delta during their DPE pilot test (in particular to the length of time it took to reach maximum extracted mass concentration and to evaluate the two zones of suspected residual hydrocarbon mass in relation to the northerly corner of the former UST complex). Other objectives for the DPE pilot test include determining:

- The rates and sustenance of groundwater and vapor recovery and associated hydrocarbon mass recovery rates;
- The magnitude of fuel-based hydrocarbon and oxygenate concentrations in the recovered groundwater and vapor streams;
- The optimal vacuum and air flow from the wells to limit the occurrence of short circuiting;
- The radius of influence for both vapor and groundwater extraction from a designated well;
- If short-circuiting will occur in the utilization of the extraction wells in proximity to the more porous backfill of the former UST complex; and
- If DPE is a practical, viable, and cost-effective remedial measure to meet the remediation objectives at the site.

3.2 WELL INSTALLATION

The following presents the details and activities for installing the proposed extraction well for the DPE pilot test.

3.2.1 PERMITS

CRA will obtain the necessary well installation permit from Alameda County Environmental Health (ACEH).

3.2.2 HEALTH AND SAFETY PLAN (HASP)

CRA will update the existing HASP to protect site workers. Job safety analyses (JSAs) will also be prepared in relation to the work associated with the well installations. The HASP and JSAs will be kept on site during field activities and will be reviewed and signed by each site worker prior to the commencement of work.

3.2.3 <u>UTILITY CLEARANCE</u>

CRA will mark the proposed drilling locations, and the locations will be cleared through Underground Service Alert and a private line locator service prior to drilling.

3.2.4 EXTRACTION WELL INSTALLATION

One new 4-inch diameter extraction well, EW-1, will be installed near former well S-1/WA-1. Proposed piezometers P-1 and P-2 will be installed approximately 10 feet to the southwest and northeast, respectively. Proposed 4-inch-diameter extraction well EW-2 will be installed between well MW-4 and boring SB-5 at a deeper depth. Proposed piezometer P-3 will be installed approximately 10 feet to the east of proposed well EW-3. The exact extraction well locations will be determined in the field based upon site conditions. The proposed well locations are shown in Figure 2.

Reportedly, the former UST tank complex excavation pit was excavated to approximately 14 fbg and backfilled with pea gravel. There is a potential risk of DPE

short-circuiting through the former UST complex backfilled excavation. Well EW-1 will be completed to a depth of approximately 24 fbg, with a screen interval extending from approximately 10 to 20 fbg using 0.020-inch Circumslot® screen. The exact screened interval will be determined in the field and will focus on the potentially confined layer between 10 and 20 fbg. As stated above, if no groundwater is encountered, this well will be developed as an SVE well and no groundwater extraction will be needed during its pilot test. A 2-foot sump will be installed as the bottom of the extraction well. An appropriate sand pack will be installed from approximately 0.5 feet above the perforated well casing interval to the total depth of the boring. Approximately 2 feet of bentonite will be installed above the sand pack (9.5 to 11.5 fbg), and a neat cement surface seal will be installed to near the ground surface.

Upon completion of DPE pilot testing, well EW-1 will either be properly destroyed or used for future DPE/SVE operation. P-1 will be installed approximately 10 feet away at a similar depth but as a 2-inch-diameter piezometer in order to monitor radius of influence effects of both groundwater and soil vapors during pilot testing activities. P-2 will be a 2-inch-diameter piezometer and will be placed approximately 10 feet away. This well will be screened from 10 to 40 fbg and will be used for in pilot test activities in both EW-1 and EW-2.

Well EW-2 will be completed to a depth of approximately 42 fbg, with a screen interval extending from 30 to 40 fbg, using 0.020-inch Circumslot® screen. EW-2 will be installed, completed, and destroyed after the pilot test similarly to EW-1, the sand pack being installed approximately 28 to 40 fbg. P-3 will be a 2-inch-diameter piezometer, similar to P-1, but installed to a depth of 42 fbg with a screen interval from 30 to 40 fbg. The installed piezometer will be used as observation wells during pilot testing of well EW-2.

A CRA geologist will supervise the drilling and describe encountered soils using the Unified Soil Classification System and Munsell Soil Color Charts. Soil samples will be collected continuously in EW-1 and at 5-foot intervals in the other well borings. Soil samples will be screened in the field for organic vapors using a photo-ionization detector (PID). CRA will prepare a boring log for each well boring, and PID measurements will be recorded on the boring logs.

3.2.5 WELL DEVELOPMENT AND SAMPLING

Blaine Tech Services, Inc. (Blaine) of San Jose, California will develop the new well at least 72 hours after installation is completed. At least 72 hours after well development,

Blaine will sample the site's groundwater monitoring wells according to the existing sampling schedule and chemical analysis protocol.

3.2.6 CHEMICAL ANALYSES

Soil and groundwater samples will be analyzed by a State-certified laboratory for TPHg, benzene, toluene, ethylbenzene and total xylenes (BTEX), and methyl tertiary-butyl ether (MTBE) by EPA Method 8260B.

3.2.7 WELLHEAD SURVEY

Following well installation, a licensed land surveyor will survey the installed wellhead Northing and Easting coordinates (or latitude and longitude) to NAD83, and wellhead vertical elevation referenced to mean sea level in compliance with AB2886 (Geotracker) requirements.

3.2.8 REPORT PREPARATION

Following the receipt of analytical results, CRA will prepare a written report which will include field procedures, laboratory results, and boring logs.

3.3 <u>DPE PILOT TEST</u>

3.3.1 PILOT TEST WELLS

CRA will utilize installed extraction wells EW-1 and EW-2 for DPE. Piezometers P-1, P-2, and P-3, and existing monitoring wells MW-4, SVE-3, and SVE-4 will be utilized as observation wells for the DPE pilot test. The extraction and observation well diameters, screen intervals, and approximate estimated distance from the planned locations of wells EW-1 and EW-2 are summarized in the table below.

Well ID	Well Diameter (inches)	Screen Interval (fbg)	Estimated Distance from EW-1 (feet)	Estimated Distance from EW-2 (feet)
P-1	2	10-20	12	36
P-2	2	10-40	15	12
P-3	2	30-40	36	10
SVE-3	4	20-30	14	16
SVE-4	4	20-30	32	14
MW-4	4	37-47	16	12

3.3.2 PILOT TEST EQUIPMENT

Critical components for conducting a DPE pilot test include an extraction device, water storage, and a vapor treatment device. A mobile unit (Solleco® 300TCat or equivalent) will be used for testing. It will be equipped with a vacuum pump as the extraction device and a thermal/catalytic oxidizer to treat the extracted soil vapor. Propane will be used as supplemental fuel for the operation of the TCat. The TCat is equipped with a vapor-liquid separator to remove condensation from the vapor stream. Accumulated condensation will be pumped from the separator to an on-site temporary storage tank through an aboveground hose. A down-well submersible pump will be used to extract groundwater from the extraction wells. Extracted groundwater will be pumped directly into the storage tank. The extracted groundwater will be temporarily stored in the storage tank and subsequently transported off site for proper disposal. The anticipated equipment layout is shown in Figure 3.

The TCat is equipped with controls to manage well flow, dilution air flow, pump vacuum, and well vacuum data. A Thomas Industries model 907CDC18F vacuum pump or similar will be used to collect the vapor samples. A Horiba organic vapor analyzer or similar will be used to field measure hydrocarbon concentrations in the extracted vapor stream. A TSI model 8330 air velocity meter or similar will be used to measure vapor extraction velocity rates and temperatures. Magnehelic differential pressure gauges will be used to measure induced vacuum in adjacent wells. A water level meter and/or down well pressure transducers will be used to measure groundwater drawdown in extraction and observation wells. A Kent C700 flow totalizing meter or similar will continuously measure extracted groundwater volume.

As noted above, should groundwater not be encountered in well EW-1, an SVE pilot test only will be conducted on this extraction well. All measurements and equipment will be the same as in DPE testing with the exemption of any equipment or measurements related to groundwater extraction.

9

3.3.3 <u>AIR DISCHARGE PERMIT</u>

CRA will contact the Bay Area Air Quality Management District (BAAQMD) regarding the proposed DPE test and will provide them with the required notification. The TCat will abate the extracted soil vapors to comply with the BAAQMD requirements. BAAQMD regulations limit pilot tests to 5 days in duration. A longer duration pilot test would require a site-specific BAAQMD permit-to-operate. CRA anticipates this pilot test to last 5 days or less.

3.3.4 SITE HEALTH AND SAFETY PLAN

Pursuant to Shell and CRA requirements and prior to commencement of field activities, CRA will again revise and update the site's HASP and JSAs to identity potential hazards and to protect site workers during the pilot test activities. The HASP will be kept on site during field activities and will be reviewed and signed by each site worker. The HASP and all JSAs will be reviewed and signed by each site worker prior to commencement of the applicable work associated with the JSAs.

3.3.5 <u>DATA COLLECTION</u>

Prior to pilot test activities, CRA will measure and record the water level in all extraction and observation wells. During the test, CRA will periodically measure and record the following DPE operational and monitoring information: applied vacuum to the extraction well, induced vacuum at observation wells, extraction well soil-vapor flow, dilution air flow, extracted hydrocarbon-vapor concentrations, extracted groundwater volume and olfactory and visual observations, and groundwater levels to calculate induced drawdown. This information will be collected initially every 30 minutes, then in longer intervals after operational data has stabilized.

Vapor samples will be collected periodically in 1-liter Tedlar bags to confirm field measured concentrations through laboratory analysis. The vapor samples will be collected from the extracted vapor of each well under extraction near the beginning of extraction, in the approximated middle of the extraction period, and prior to extraction shut-down. Grab groundwater samples will be collected prior to the beginning of tests in EW-1 (if groundwater is present) and EW-2, and one additional time prior to the conclusion of pilot testing, as warranted by system operation.

3.3.6 DPE TEST PROCEDURE

CRA will initiate each test on the well under extraction by commencing operation of the submersible pump in the well to dewater the well casing and associated well pack. Once dewatered, the SVE portion of the test will commence. The SVE system's blower will initially be started utilizing dilution air in the vacuum stream and adjusting the dilution air valve until the optimal vacuum and flow rate is determined to achieve the most effective mass removal rate and to avoid short-circuiting problems.

It is anticipated that a test lasting between 24 to 36 hours will be performed on each of the two extraction wells; the length of test will depend on the field results obtained. The total DPE testing is planned to be commenced and operated in a continuous 24-hour operation mode and to last between 3 to 5 days.

3.3.7 CHEMICAL ANALYSES FOR PILOT TEST SAMPLES

A State-certified analytical laboratory will analyze soil vapor samples for TPHg by EPA Method TO-3 and for BTEX and MTBE by EPA Method 8260B(M). Groundwater samples will be analyzed by a State-certified analytical laboratory for TPHg, BTEX, and MTBE by EPA Method 8260B.

3.2.8 REPORT PREPARATION

Following the receipt of analytical results from the laboratory, CRA will prepare a written report which will include field procedures for pilot testing; a tabulation and summary of field data collected; a summary and tabulation of the laboratory analytical obtained; and a summary of finding of conclusions for the DPE test.

3.3 <u>SCHEDULE</u>

Upon receiving ACEH approval of this work plan and a drilling permit from Zone 7 Water Agency, CRA will proceed with advancing the borings and installing the extraction wells and piezometers. Concurrently, CRA will schedule and prepare for the DPE pilot test. The DPE pilot test will occur after development and sampling of new wells EW-1, EW-2, P-1, P-2, and P-3. Prior to conducting the test, CRA will submit a

notification to the BAAQMD. CRA expects the pilot test to last 4 days, but it may be extended up to 5 days if the testing data and confirmation of feasibility warrants such.

The well installation and pilot test reports will be submitted approximately 60 days after receipt of the respective laboratory analytical data for samples collected during the well installation and the pilot test. Extraction wells EW-1 and EW-2, and piezometers P-1, P-2, and P-3 will be properly destroyed or used for future DPE operation if warranted.

All of which is Respectfully Submitted, CONESTOGA-ROVERS & ASSOCIATES

Peter Schaefer, CEG, CHG

Lee Brennen, P.E



FIGURES

Shell-branded Service Station

1/8

4212 First Street Pleasanton, California

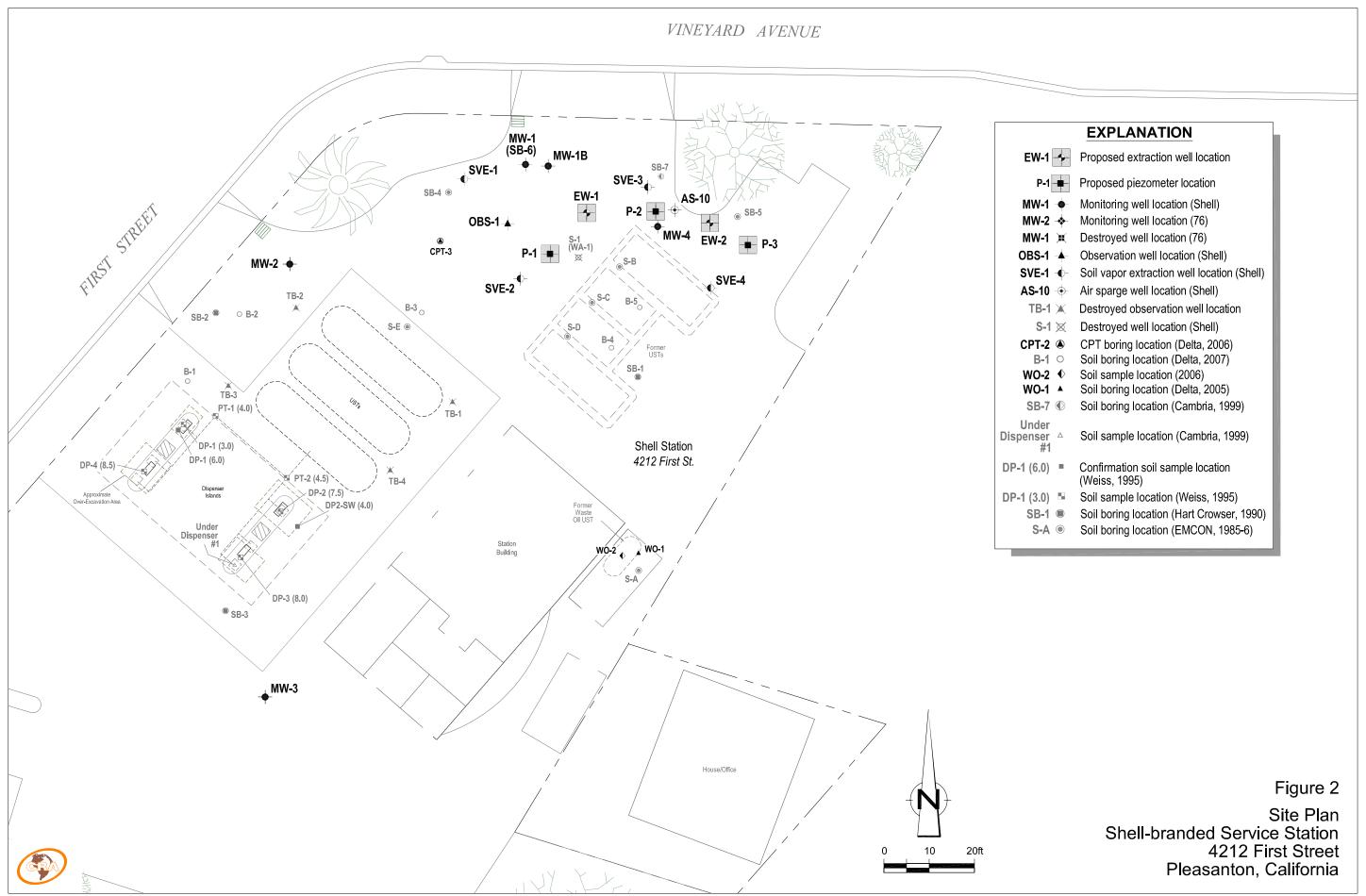
SOURCE: TOPO! MAPS

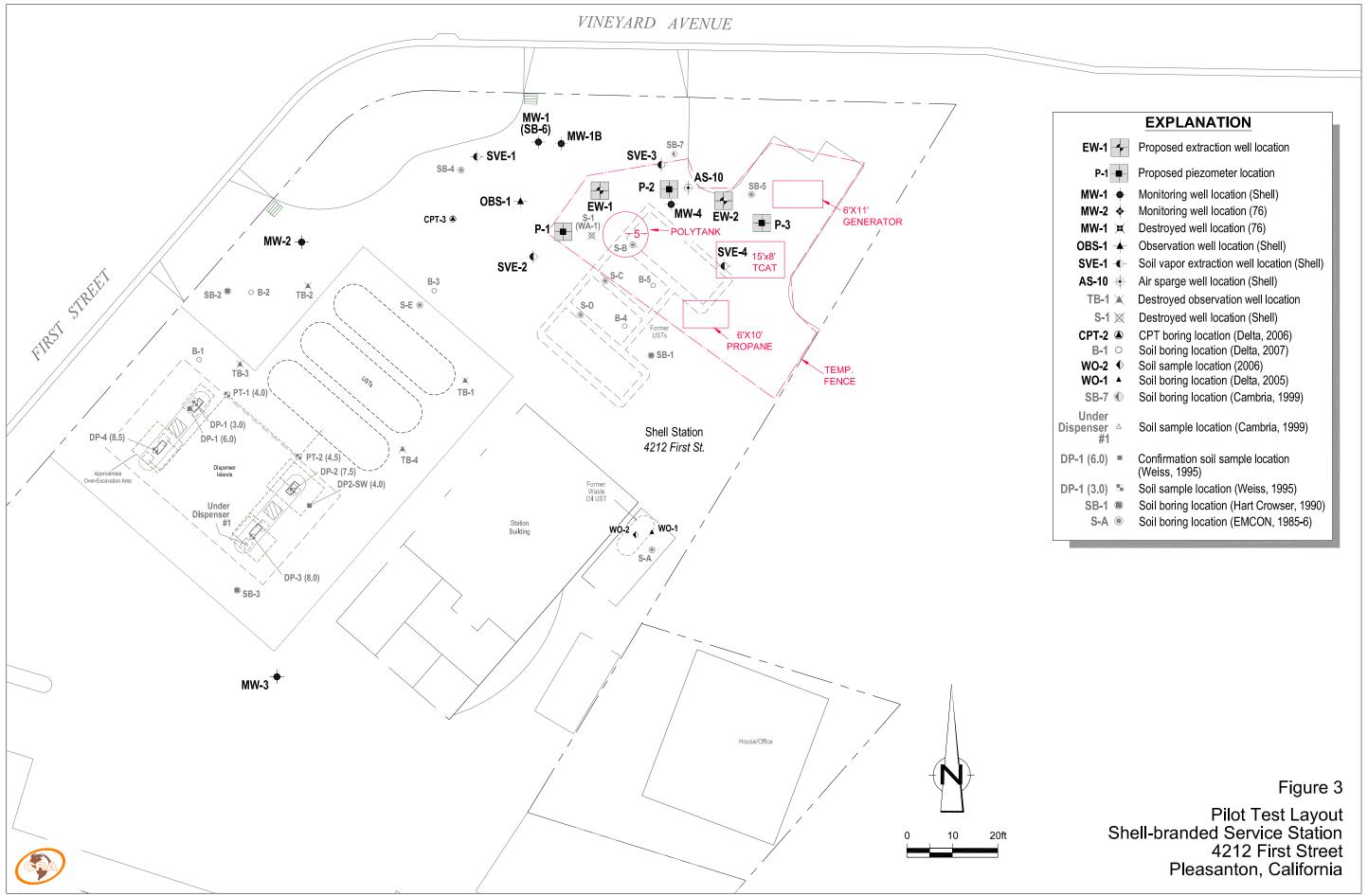


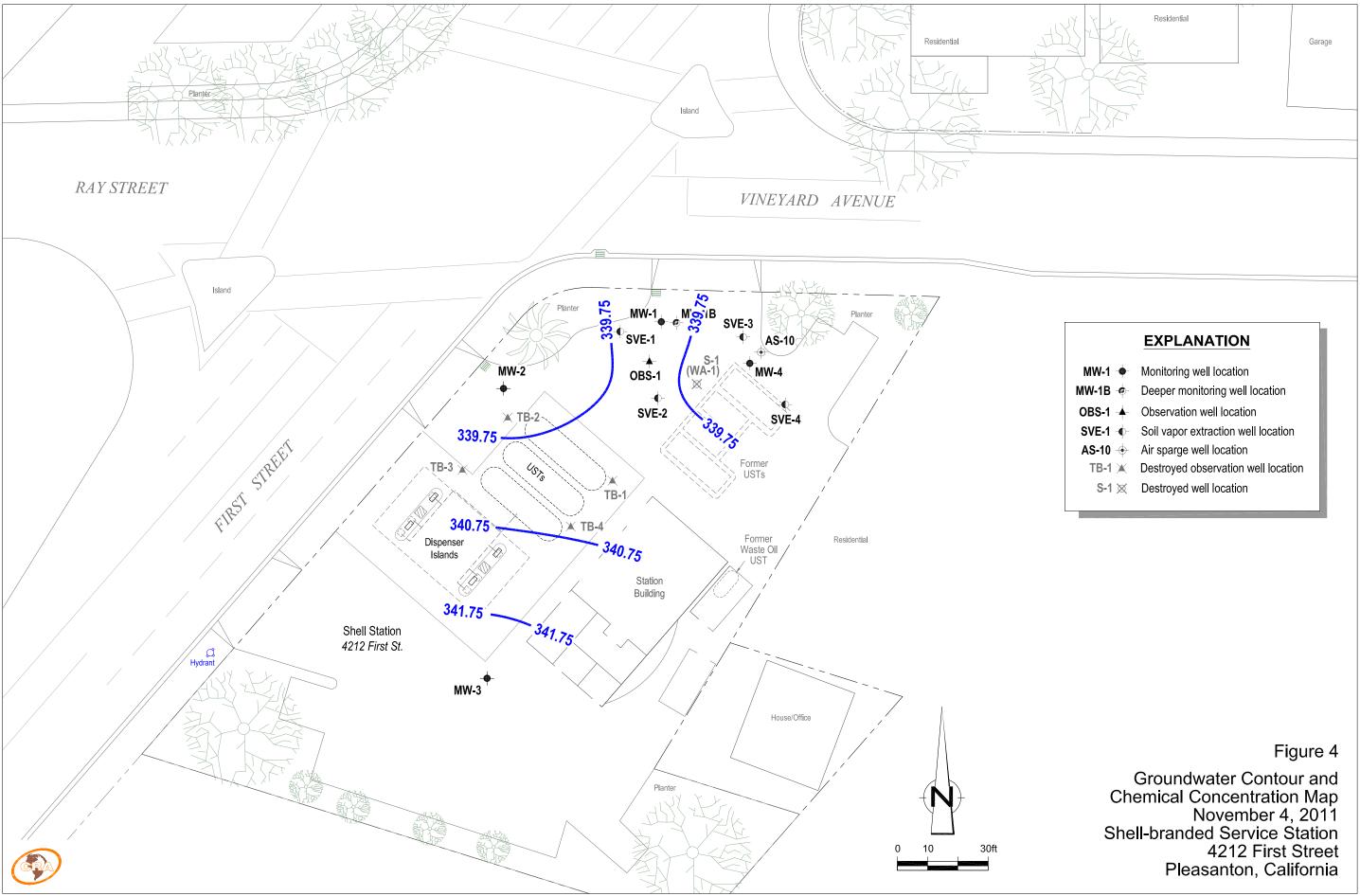
1/2 SCALE : 1" = 1/4 MILE

Vicinity Map

FIGURE







APPENDIX A

SITE HISTORY

SITE HISTORY

1985 Subsurface Investigation: In September 1985, Emcon Associates (Emcon) drilled one soil boring (S-A) adjacent to the waste oil underground storage tank (UST), and drilled three soil borings (S-B through S-D) and installed one groundwater monitoring well (S-1) adjacent to the gasoline USTs. Soil samples contained up to 1,300 milligrams per kilogram (mg/kg) total petroleum hydrocarbons as gasoline (TPHg), 9.6 mg/kg toluene, and 260 mg/kg xylenes and ethylbenzene. Benzene was not detected in the soil samples. The monitoring well was dry. Emcon's November 12, 1985 letter presents investigation details.

1986 Subsurface Investigation: In March 1986, one soil boring (S-E) was drilled adjacent to product lines. No TPHg, benzene, toluene, ethylbenzene, or xylenes (BTEX) were detected in soil samples.

1986 *UST Removal:* In May 1986, Blaine Tech Services (Blaine) collected soil samples following removal of four gasoline USTs and one waste oil UST. Soil samples from the gasoline UST excavation contained up to 240 mg/kg TPHg. Hydrocarbons were not detected in a soil sample collected from the waste oil tank excavation. Three 10,000-gallon, double-walled, fiberglass tanks were installed at a location closer to the dispenser islands.

1988 Gasoline Spill: In August 1988, approximately 40 gallons of gasoline were spilled in the area of the pump islands. Impacted soil was removed to a depth of 1 to 2 feet below grade (fbg).

1990 Subsurface Investigations: In March 1990, Hart Crowser, Inc. (Hart) drilled three soil borings (SB-1 through SB-3) and destroyed one groundwater monitoring well (S-1). Following the well destruction, Hart continued drilling a boring (WA-1) below the depth of the monitoring well. Soil samples contained up to 380 mg/kg TPHg, 2.2 mg/kg benzene, 2.7 mg/kg toluene, 5.3 mg/kg ethylbenzene, and 32 mg/kg xylenes. Hart's April 23, 1990 Report of Supplemental Site Assessment provides details of this investigation.

In July 1990, Hart drilled two additional soil borings (SB-4 and SB-5) down gradient from the former UST complex. Soil samples contained up to 820 mg/kg TPHg, 65 mg/kg benzene, 3.7 mg/kg toluene, 6.5 mg/kg ethylbenzene, and 65 mg/kg xylenes (SB-5 at 35 fbg). Hart's December 11, 1990 *Supplemental Site Assessment* presents the soil boring investigation details.

1995 Dispenser and Piping Replacement: In September 1995, Paradiso Mechanical of San Leandro, California removed the product lines and replaced the dispensers and piping. Weiss Associates (Weiss) collected soil samples from beneath the gasoline product piping (PT-1 and PT-2) and dispensers (DP-1 through DP-4). Soil samples contained up to 120 mg/kg TPHg, 0.038 mg/kg ethylbenzene, and 0.19 mg/kg xylenes. Benzene and toluene were not detected in the soil samples. Approximately 40 cubic yards of soil were over-excavated at the direction of the Pleasanton Fire Department. Weiss' December 21, 1995 Dispenser Replacement Sampling report presents soil sampling locations and results.

1998 Facility Upgrade: In July 1998, Cambria Environmental Technology, Inc. (Cambria) inspected the waste oil tank remote-fill piping during its removal by Gettler-Ryan of Dublin, California. No hydrocarbon impact was observed during the site visit, and, therefore, no sampling was required. A pea gravel sample contained 27 mg/kg total petroleum hydrocarbons as diesel (TPHd). Cambria's September 2, 1998 1998 Upgrade Site Inspection Report provides inspection details.

1999 Subsurface Investigation: In April 1999, Cambria drilled two soil borings (SB-6 and SB-7) and converted SB-6 to monitoring well MW-1. Soil samples contained up to 83 mg/kg TPHg, 0.10 mg/kg benzene, 0.37 mg/kg toluene, 0.26 mg/kg ethylbenzene, and 0.26 mg/kg xylenes. Methyl tertiary-butyl ether (MTBE) was not detected in soil samples. Grab groundwater samples contained up to 10,000 micrograms per liter (μ g/l) TPHg, 4,500 μ g/l benzene, 3.4 μ g/l ethylbenzene, and 2.9 μ g/l xylenes. Toluene and MTBE were not detected in the grab groundwater samples. Cambria's August 12, 1999 Subsurface Investigation Report presents investigation details.

2000 Subsurface Investigation: In January 2000, Cambria installed two wells (MW-2 and MW-3) to determine whether groundwater had been impacted by petroleum hydrocarbons. No petroleum hydrocarbons or MTBE were detected in soil samples. Cambria's June 23, 2000 Subsurface Investigation Report presents well installation details.

2004 *Well Survey:* In May 2004, Toxichem Management Systems, Inc. (Toxichem) conducted a well survey, which identified a municipal well (3S/1E-21B1) and a well of unknown use (3S/1E-21B) approximately 900 feet northeast of the site and another municipal well (3S/1E-16P1) approximately 1,200 feet north of the site. The locations of the wells could not be field verified.

2005 *UST Upgrades:* In January 2005, Town and Country Contractors, Inc. (T & C) upgraded the gasoline USTs.

2005 *Tank Backfill Well Destructions:* In January 2005, T & C destroyed four tank backfill wells (TB-1 through TB-4).

2005 Waste Oil UST Investigation: In January 2005, an unknown liquid was likely poured into a port on the waste oil UST which led directly into the pea gravel surrounding the UST. Based on this observation, Shell submitted a UST Unauthorized Release (Leak)/Site Contamination Report on January 19, 2005. Able Maintenance (Able) and Service Station Systems sealed the UST port with epoxy and excavated pea gravel around the UST. Toxichem collected pea gravel samples which contained 1.4 mg/kg TPHg, 1,400 mg/kg TPHd, and 10,000 mg/kg total petroleum hydrocarbons as oil and grease. In June 2005, Delta Consultants (Delta) drilled one soil boring (WO-1) adjacent to the waste oil UST to determine if the liquid poured into the pea gravel had impacted soils. Petroleum hydrocarbons were not detected in the soil samples. Delta's July 11, 2005 Soil and Water Investigation Report provides investigation details.

2005 Receptor Survey: In September 2005, Delta conducted a well survey which located an old water tower in the area of the wells identified in Toxichem's 2004 well survey and identified a water supply well (3S/1E-21C1) and an irrigation well (3S/1E-21C4) approximately 1,000 feet northwest of the site and another irrigation well in Kottinger Park, approximately 800 feet east of the site. Delta identified the nearest surface water as Arroyo del Valle Creek located approximately 1,130 feet northwest of the site.

2006 Waste Oil UST Removal: In July 2006, Wayne Perry Inc. removed a 550-gallon waste oil UST. Cambria collected a soil sample from the bottom of the UST excavation (WO-2) which contained 26 mg/kg oil and grease, 5.5 mg/kg TPHd, 0.021 mg/kg MTBE, 40.7 mg/kg chromium, 6.00 mg/kg lead, 46.9 mg/kg nickel, and 52.5 mg/kg zinc. Based on these concentrations, Shell submitted a UST Unauthorized Release (Leak)/Site Contamination Report on July 28, 2006. Cambria's September 21, 2006 UST Removal Report details the UST removal and sampling.

2006 Subsurface Investigation: In August and September 2006, Delta installed two monitoring wells (MW-1B and MW-4) and drilled two cone penetrometer test (CPT) borings (CPT-2 and CPT-3). Well MW-4 was installed in first-encountered groundwater, and well MW-1B was installed in a deeper water-bearing zone. Soil samples from well boring MW-4 contained up to 380 mg/kg TPHg, 1.2 mg/kg ethylbenzene, 1.6 mg/kg xylenes, and 0.59 mg/kg MTBE. TPHg, BTEX, MTBE, and tertiary-butyl alcohol (TBA) were not detected in soil samples from MW-1B, and benzene, toluene, and TBA were not detected in soil samples from MW-4. Grab groundwater samples from off-site CPT boring CPT-2 contained up to 0.99 μg/l

benzene, $47 \mu g/1 \text{ MTBE}$, and $27 \mu g/1 \text{ TBA}$. Grab groundwater samples from on-site CPT boring CPT-3 contained up to 700 $\mu g/1 \text{ TPHg}$, 0.78 $\mu g/1 \text{ ethylbenzene}$, 2.1 $\mu g/1 \text{ xylenes}$, 79 $\mu g/1 \text{ MTBE}$, and 2,000 $\mu g/1 \text{ TBA}$. Delta's October 31, 2006 *Soil and Groundwater Investigation Report* provides well installation and CPT investigation details.

2007 Subsurface Investigation: In March 2007, Delta drilled five soil borings (B-1 through B-5) in or near on-site source areas. Soil samples from the soil borings contained up to 710 mg/kg TPHg, 2.3 mg/kg ethylbenzene, 16 mg/kg xylenes, 0.78 mg/kg MTBE, and 0.80 mg/kg TBA. Delta's June 25, 2007 Site Investigation and Interim Remediation Report provides details of this investigation.

2007 *Mobile Groundwater Extraction (GWE):* From June through August 2007, Delta extracted approximately 4,226 gallons of groundwater from MW-4. Delta's June 25, 2007 *Site Investigation and Interim Remediation Report* and November 2, 2007 *Draft Corrective Action Plan (CAP)* provide remediation details.

2009 Dual-Phase Extraction (DPE) Pilot Test: In January 2009, Delta conducted a 5-day DPE pilot test on MW-4 and 4-hour DPE pilot tests on MW-1 and MW-2. Prior to conducting the DPE pilot tests, Delta conducted step drawdown tests in MW-1 and MW-4. Delta calculated hydraulic conductivities of 3.59x10-5 centimeters per second (cm/sec) in MW-1 at a pumping rate of 0.48 gallons per minute (gpm) and 3.17x10-5 cm/sec in MW-4 at a pumping rate of 0.40 gpm. Based on the results of the DPE pilot test, Delta calculated a theoretical radius of influence of 26 feet for soil vapor extraction and estimated that 286.3 pounds of hydrocarbons were removed from the vadose zone. An estimated 0.23 pounds of dissolved hydrocarbons were removed along with 2,748 gallons of groundwater. Delta concluded that while GWE results indicated it was likely not a viable remediation strategy, soil vapor extraction (SVE) could be a viable remediation alternative. Delta's February 12, 2009 DPE Pilot Test Report provides pilot test data.

2009 *Dispenser Repairs*: In January 2009, Able replaced the faulty pan beneath the south dispenser on the pump island closest to the station building. Delta collected a soil sample (Under Dispenser #1) from the dispenser excavation. No TPHg, TPHd, BTEX, fuel oxygenates, or lead scavengers were detected in the soil sample. Delta's March 6, 2009 *Dispenser Repair Report* presents details of the repair and soil sampling.

2010 *Subsurface Investigation:* In January 2010, Delta installed one observation well (OBS-1), one air sparging (AS) well (AS-1), and four SVE wells (SVE-1 through SVE-4). Delta's June 7, 2010 2010 AS Pilot Test Report provides well installation details.

2010 AS Pilot Test: In January 2010, Delta conducted an AS pilot test using well AS-10. Delta calculated an air sparging radius of influence of 31 feet; however, CRA's subsequent analysis of the pilot test data determined that the test was flawed and therefore inconclusive. Delta's June 7, 2010 2010 AS Pilot Test Report details pilot testing results.

2011 Subsurface Investigation: In June 2011, CRA attempted to install two off-site wells across Vineyard Avenue from the site. CRA abandoned the well installation attempts because there were no other locations in the sidewalk where the wells could be installed safely due to the interference of underground utilities. CRA's July 28, 2011 letter provides investigation details.

Groundwater Monitoring Program: Groundwater monitoring and sampling began in June 1999. The depth to first-encountered groundwater typically ranges between 31 to 34 fbg. Groundwater flow is generally northwesterly.

APPENDIX B

BORING LOGS

PROJECT NUMBER 738-60.01

BORING NO. S-A

PROJECT NAME Gettler-Ryan, Shell, 4226 First St., Pleasanton

PAGE 1 OF 1

BY MGB

DATE 9/27/85

SURFACE ELEV. 375'±

	MPR	7.	112 5	1/2/	/85		SURFACE ELEV. 375'±
TORVANE (TSF)	POCKET PENETRO- METER (TSF)	PENETRA- TION (Blows/ Ft.)	CROUND WATER LEYELS	DEPTH IN FT.	SAMPLES	LITHO- CRAPHIC COLUMN	DESCRIPTION
	4.4	. 88		5-		ML CL	ASPHALT and SAND - Fill GRAVELLY SILT - Fill; black (5Y, 2.5/2); 20% fine to coarse sand; 10% fine gravel; damp; no product odor. CLAY; light olive brown (2.5Y, 5/6); silty; 10% fine to medium sand; stiff; damp; no product odor. @7': no sand; hard; no product odor. @10': 20% fine gravel; no product odor.
·	1.5	21	- -	15	2		@14': 15-20% fine to medium sand; trace fine gravel; stiff; moist; no product odor.
	5	61	-	20	3	. //	@18½': brownish yellow (10YR, 6/8); silty; hard; moist; no product odor. BOTTOM OF BORING AT 20 FEET.
	٠		-	25-			
·				35- - - 4 0 -			

REMARKS Drilled by 5-inch continuous flight, auger; samples collected with 2-inch California modified split-spoon sampler; borehole backfilled with soil cuttings to 1/2 foot; concrete to surface.



PROJECT NUMBER 738-60.01

PROJECT NAME Gettler-Ryan, Shell, 4226 First St., Pleasanton PAGE 1 OF 1

BY MGB DATE 9/27/85

SURFACE ELEV. 373'±

	,	·				·	JOKINGE LLEV. 3/31±
TORVANE (TSF)	POCKET PENETRO- METER (TSF)	PENETRA- TION (Blows/ Ft.)	GROUND WATER LEVELS	DEPTH IN FT.	SAMPLES .	LITHO- GRAPHÌC COLUMN	DESCRIPTION
	3.6 	Push 2 64 39 41	∀	10 15 20 (65 25 30 40	(1)	GC O CO	CONCRETE. SAND - Fill; very dark gray (5Y, 3/1); fine to coarse grained; trace fine gravel; trace fines; loose; damp; strong gasoline odor. @7': strong gasoline odor. CLAYEY GRAVEL; olive gray (5Y, 5/2); to olive (5Y, 4/3); fine to coarse grained; 30% fines; 15% fine to coarse sand; very dense; damp; moderate gasoline odor. CLAY; light olive brown (2.5Y, 5/6) to dark grayish brown (2.5Y, 4/2); 15% fine sand; trace coarse sand; very stiff; damp; no gasoline odor. @19': olive gray (5Y, 4/2) to olive (5Y, 5/6); 20% fine to medium sand; no coarse sand; no gasoline odor. @24': olive (5Y, 4/4); 25% fine to coarse sand; very plastic; soft; faint gasoline odor. BOTTOM OF BORING AT 24½ FEET.

REMARKS Drilled by 8-inch continuous flight, hollow stem auger; samples collected with 2-inch California modified split-spoon sampler; borehole backfilled with soil cuttings to ½ foot; concrete to surface.



PROJECT NUMBER 738-60.01

BORING NO. S-C

PROJECT NAME Gettler-Ryan, Shell, 4226 First St., Pleasanton

PAGE 1 OF 1

BY MGB DATE 9/27/85

SURFACE ELEV. 373'±

D 1	maa	. Ur	VIE 3	141	/ 03			SURFACE ELEV. 3/3'±
TORVANE (TSF)	POCKET PENETRO- METER (TSF)	PENETRA- TION (Blows/ Ft.)	GROUND WATER LEVELS	DEPTH IN FT.	SAMPLES	CR.A	HO- PHIC UMN	DESCRIPTION
	0.4	Push 2 30 50for 6" 19 72 48		5- 10- 15 20- 25- 30-	(5) (6) (7)	CL GC CL SW ML SC		CONCRETE. SAND - Fill; very dark gray (5Y, 3/1); fine to coarse grained; trace fine gravel; trace fines; damp; strong gasoline odor. 07': loose; strong gasoline odor. CLAY; olive (5Y, 5/6, 5/3); 20% fine to coarse sand; silty; hard; damp; no gasoline odor. CLAYEY GRAVEL; olive (5Y, 5/6, 5/4); fine grained; 35% fine to coarse sand; 15% fines; very dense; damp; no gasoline odor. CLAY; yellowish brown (10YR, 5/6, 5/8); 35% fine to coarse sand; silty; soft; moist; no gasoline odor. SAND: olive (5Y, 4/3); fine to coarse grained; 10% fines; medium dense; moist; no gasoline odor. SANDY SILT; light olive brown (2.5Y, 5/6) 40% fine sand; very stiff; moist; no gasoline odor. CLAYEY SAND; olive brown (2.5Y, 4/4); fine to coarse grained; 40% clay; dense; moist; faint gasoline odor. BOTTOM OF BORING AT 28 FEET

REMARKS Drilled by 8-inch continuous flight, hollow-stem auger; samples collected with 2-inch California modified split-spoon sampler; borehole backfilled with concrete from 28 to 15 feet, soil cuttings to ½ foot; concrete to surface.



PROJECT NUMBER 738-60.01

DATE 9/27/85

H.

BY

MGB

BORING NO. S-D

PROJECT NAME Gettler-Ryan, Shell, 4226 First St., Pleasanton

PAGE 1 OF 1

SURFACE ELEV. 374'±

TORVANE (TSF)	POCKET PENETRO- METER (TSF)	PENETRA- TION (Blows/ Ft.)	CROUND WATER LEVELS	DEPTH IN FI	SAMPLES	LITH GRAP COLL	ΥНС	DESCRIPTION	
	4.25	Push 2		5-	(1)	SW		CONCRETE. SAND - Fill; very dark gray (5Y, 3/1); fine to coarse grained; 15% fine gravel; trace fines; loose; damp; strong gasoline odor. 07': strong gasoline odor. CLAY; olive yellow (5Y, 6/8) to olive	
	5	44	- -	15	4				(5Y, 4/3); 20% fine to coarse sand; silty; hard; damp; faint gasoline odor. 014': olive (5Y, 4/3); 35% fine to coarse sand; 10% fine gravel; faint gasoline odor.
,	2.2 1.25·	22 31	-	20	(<u>§</u>	ML		@19': olive (5Y, 4/3); to gray (5Y, 5/1); 20% fine to medium sand; slightly silty; very stiff; damp; faint gasoline odor. SANDY SILT; olive (5Y, 4/4); 40% fine sand; slightly clayey; stiff; damp; faint gasoline odor.	

BOTTOM OF BORING AT 221 FEET.

REMARKS Drilled by 8-inch continuous flight, hollow-stem auger; samples collected with 2-inch California modified split-spoon sampler; borehole backfilled with concrete from 22½ to 11½ feet, soil cuttings to ½ foot; concrete to surface.



PROJECT NUMBER 738-60.01

BORING NO. S-1

PROJECT NAME Gettler-Ryan, Shell, 4226 First St., Pleasanton

PAGE 1 OF 1

BY MGB

DATE 9/27/85

SURFACE ELEV. 373'±

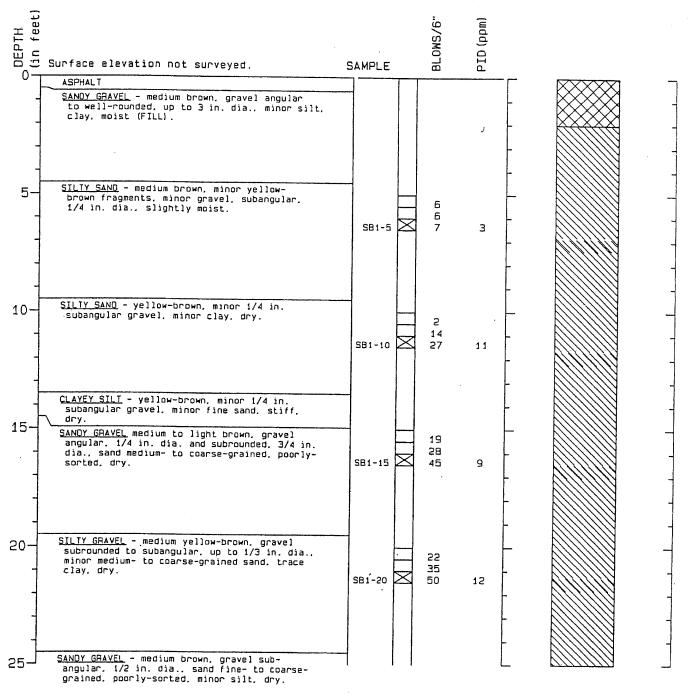
TORVANE (TSF)	POCKET PENETRO- METER (TSF)	PENETRA- TION (Blows/ Ft.)	GROUND WATER LEVELS	DEPTH IN FT.	SAMPLES	GR.	THO- APHIC DLUMM	DESCRIPTION
	4.25 	34 28 57		5 - 10 15 20 35 30 35 40	(3)	SW SC		ASPHALT and GRAVEL - Fill SAND - Fill; very dark gray (5Y, 3/1); fine to coarse grained; 10% fine gravel; trace fines; damp; moderate gasoline odor. CLAYEY SAND; very dark gray (5Y, 3/1); fine to coarse grained; damp; moderate gasoline odor. @12½': 10% fine gravel. CLAY; light olive brown (2.5Y, 5/6); 5% fine to coarse sand; silty; hard; damp; faint gasoline odor. @19': 20% fine to coarse sand; silty; very stiff; faint gasoline odor. CLAYEY GRAVEL; olive (5Y, 5/4); fine grained; 35% fine to coarse sand; clayey; very dense; damp; no gasoline odor. @29': no gasoline odor. BOTTOM OF BORING AT 30½ FEET.

REMARKS Drilled by 8-inch continuous flight, hollow-stem auger; samples collected with 2-inch California modified split-spoon sampler; borehole converted to 3-inch monitoring well as detailed on Plate F.



Boring Log SB-1

Geologic Log



- Refer to Figure A-1 for explanation of descriptions and symbols.
- Soil description and stratum lines are interpretive and actual changes may be gradual.
- 3. No free water encountered.



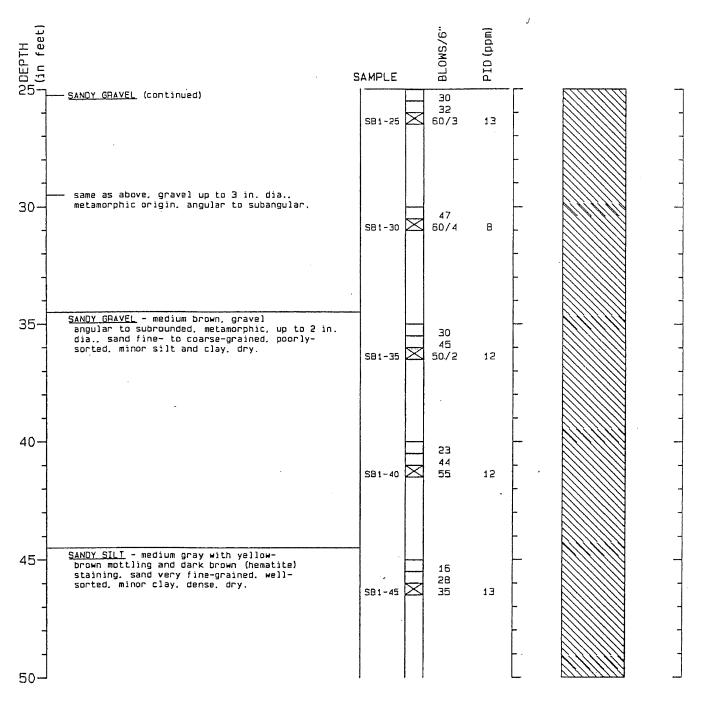
J-6006

4/90

Figure A-2

Page 1 of 3

Geologic Log



- Refer to Figure A-1 for explanation of descriptions and symbols.
- Soil description and stratum lines are interpretive and actual changes may be gradual.
- 3. No free water encountered.



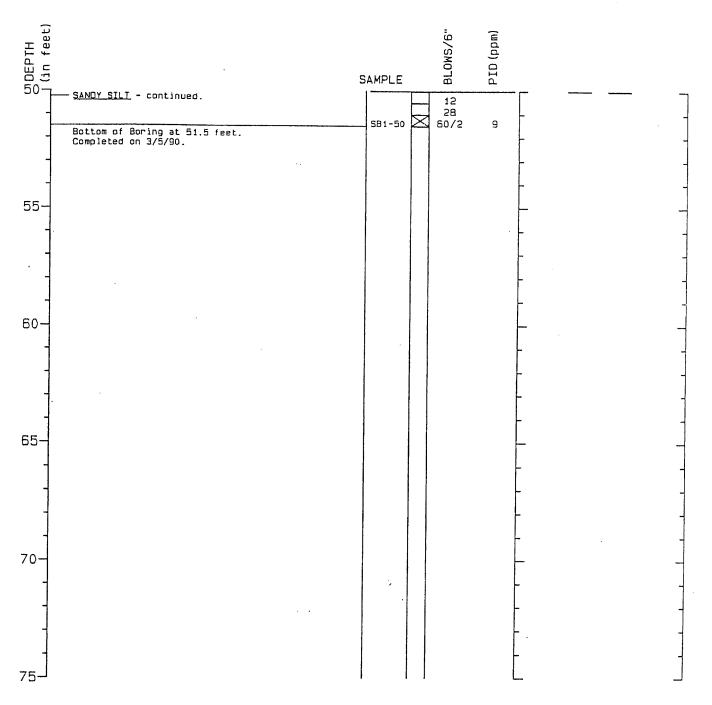
J-6006

4/90

Figure A-2

Page 2 of 3

Geologic Log

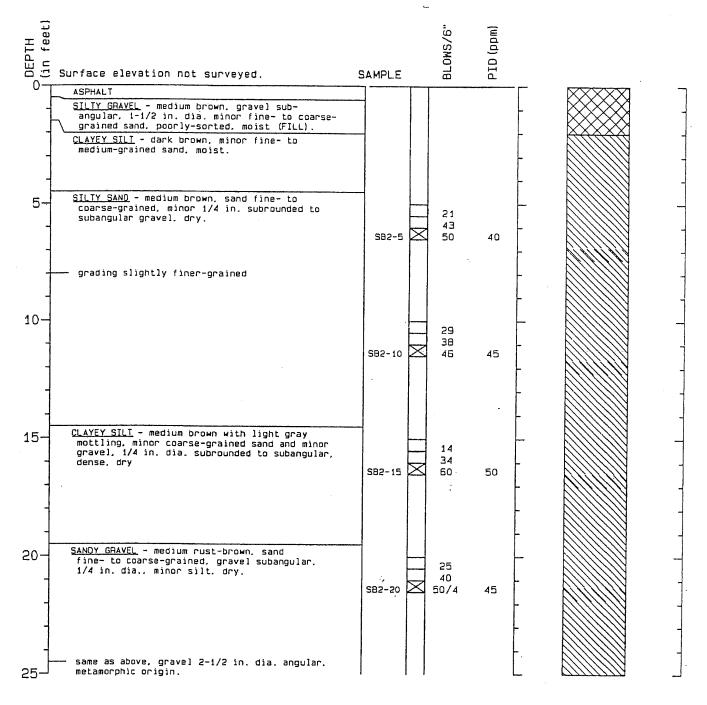


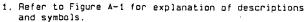
^{1.} Refer to Figure A-1 for explanation of descriptions and symbols.



Soil description and stratum lines are interpretive and actual changes may be gradual.
 No free water encountered.

Geologic Log





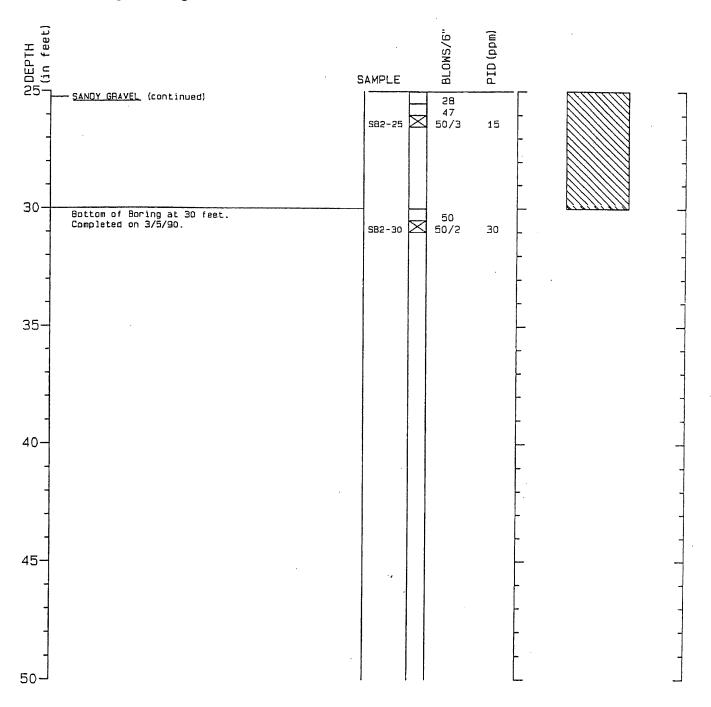
- Soil description and stratum lines are interpretive and actual changes may be gradual.
- 3. No free water encountered.



Figure A-3

Page 1 of 2

Geologic Log

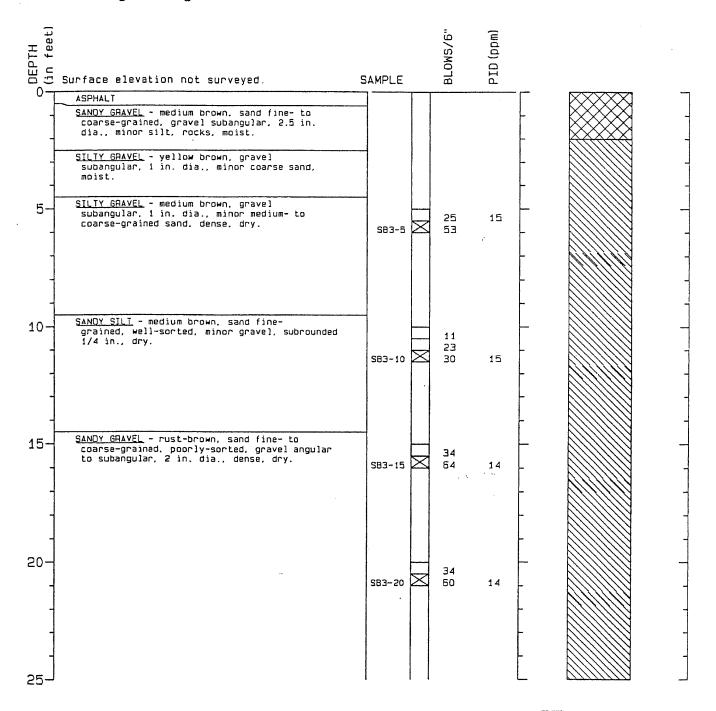


- 1. Refer to Figure A-1 for explanation of descriptions
- 2. Soil description and stratum lines are interpretive and actual changes may be gradual. 3. No free water encountered.



Figure A-3 Page 2 of 2

Geologic Log



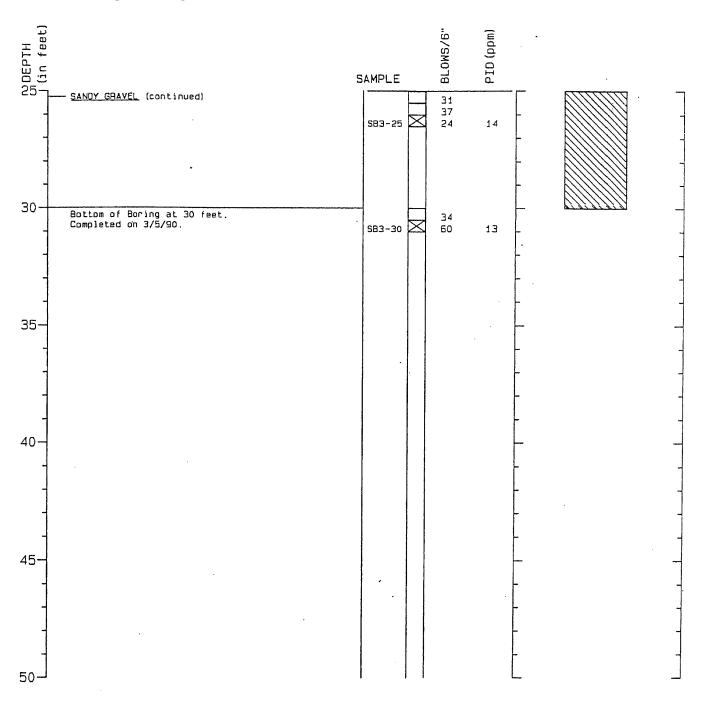
- 1. Refer to Figure A-1 for explanation of descriptions and symbols.
- 2. Soil description and stratum lines are interpretive and actual changes may be gradual.

 3. No free water encountered.



Figure A-4 Page 1 of 2

Geologic Log



- 1. Refer to Figure A-1 for explanation of descriptions and symbols.
- 2. Soil description and stratum lines are interpretive and actual changes may be gradual.

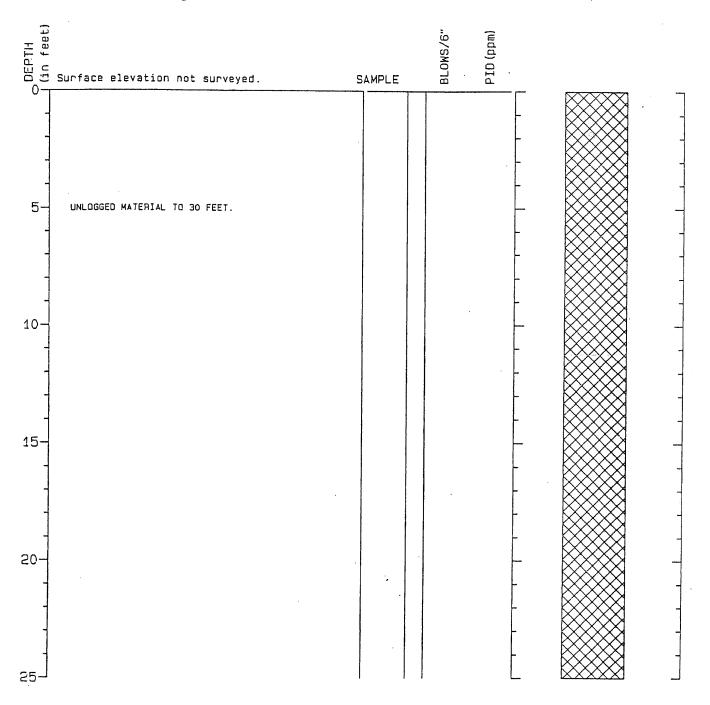
 3. No free water encountered.

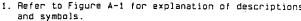


Figure A-4 Page 2 of 2

Boring Log WA-1

Geologic Log





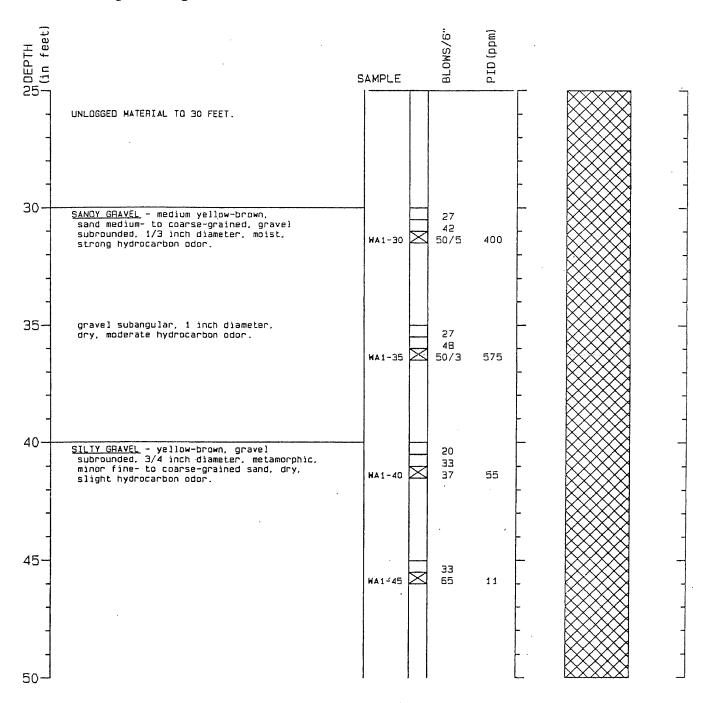
Refer to Figure A-1 for explanation of descriptions and symbols.
 Spil description and stratum lines are interpretive and actual changes may be gradual.
 No free water encountered.



Page 1 of 3

Boring Log WA-1

Geologic Log



- Refer to Figure A-1 for explanation of descriptions and symbols.
- Soil description and stratum lines are interpretive and actual changes may be gradual.
- 3. No free water encountered.

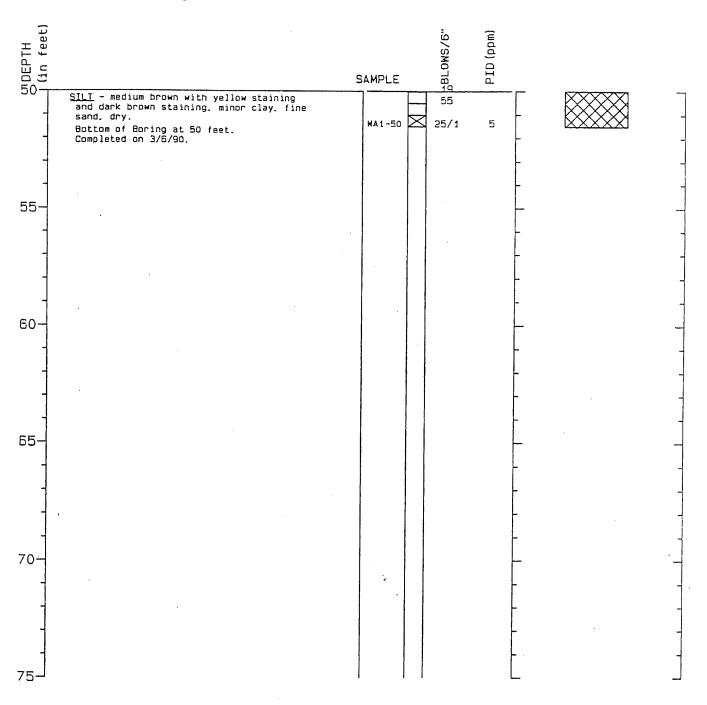


Figure A-5

Page 2 of 3

Boring Log WA-1

Geologic Log

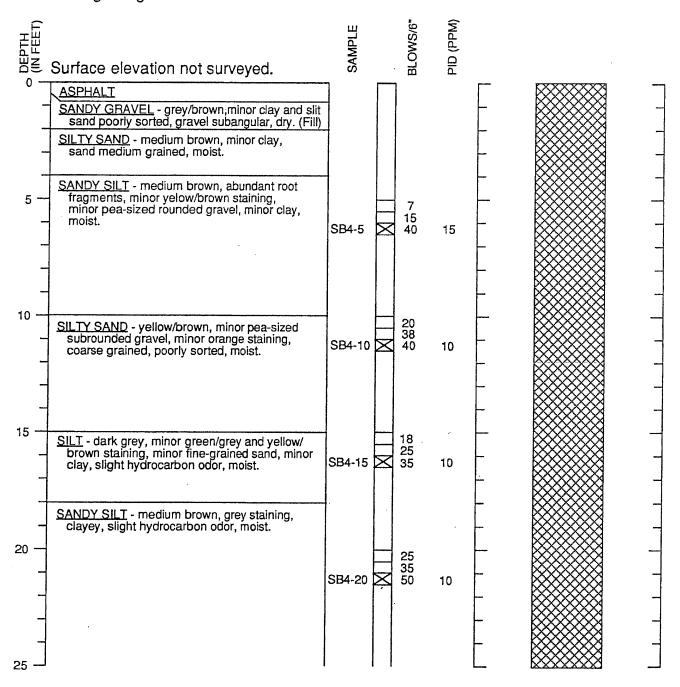


^{1.} Refer to Figure A-1 for explanation of descriptions and symbols.



Soil description and stratum lines are interpretive and actual changes may be gradual.
 No free water encountered.

Geologic Log

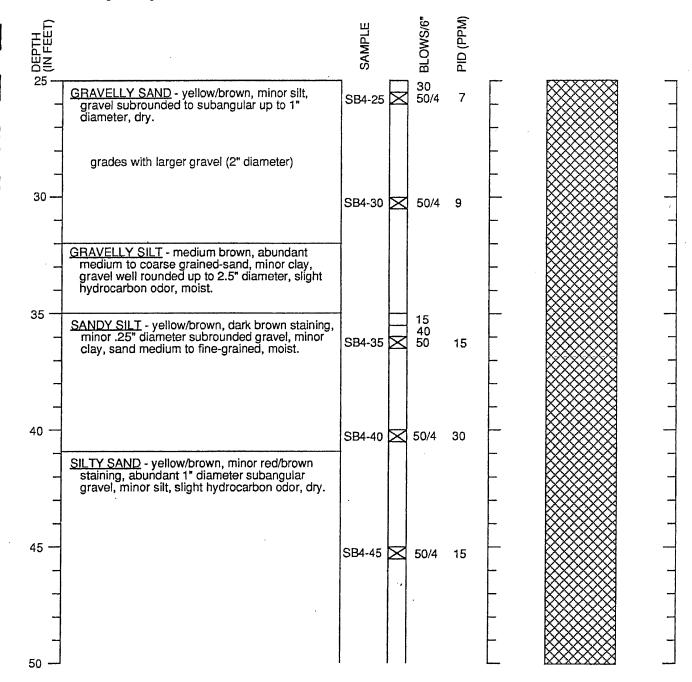


- 1. Refer to Figure A-1 for explanation of descriptions and symbols.
- Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
 No free water encountered.



12/90 J-6006 Figure A-2 Page 1 of 3

Geologic Log



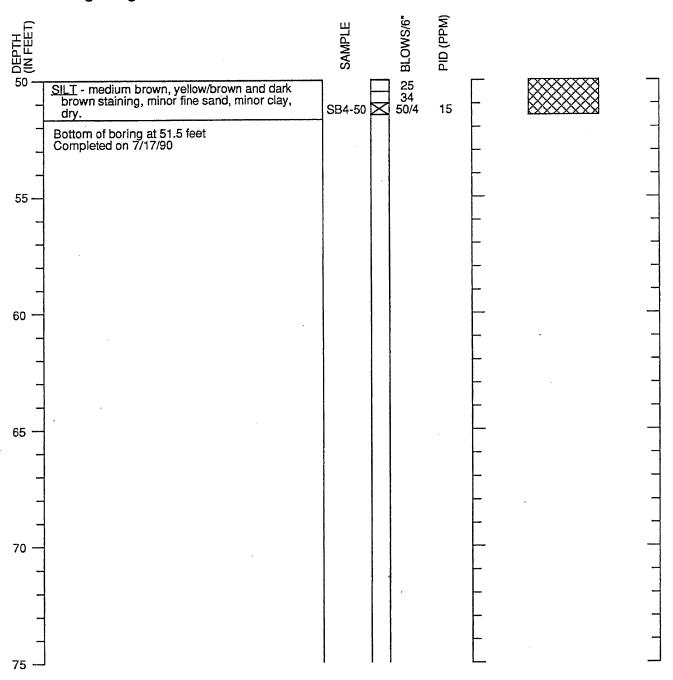
- Refer to Figure A-1 for explanation of descriptions and symbols.
- Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
- 3. No free water encountered.



HARTCROWSER

J-6006 12/90 Figure A-2 Page 2 of 3

Geologic Log

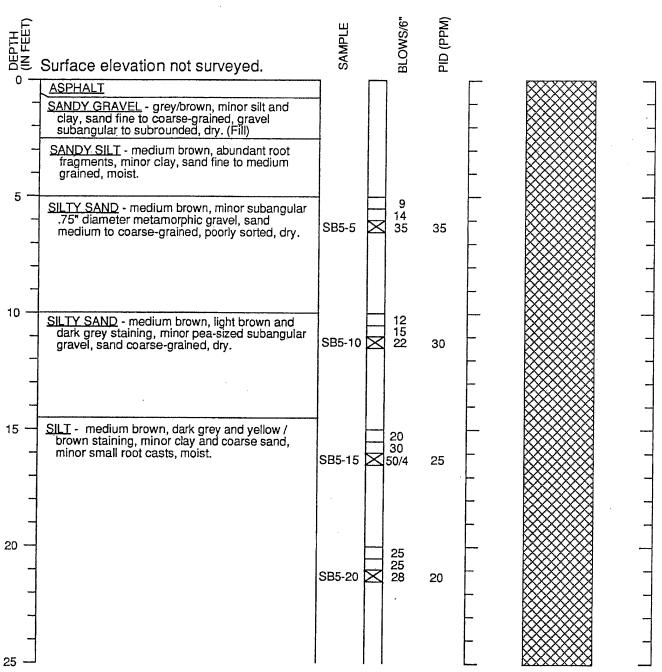


- Refer to Figure A-1 for explanation of descriptions and symbols.
- Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
 No free water encountered.



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Geologic Log

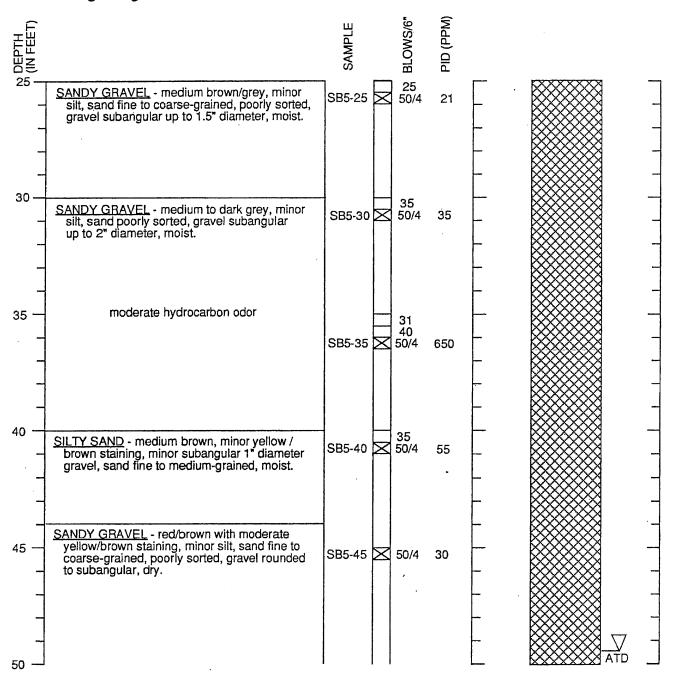


- Refer to Figure A-1 for explanation of descriptions and symbols.
- Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
- 3. Perched water encountered at 49.5 feet BGS.



J-6006 12/90 Figure A-3 Page 1 of 3

Geologic Log.



- Refer to Figure A-1 for explanation of descriptions and symbols.
- Soil descriptions and stratum lines are interpretive and actual changes may be gradual.
- 3. Perched water encountered at 49.5 feet BGS

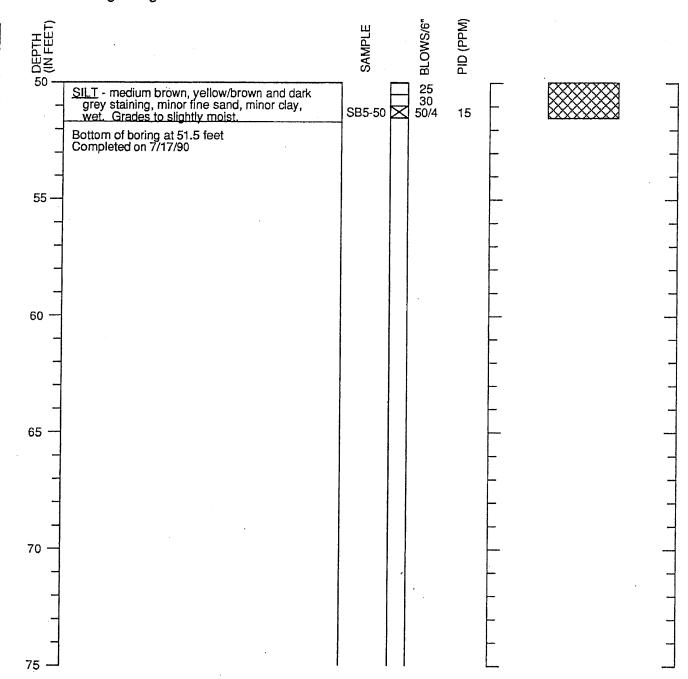


HARTCROWSER

12/90

J-6006 Figure A-3 Page 2 of 3

Geologic Log



- Refer to Figure A-1 for explanation of descriptions and symbols.
- 2. Soil descriptions and stratum lines are interpretive and actual changes may be gradual.

 3. Perched water encountered at 49.5 feet.



J-6006 12/90 Figure A-3 Page 3 of 3



WELL LOG (TPH-G) G:NPLE4228/GINT\PLE4228.GPJ DEFAULT.GDT 8/11/99

Cambria Environmental Technology, Inc. 1144 - 65th St. Oakland, CA 94608 Telephone: (510) 420-0700 Fax: (510) 420-9170

BORING/WELL LOG

PAGE 1 OF 3

JOB/SITE NAME ple-4226 LOCATION 4226 First Street, Pleasan PROJECT NUMBER 241-0395 DRILLER Gregg Drilling DRILLING METHOD Hollow-stem auger BORING DIAMETER 8" LOGGED BY B. Jakub REVIEWED BY B. Jakub REMARKS Hand augered to 4' bgs; lo									BORING/WELL NAME DRILLING STARTED DRILLING COMPLETED WELL DEVELOPMENT D GROUND SURFACE ELE TOP OF CASING ELEVAT SCREENED INTERVAL DEPTH TO WATER (First DEPTH TO WATER (Statio	ATE (YIELD) VATION FION Not Su NA Encountered	NA Not Surveyed urveyed			
TPHg (mg/kg)	BLOW	RECOVERY	SAMPLE ID	EXTENT	DEPTH (ft bgs)	U.S.C.S.	GRAPHIC LOG	LITHO	LOGIC DESCRIPTION		CONTACT DEPTH (# bgs)	WELL DIAGRAM		
	11 12 19				5 -	ML ML		clay, 70% silt, 25% fin plasticity; moderate to <u>SILT;</u> (ML); dark yello	own (10YR4/3); very soft; whe to medium grained sand to low estimated permeabilitions by brown (10YR4/6); stiff; mesand, 2% fine grained graved permeability.	; low y. noist;	0.3 1.5 4.5			
	15 25 31 16 25 35				 10 15	ML		38% clay, 50% silt, 29 coarse subangular grapermeability. @ 14.3 - olive brown	llow brown (10YR5/8); stiff; % fine grained sand, 10% fi avel; high plasticity; low est (2.5Y4/4) mottled with olive	ne to imated	9.7			
<1.0	11 22 25		SB-7 -15.0 SB-7 - 19.5	X	-20-	SP		low estimated permea	grained gravel; medium pl bility. (ilit; (SP); olive gray (5Y4/2) r, 15% silt, 62% fine to coar e to coarse grained gravel;		19.5 20.3			
<1.0	20 20 20		SB-7 - 24.5	X	-25	GP		plasticity; high estimat Clayey Sandy GRAVE 20% clay, 20% fine to coarse grained gravel medium plasticity; low permeability. Gravelly SAND with S dense; damp; 3% clay	led permeability. L; (GP); yellow brown (10Y coarse grained sand, 80% (quartz, possibly chert); low to moderate estimated ilt; (SP); yellow brown (10Y , 15% silt, 52% medium grad gravel; no plasticity; high	(R5/6); fine to v to (R5/6);	24.3 25.3			
<1.0	35 36 40		SB-7 - 29.3	X	-30-	GP P		Clayey SILT; (ML); stiffine grained sand; high permeability; trace car Sandy GRAVEL with C (5Y3/2); 15% clay, 5% sand, 45% fine to coar	f; damp; 30% clay, 60% sili n plasticity; low estimated	ained low	29.0			
	19			×	-35	1.		Clayey GRAVEL with S	Silt; (GC); yellow brown		34.0			

Continued Next Page



Cambria Environmental Technology, Inc. 1144 - 65th St. Oakland, CA 94608

BORING/WELL LOG

PAGE 2 OF 3

Telephone: (510) 420-0700 Fax: (510) 420-9170

CLIENT	N.	AME	
JOB/SIT	Έ	NAN	1E

LOCATION

Equiva Services LLC
ple-4226

4226 First Street, Pleasanton, California

SB-7

BORING/WELL NAME
DRILLING STARTED

 DRILLING STARTED
 07-Apr-99

 DRILLING COMPLETED
 07-Apr-99

Continued from Previous Page

CONTACT DEPTH (# bgs) TPHg (mg/kg) BLOW COUNTS RECOVERY GRAPHIC LOG DEPTH (ff bgs) U.S.C.S. SAMPLE LITHOLOGIC DESCRIPTION WELL DIAGRAM SB-7 圣 (10YR5/8); very dense; damp; 35% clay, 15% silt, 10% - 34.3 sand, 40% fine to coarse grained gravel (quartz); medium plasticity; moderate to low estimated permeability. 25 45 @ 39' - quartz, siltstone, chert gravels. 53 <1.0 SB-7 40.0 GC Ţ 25 @ 44' - moist to wet. 83 SB-7 50/3 44.5 49.0 Clayey GRAVEL; (GC); yellow brown (10YR5/4); very dense; moist to wet; 20% clay, 10% silt, 10% medium to coarse grained sand, 60% fine grained gravel; medium <1.0 -50 Portland Type -49.5 plasticity; low to moderate estimated permeability. GC 30 50/3 <1.0 SB-7 -55 - 54.3 59.0 Clayey SILT; (MH); mottled yellow brown (10YR4/6) and light brownish gray (2.5Y6/2); hard; dry; 20% clay, 70% silt, 10% very fine to fine grained sand; medium plasticity; <1.0 SB-7 -60 50/3 \times - 59.5 low estimated permeability. WELL LOG (TPH-G) G: PLE4226/GINT/PLE4226.GPJ DEFAULT.GDT МН @ 64' - dark brown MnO2 or organic blebs throughout. SB-7 - 64.5 69.0 Clayey SILT; (MH); light olive brown (2.5Y5/4); hard; dry; 25% clay, 75% silt; medium plasticity; very low SB-7 70 69.5 estimated permeability. 74.5 @ 74' - increasing mottled with yellow brown (10YR5/8).

Continued Next Page

BORING/WELL LOG

PAGE 3 OF



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CLIENT NAME	Equiva Services LLC	BORING/WELL NAM
JOB/SITE NAME	ple-4226	DRILLING STARTED
	1000 F1 + 0+ 1 D1 + 1 D H1	DOLLLING COMPLET

BORING/WELL NAME SB-7
DRILLING STARTED 07-Apr-99

DRILLING COMPLETED 07-Apr-99 LOCATION 4226 First Street, Pleasanton, California Continued from Previous Page CONTACT DEPTH (ft bgs) TPHg (mg/kg) RECOVERY GRAPHIC LOG BLOW COUNTS EXTENT U.S.C.S. DEPTH (ft bgs) SAMPLE LITHOLOGIC DESCRIPTION WELL DIAGRAM SB-7 @ 74' to 74.5' - black blebs, possibly MnO2. - 74.5 ر. 79.5 × SB-7 -80 50/2 MH 15 50 SB-7 @ 84' - dark yellow brown (10YR4/6); damp; 30% clay, - 85.0 46 50 @ 94' - MnO₂ blebs throughout; becomes siltier. 30 7-تار - 94.5 99.0 Clayey SAND with Gravel; (SC); dark yellow brown (10YR4/6); dense; damp; 30% clay, 5% silt, 50% fine to SB-7 SC 100.0 Bottom of 100.0 coarse grained sand, 15% fine grained gravel (quartz); Boring @ 100 ft LOG (TPH-G) G:PLE4226/GINT/PLE4226.GPJ DEFAULT.GDT 8/11/99 medium plasticity; low to moderate estimated permeability. Ground water sample (SB-7-GW) collected.

DELTA	Client Shell Oil Produ Project Number SJ42	Boring No. 3-1				
Address: 4226 1st Street Pleasanton, California Logged By: Andy Persio	Drilling Date(s): 3/27/07 Drilling Company: Gregg Drilling Method: HSA Boring Depth (ft): 35	ng Company: Gregg Sampling Method: Hand Auger/Split Spoon ng Method: HSA Well Depth (ft.): NA				
Depth (ft.) Water Level Soil/Rock Graphic Sampled Interval Blow Counts (blows/ft)	Recovery (%)	ock Visual Description	PID Reading (ppm) Boring Completion			
5— 10— 15— 20— 25— 30— 35—	SC: Clayey SAND, orar fines, 10% gravels up to 100% 5.5 - 7 feet bgs: as abov CL: Lean CLAY with sa 80% fines, low plasticity SC: Clayey SAND, orar 40% fines, trace gravel SC: Clayey SAND with coarse sand, 30-40% fidry. 83% (as above, orangish b 25-30% gravels up to 1 SC: Clayey SAND, ora 30% fines, 10% gravels	e, clay increasing, more compact, moist. Ind, light orangish brown, 20-30% fine sand, 70%, dry. Ingish brown, 60-70% fine to medium sand, 30-s, dry. Ingravel, dark brown to dark gray, 50-60% fine to nes, 10-20% gravels up to 1 inch in diameter, frown, 50-60% fine to coarse sand, 15-25% fine inch in diameter, dry) Ingish brown, 65-70% fine to coarse sand, 25-s up to 0.5 inches in diameter, dry. Ingish brown, 65-70% fine to coarse sand, 25-s up to 0.5 inches in diameter, dry.	13.3 2.4 4.8 33.3 33.3			

BORING LOG Client Shell Oil Products US Boring No. DELTA Project Number SJ4226F1X B-2 XInogen' Drilling Date(s): 3/27/07 Boring diameter (in.): 6 Casing Material: NA Address: Sampling Method: Hand Auger/Split Spoon Screen Interval: NA 4226 1st Street Drilling Company: Gregg Pleasanton, California Well Depth (ft.): NA Screen slot size: NA Drilling Method: HSA Casing Diameter (in.): NA Sand Pack: NA Boring Depth (ft): 35 Logged By: Andy Persio Water Level Soil/Rock Graphic Depth (ft.) Soil/Rock Visual Description ASPHALT: 6 inches concrete, 1-2 inches base rock. CL: Sandy lean CLAY, dark brown, 60-70% fines, 30-40% fine to medium sand. SC: Clayey SAND, orangish brown, 55-65% fine to medium sand, 35-6.4 45% fines, trace gravels. 5.5 - 7 feet bgs: as above, clay increasing, more compact, dry. (as above, 65-75% fine to medium sand, 25-35% fines, very dense, dry) 0.6 100% 10 10 CL: Sandy CLAY, orangish brown, 20-30% fine sand, 70-80% fines, low plasticity, dry. 100% 0.2 15 15 SC: Clayey SAND, orangish brown, 60-70% fine to coarse sand, 30-40% fines, trace gravels, dry. 83% 0.3 20 20 SC: Clayey SAND with gravel, brown to orangish brown, 50-60% fine to coarse sand, 25-35% fines, 5-25% gravels up to 1 inch in diameter, dry. 5.8 67% - 25 25 0.1 50% (same as above, dry) - 30 30 67% 33.2 (same as above, dry) 35 35 Bottom of boring = 35 feet bgs. Note: Sampled to 35 feet bgs. Page 1 of 1

BORING LOG Client Shell Oil Products US Boring No. DELTA Project Number SJ4226F1X B-3 XInogen_ Address: Drilling Date(s): 3/27-28/07 Boring diameter (in.): 8 Casing Material: NA Drilling Company: Gregg 4226 1st Street Sampling Method: Hand Auger/Split Spoon Screen Interval: NA Pleasanton, California Well Depth (ft.): NA Screen slot size: NA Drilling Method: HSA Boring Depth (ft): 35 Casing Diameter (in.): NA Sand Pack: NA Logged By: Andy Persio Soil/Rock Graphic Depth (ft.) Soil/Rock Visual Description ASPHALT: 6 inches concrete, 1-2 inches base rock. SC: Clayey SAND, orangish brown, 55-65% fine to medium sand, 35-45% fines. 100% 12.5 (as above, clay increasing, more compact, dry) 100% (as above, 60-70% fine to medium sand, 30-40% fines, trace gravels, 0.4 10 10 CL: Sandy lean CLAY, orangish brown, 30-40% fine sand, 60-70% fines, low plasticity, dry. 100% 6.2 15 - 15 SC: Clayey SAND, orangish brown, 60-70% fine sand, 30-40% fines, 83% 2.1 - 20 20 67% (as above, 60-70% fine to coarse sand, 20-30% fines, 10% gravels up to 98.1 0.5 inches in diameter, dry) - 25 25 536 (as above, 50-60% fine to medium sand, 30-40% fines, 5-10% gravels 50% up to 0.5 inches in diameter, dry) 30 30 SC: Clayey SAND with gravel, dark brown, 50-60% fine to coarse sand, 25-35% fines, 15-25% gravels up to 1 inch in diameter, dry. 2.7 83% - 35 35 Bottom of boring = 35 feet bgs. Note: Sampled to 35 feet bgs. Page 1 of 1

BORING LOG Client Shell Oil Products US Boring No. DELTA Project Number SJ4226F1X **B-4** XInogen' Address: Drilling Date(s): 3/27-28/07 Boring diameter (in.): 6 Casing Material: NA 4226 1st Street Drilling Company: Gregg Sampling Method: Hand Auger/Split Spoon Screen Interval; NA Pleasanton, California Drilling Method: HSA Well Depth (ft.): NA Screen slot size: NA Logged By: Andy Persio Boring Depth (ft): 35 Casing Diameter (in.): NA Sand Pack: NA Nater Level Soil/Rock Graphic Depth (ft.) Soil/Rock Visual Description ASPHALT: 6 inches concrete, 2 inches base rock. SC: Clayey SAND with gravel, dark brown, 50-60% fine to medium sand, 20-30% fines, 10-20% gravels up to 1 inch in diameter, dry. 100% 56.3 67% (same as above, dry) 13.0 10 10 SC: Clayey SAND, orangish brown, 50-60% fine to medium sand, 40-50% fines, trace gravels, dry. 67% 5.8 15 (as above, 60-70% fine to coarse sand, 30-40% fines, trace gravels, dry) 80% 20 20 100% (as above, 55-65% fine to coarse sand, 30-40% fines, 5-15% gravels up 12.3 to 1 inch in diameter, dry) 25 25 SC: Clayey SAND with gravel, orangish brown, 50-60% fine to coarse sand, 25-35% fines, 15-25% gravels up to 1.5 inches in diameter, dry. 67% 18.2 30 30 83% 46.5 (same as above, dry) 35 35 Bottom of boring = 35 feet bgs. Note: Sampled to 35 feet bgs. 40 Page 1 of 1

BORING LOG Client Shell Oil Products US Boring No. DELTA Project Number SJ4226F1X B-5 X Inogen Address: Drilling Date(s): 3/27-28/07 Boring diameter (in.): 6 Casing Material: NA 4226 1st Street Drilling Company: Gregg Sampling Method: Hand Auger/Split Spoon Screen Interval: NA Pleasanton, California Drilling Method: HSA Well Depth (ft.): NA Screen slot size: NA Logged By: Andy Persio Boring Depth (ft): 35 Casing Diameter (in.): NA Sand Pack: NA PID Reading (ppm) Depth (ft.) Soil/Rock Graphic Depth (ft.) Soil/Rock Visual Description ASPHALT: 6 inches concrete, 2 inches base rock. SC: Clayey SAND with gravel, brown, 50-60% fine to medium sand, 20-30% fines, 10-20% gravels up to 1 inch in diameter, dry. 100% 1.7 100% 2.4 10 (as above, orangish brown, 50-60% fine to medium sand, 35-45% fines, 5-10% gravels up to 0.5 inches in diameter, dry) 100% 0.1 CL: Sandy lean CLAY, dark brown to dark gray, 65-75% fines, 25-35% 15 15 fine sand, trace gravels up to 0.5 inches in diameter, low plasticity, dry. 100% 6.1 20 20 (as above, orangish brown, 60-70% fines, 30-40% fine sand, medium plasticity, dry) 100% 7.4 SC: Clayey SAND, orangish brown, 55-65% fine to medium sand, 35-25 25 45% fines, dry. 67% 7.4 SC: Clayey SAND with gravel, dark gray, 50-60% fine to coarse sand, 30 30 25-35% fines, 10-20% gravels up to 1.5 inches in diameter, dry. 100% (as above, orangish brown, 50-60% fine to coarse sand, 35-45% fines, 887 5-15% gravels up to 1 inch in diameter, moist) 35 35 Bottom of boring = 35 feet bgs. Note: Sampled to 35 feet bgs. Page 1 of 1

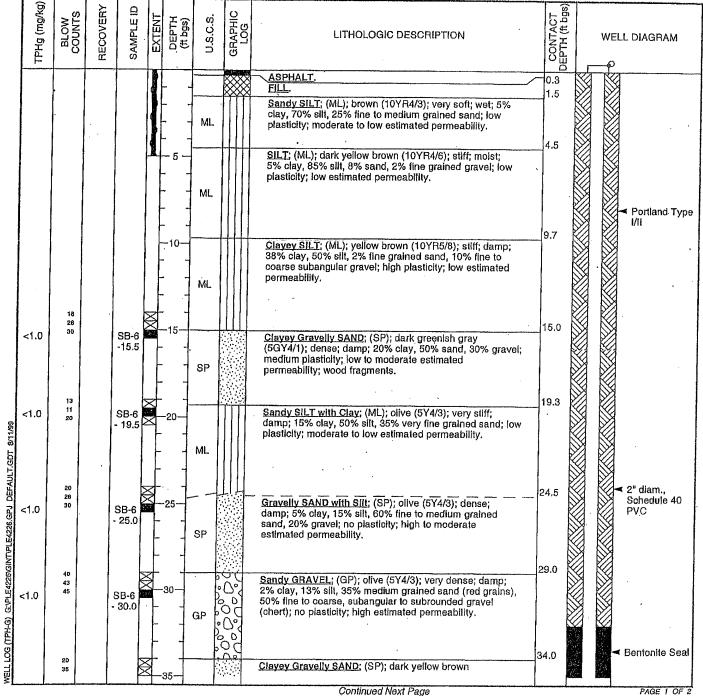


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BORING/WELL LOG

(SB-6)

CLIENT NAME	Equiva Services LLC	BORING/WELL NAME MW-1						
JOB/SITE NAME	pie-4226	DRILLING STARTED 08-Apr-99						
LOCATION	4226 First Street, Pleasanton, California	DRILLING COMPLETED 09-Apr-99						
PROJECT NUMBER	241-0395	WELL DEVELOPMENT DATE (YIELD) NA						
DRILLER	Gregg Drilling	GROUND SURFACE ELEVATION 371,83 ft						
DRILLING METHOD _	Hollow-stem auger	TOP OF CASING ELEVATION 371,20 ft						
BORING DIAMETER _	8 ¹¹	SCREENED INTERVAL 37.5 to 57.5 ft bgs						
LOGGED BY	B. Jakub	DEPTH TO WATER (First Encountered) 42.5 ft (08-Apr-99)						
REVIEWED BY	B. Jakub	DEPTH TO WATER (Static) NA						
REMARKS	Hand augered to 5' bgs; located near NW planter	/entrance to Shell station on Vineyard and W of SB-7.						
···		The state of the s						
mg/kg)	C.S. C.S.	ACT (# bgs)						



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BORING/WELL LOG

PAGE 2 OF

CLIENT NAME _____

Equiva Services LLC ple-4226 MW-1

BORING/WELL NAME

DRILLING STARTED 08-Apr-99 LOCATION 4226 First Street, Pleasanton, California Continued from Previous Page CONTACT DEPTH (ft bgs) TPHg (mg/kg) SAMPLE ID GRAPHIC LOG RECOVERY BLOW COUNTS EXTENT DEPTH (ft bgs) U.S.C.S. LITHOLOGIC DESCRIPTION WELL DIAGRAM <1.0 SB-6 (10YR4/6); very dense; damp; 20% clay, 10% silt, 40% - 35.0 medium grained sand, 30% fine to coarse grained gravel ■ Monterey (sandstone/claystone, serpentinite, some MnO₂/Fe staining); low plasticity; moderate to low estimated Sand #3 permeability. 60/4 <1.0 SB-6 - 40.0 SP 立 @ 44' - moist to wet. 45 46 2"-diam., 0.020" Slotted Schedule 40 PVC 50.0 Clayey GRAVEL with Silt; (GC); dark yellow brown (10YR4/6); very dense; moist to wet; 25% clay, 15% silt, 20% fine to coarse grained sand, 40% fine to coarse grained gravel, GC 55.2 Clayey SILT; (MH); light olive brown (2.5Y5/4); hard; damp; 25% clay, 75% silt; medium to high plasticity; very 40 МН low estimated permeability; black MnO2 blebs throughout. 58.0 Bottom of Boring @ 58 ft WELL LOG (TPH-G) G:\PLE4226\G\NT\PLE4226.GPJ DEFAULT.GDT 8/11/99

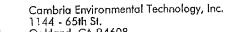
BORING/WELL LOG



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	BORING LOGGED	E NAMI DN T NUM G METI DIAME DIAME DIAME DIAME	SH 42 BER 24 GHOD HOD HOD B. ETER 8" B. S.	26 1-0 egg bliov Jal	First St 395 Drillin w-stem kub rk, RG	d servi reet, P g auger	ce sta		BORING/WELL NAME DRILLING STARTED DRILLING COMPLETED WELL DEVELOPMENT DA GROUND SURFACE ELEV TOP OF CASING ELEVAT SCREENED INTERVAL DEPTH TO WATER (First DEPTH TO WATER (Statio	ATE (YIELD)/ATION	372.6 It above It bgs 33.	5 ft above r e msi .0 ft (18-Jar	
	ТРНд (ррт)	BLOW COUNTS	SAMPLE ID	EXTENT	DEPTH (ft bgs)	U.S.C.S.	GRAPHIC LOG	цт і но	LOGIC DESCRIPTION		CONTACT DEPTH (# bgs)	WELI	L DIAGRAM
WELL LOG (SHELL) G.NPLEASA-4(GINTP-LE4226.GPJ DEFAULT.GDT 6/23/00	<1.0		MW-2-8.3' MW-2-16.5'		- 5	ML SM MH SM SM SM GM		ASPHALT. Sandy SiLT; (ML); da 80% silt, 15% fine gra 16w to moderate estin Sility SAND; (SM); yei 43% silt, 50% sand, 8 estimated permeabilit Clayey SiLT; (MH); yei 50% silt, 2% fine gral subangular gravel; his permeability. Sility SAND; (SM); yei 40% silt, 50% sand, 8 estimated permeability. Clayey Silty SAND; (SM); yei 40% silt, 50% sand, 8 estimated permeability. Sility SAND; (SM); yei 40% silt, 50% sand, 8 estimated permeability. Q 12,8' - 10% clay, 30% silt, 50% sand, 8 estimated permeability. Q 12,8' - 10% clay, 35% sal estimated permeability. Gravelly Silty SAND; (solay, 25% silt, 45% filt gravel to 3/4"; high estimated permeability. Clayey Silty SAND; (clay, 25% silt, 50% silt, 50% silt, 50% silt, 50% silt, 20% silt,	ellow brown; stiff; damp; 38 ned sand, 10% fine to coar gh plasticity; low estimated llow brown; dense; damp; 28% gravel; no plasticity; hig ty, 58M); yellow brown; stiff; dan premeability. Illow brown; dense; damp; 28% gravel; no plasticity; hig ty. 10% sand, 8% gravel; higher sand, 2% gravel; medium plasticy. (SM); yellow brown; damp; and, 10% fine gravel. (SM); yellow brown; damp; and, 10% fine to coarse grand, 10% fine to coarse grand, 40% fine grand, 40% fine to coarse grand, 40% fine grand, 40% fine to coarse grand, 40% fine to coarse grand, 40% fine grand, 40% f	plasticity; ee roots. clay, oderate % clay, se, 2% clay, h mp; asticity; 2% clay, h vel; 15% sticity; low 25% 15% p; 10% avel; chert	10.0 11.0 16.5 18.2 22.0		 ✓ 4" diam., Schedule 40 PVC ✓ Portland Type I/II ✓ Bentonite Seal ✓ Lonestar Sand #3
VELL LOG (SHELL) G:\PLEASA-4\@	<1.0	35 50/8 38 50/8 45 50/8	MW-2-30.5'		30 30 	GM 		very dense; damp; 14 fine to coarse gravel; permeability. @ 31.7' - color chang Sandy Clayey GRAV saturated; 25% clay,	BRAVEL; (GM); dark yellow 5% clay, 15% silt, 20% san low plasticity; moderate es ge to dark greenish gray; ch EL; (GC); very dense; wet to 15% silt, 20% sand, 40% g	d, 50% stimated nert to 2" to	30.0		PAGE 1 OF 2

Continued Next Page



CLIENT NAME

JOB/SITE NAME

Oakland, CA 94608 Telephone: (510) 420-0700 Fax: (510) 420-9170

MW-2 BORING/WELL NAME Equiva Services LLC DRILLING STARTED 18-Jan-00 Shell-branded service station

BORING/WELL LOG

19-Jan-00 DRILLING COMPLETED 4226 First Street, Pleasanton, California LOCATION Continued from Previous Page CONTACT DEPTH (ft bgs) TPHg (ppm) GRAPHIC LOG BLOW COUNTS U.S.C.S. DEPTH (ft bgs) SAMPLE EXTENT WELL DIAGRAM LITHOLOGIC DESCRIPTION Sandy Clayey GRAVEL; (GC); very dense; wet to saturated; 25% clay, 15% silt, 20% sand, 40% gravel. <1.0 MW-2-35.0 4"-diam., 0.020" Slotted Schedule 40 50/8 GC PVC 35 40.3 Sandy Gravelly SILT; (ML); hard; saturated; 12% clay, 58% silt, 15% sand, 15% gravel; medium plasticity; low estimated permeability. 50/8 ML **60/**6 43.5 Sandy Clavey SILT; (ML); hard; saturated; 15% clay, 60% silt, 15% sand, 10% gravel. ML 50/8 45.0 Sandy SILT; (ML); hard; saturated; 12% clay, 45% silt, 43% fine grained sand; slight plasticity; low estimated ML permeability. 19 27 48.0 Bottom of Boring @ 48 ft



WELL LOG (SHELL) G:IPLEASA-4/GINT/PLE4226.GPJ DEFAULT.GDT 6/23/00

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BORING/WELL LOG

CLIENT NAME Equiva Services LLC **BORING/WELL NAME** MW-3 JOB/SITE NAME Shell-branded service station DRILLING STARTED 18-Jan-00 DRILLING COMPLETED ___19-Jan-00 LOCATION 4226 First Street, Pleasanton, California PROJECT NUMBER WELL DEVELOPMENT DATE (YIELD) 03-Feb-00 241-0395 375.90 ft above msl **GROUND SURFACE ELEVATION** DRILLER Gregg Drilling **DRILLING METHOD** Hollow-stem auger TOP OF CASING ELEVATION 375.05 ft above msl

. BORING DIAMETER _ SCREENED INTERVAL 20 to 35 ft bgs B, Jakub DEPTH TO WATER (First Encountered) ___25.0 ft (18-Jan-00)

LOGGED BY S. Bork, RG# 5620 Y REVIEWED BY_ **DEPTH TO WATER (Static)** NA REMARKS Hand augered to 5' bgs. CONTACT DEPTH (# bgs) GRAPHIC LOG (mdd) BLOW COUNTS DEPTH (ft bgs) SAMPLE EXTEN U.S.C.S LITHOLOGIC DESCRIPTION WELL DIAGRAM TPHg (ASPHALT. SILT; (ML); brown; soft; damp; 2% clay, 90% silt, 7% 0.5 fine grained sand, 1% gravel; no plasticity; moderate to low estimated permeability. ML 4" diam., Schedule 40 5.3 **PVC** MW-3-5.0' <1.0 SM Silty SAND; (SM); yellow brown; dense; damp; 2% clay, 43% silt, 55% sand. 5.8 28 Clayey SILT; (ML); yellow brown; hard; wet; 15% clay, 60% silt, 5% sand, 10% fine to coarse, subangular gravel; medium plasticity; low estimated permeability. ML 50/6 MW-3-10.5' <1.0 Porlland Type 15.0 Gravelly Silty SAND; (SM); yellow brown; very dense; 5% clay, 20% sllt, 60% fine grained sand, 15% fine, <1.0 MW-3-15.5' subangular to subrounded gravel. ■ Bentonite Seal Lonestar Sand SM MW-3-20.5' <1.0 @ 22' - silt layer noted by driller. _ _ _ 25.0 Clayey Silty SAND; (SM); yellow brown; dense; wet to saturated; 15% clay, 35% sllt, 40% fine grained sand, 10% MW-3-25.5' <1.0 fine to coarse, subangular gravel; low plasticity; low estimated permeability. 4"-diam., SM 0.020" Slotted Schedule 40 **PVC** 30.0 Sandy Silty GRAVEL; (GM); very dense; 5% clay, 30% silt, 25% sand, 40% gravel; low plasticity; moderate 50/6 estimated permeability. GM 35.0 PAGE 1 OF 2

Continued Next Page



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BORING/WELL LOG

PAGE 2 OF 2

CLIENT NAME Equiva Services LLC BORING/WELL NAME MW-3

JOB/SITE NAME Shell-branded service station DRILLING STARTED 18-Jan-00

LOCATION 4226 First Street, Pleasanton, California DRILLING COMPLETED 19-Jan-00

Continued from Previous Page CONTACT DEPTH (ft bgs) TPHg (ppm) SAMPLE 1D GRAPHIC LOG BLOW COUNTS U.S.C.S. EXTENT DEPTH (ft bgs) LITHOLOGIC DESCRIPTION WELL DIAGRAM SILT; (ML); light brown; hard; 10% clay, 80% silt, 10% sand; low plasticity; low estimated permeability. ML ≺ Bentonite Seal 40.0 Clayey SILT; (ML); hard; 20% clay, 70% silt, 10% fine grained sand; medium plasticity; low estimated 25 ML 41.5 permeability. Bottom of Boring @ 41.5 WELL LOG (SHELL), G:PLEASA-4/GINTPLE4226.GPJ DEFAULT.GDT 6/23/00

numerous and												
,			Project No		Sj42-26F			ent:	Shell Oil Products		Boring No: WO-1	
<u>.</u>			Logged B	y:		Buckingham		cation:	4226 First Street, F	I	Page 1 of 2	
)elt		Driller:		Gregg			te Drilled:	6/10/2005	Location Map		
		La	Drilling M		Direct Pu		Hole Diameter			Please see site map		
			Sampling		GeoProb	Well Diamete			. 37 ft	Please se	ee site map	
	vironme		Casing T						r:			
Cor	sultants	s, Inc.	Slot Size:					ell Depth:				
			Gravel Pa					Casing Stickup:		-		
				Elevation		'	Northing		Easting			
/Vell C	Completion	Ctatio	0 + <u>-</u>	PID Reading (ppm)	ion (")	et)	Sampl	e ø				
Æ	5	Static Water	Moisture Content	ead om)	Penetration (blows/6")	Depth (feet)	ery	Soil Type	LIT	THOLOGY	/ DESCRIPTION	
Backfill	Casing	Level	Moi	ద్	ene	ebtl	Recovery	Soil				
m	Ö			<u> </u>	1.0		~ -	=				
***	╣				1	-		AF_	Asphalt ~4"			
***	∛		damp			1 —			Sandy Lean CLAY	medium br	rown; 30-40% fine to coarse	
₩	∛ ⊢		damp			-		\dashv $$	grained sand; soft; lo			
₩	∄ ─┤				'	2			gramou ouria, cort, re	W plasticity		
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****	₹ 7				hand		_					
₩	8 —				<u>0</u>	4						
₩	3 7					[]			(same as abov	ve, orangis	h brown; trace gravels)	
Ħ	₹					3						
Grout					₩	6-						
****	₿				İ	7 —		CL	Sandy Lean CLAY			
₩	፟					' _					ned sand; 15-20% rounded	
₩	×					8			gravels up to ~4 mm	in length		
₩	₿					_						
₩	×					9						
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₩	፠			0.1		10						
****	8 −			0.1			-					
₩	፠					11			(same as abo	ve trace c	oarse grained sand)	
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	Project N		SJ42-26		Client				Page 2 of 2		
	Logged E	sy:		Buckingham	Locati		4226 First Street, P		raye z oi z		
Delta	Driller:		Gregg			Orilled:	6/10/2005	Location Map			
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	Camping	g Method:	GeoProb	e	Hole Depth: Well Diameter:		37 ft	Please see site map			
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Consultants, Inc	Slot Size	:			Well [•			•		
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Casing Mate	# ≥ 0	PID Reading (ppm)	l a a	å	Recovery	S			• •		
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 				28			angular gravels				

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₩ 7				32—			Well-graded GRAVE	EL with Sa	and: orange brown; ~40%		
****				32		GW	coarse grained sand;	; 55-60% s	sub-rounded gravels; trace		
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***				33		GW			lay: orange brown; 20-30%		
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Delta Environmental Consultants, Inc. Well Completion Static Water	Casing Ty Slot Size: Gravel Pa	y: ethod; Method: ype;	SJ42-26I AP Gregg HSA/AK SS sch 40 P 0.01 #2/12 sa	Client: Location: Date Drilled: Hole Diameter Hole Depth: Well Diameter Well Depth: Casing Sticku Northing			4226 First Street 8/23/2006 T: 12" 108' T: 4" 108' 1p: - Easting	8/23/2006 Location Map 12" 108' Please see site map 4" 108' -		
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Delta Environmental Consultants, Inc. Well Completion Static Water	Project No Logged By Driller: Drilling Me Sampling I Casing Tyr Slot Size: Gravel Pace	r: // ethod: ! Method: ! pe: !	SJ42-26 AP Gregg HSA/AK SS sch 40 F 0.01 #2/12 sa	(7') PVC	Hole Depth: Well Diameter: Well Depth: Casing Stickup Northing Sample			er:	Shell Oil Products 4226 First Street 8/23/2006 12" 108' 4" 108' - Easting			
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			Project N		SJ42-26		Client		Shell Oil Products	US	Well No: MW-1B			
	_		Logged E	Зу:	AP			Locat		4226 First Street	<u> </u>	Page 3 of 6		
De		0	Driller: Drilling M	lothod:	Gregg	(7!)			Drilled:	8/23/2006 r: 12"	Location Map			
		la			HSA/AK SS	(7)	Hole Diameter: Hole Depth:				Dioneo co	e site map		
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1			Casing T Slot Size		0.01			Diametei Depth:	108'					
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		DAYEROK ZUMAN	Project N	lo:	SJ42-26	F-1		Clien	t:	** ***	Shell Oil Products	US	Well No: MW-1B			
			Logged E	Зу:	AP			Loca			4226 First Street		Page 4 of 6			
De	- 14		Driller:		Gregg				Drilled:		8/23/2006	Location Map				
	311	d	Drilling N		HSA/AK				er:	12"						
ì			1	g Method:	SS	Hole Depth:			108'	8' Please see site map						
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Delta Environment Consultants, I	Log Dril Dril Sar cal Cas nc. Slo	Project No: SJ42-26 Logged By: AP Driller: Gregg Drilling Method: HSA/AK Sampling Method: SS Casing Type: sch 40 F Slot Size: 0.01 Gravel Pack: #2/12 sc			Location: Date Drilled: Hole Diameter Hole Depth: PVC Well Diameter Well Depth:				r:	Shell Oil Products 4226 First Street 8/23/2006 12" 108' 4" 108' - Easting	Well No: MW-1B Page 5 of 6 Location Map Please see site map		
l ≣ E N	tatic light /ater light evel M	Content	PID Reading (ppm)	Penetration (blows/6")	Depth (feet)	Recovery S	Interval	Soil Type	LITHOLOGY / DESCRIPTION				
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Sand Belleville		dry	11.9	13 16 20 11 11 16	93 — 94 — 95 — 96 — 97 — 98 —		1	sc	Clay	(20-30% very	Gravel: bro	own, dense, 10-20% fines, ter, 60-70% medium to	

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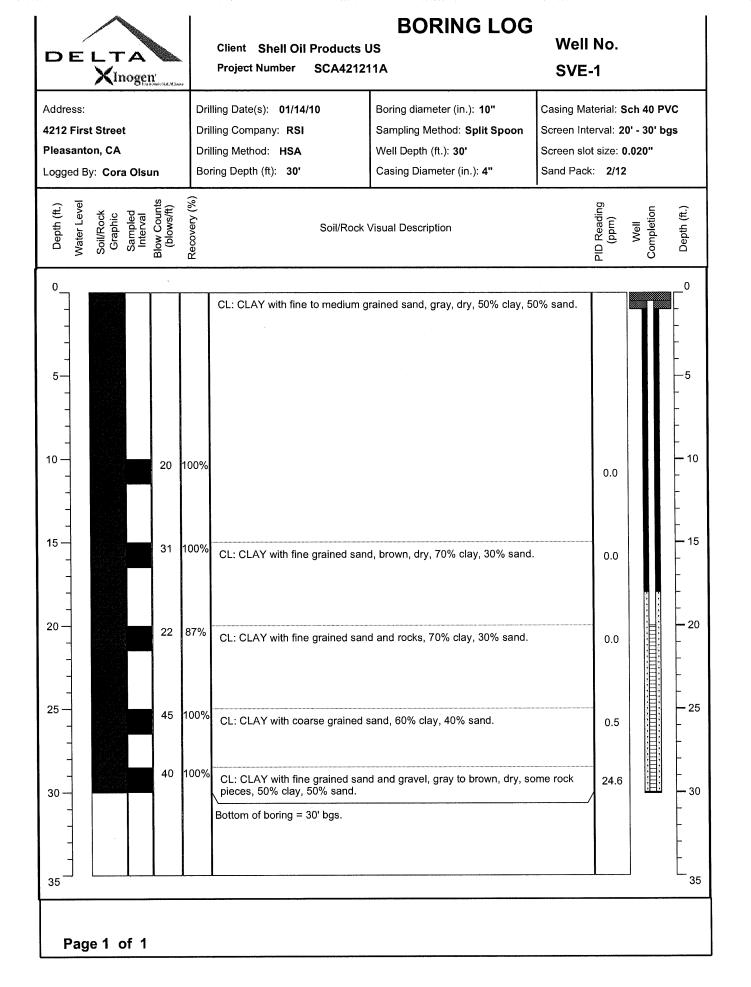
Well Completion Static Water Level	Casing Type: Inc. Slot Size: Gravel Pack: Elevation Static Vater Casing Type: Character Casing Type:		F-1 (7')	Date Hole Wel We	ation: e Drilled: e Diamete e Depth: I Diamete I Depth; ing Stick	108' Please see site map 108' 108'			
B O Sand	wet	0.7	Penetration ("lolows/6")	101— 102— 103— 104— 105— 106— 107— 108— 110— 111— 112— 113— 114— 115— 116— 117— 118— 119— 120		SC	10-20% grave	40-60% fine to coarse graines sands, is up to 1" diameter)	

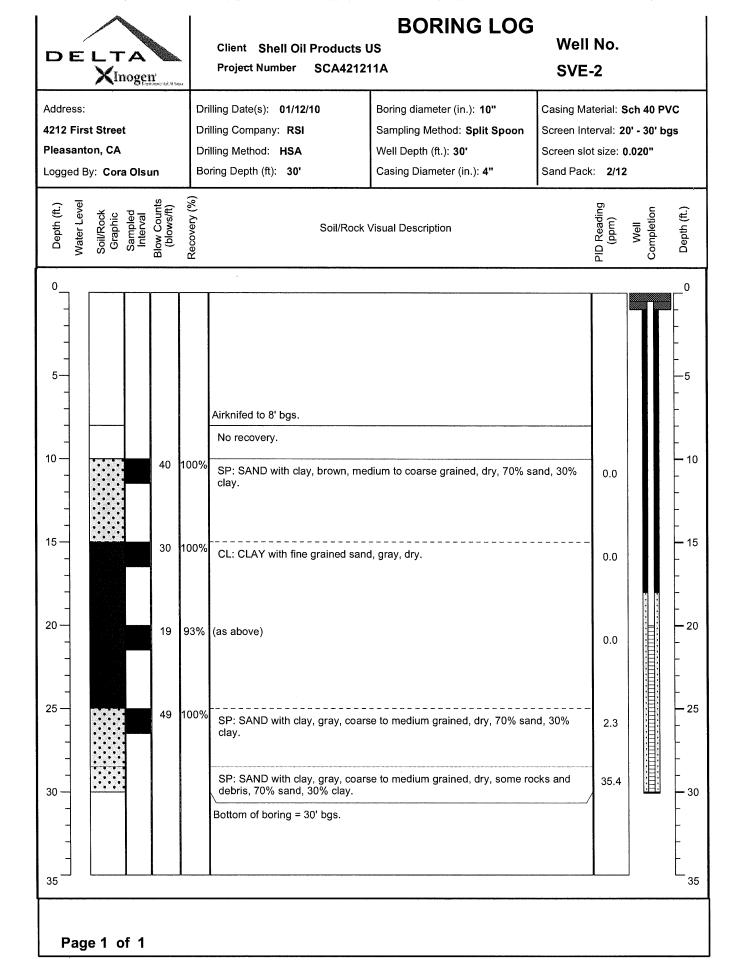
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			Logged B		AP	Location:				4226 First Street		Page 1 of 3	
	8 (4	Driller:	,	Gregg			Date I	Drilled:	8/24/2006	Location Map		
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Delta			Sampling		SS		Hole Depth:			50'	Please se	ee site map	
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Cons				:			Well I	Depth:	47'				
				ack;	#2/12 sand				ng Sticku				
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Well Cor	npietion	Static	e te	ading (c	Penetration (blows/6")	Depth (feet)	Sample <u>o</u>						
<u></u>	P	Water	Moisture Content	Rea	Penetration (blows/6")	Ę	Ven	۱۷a	Soil Type	LIT	THOLOGY	/ DESCRIPTION	
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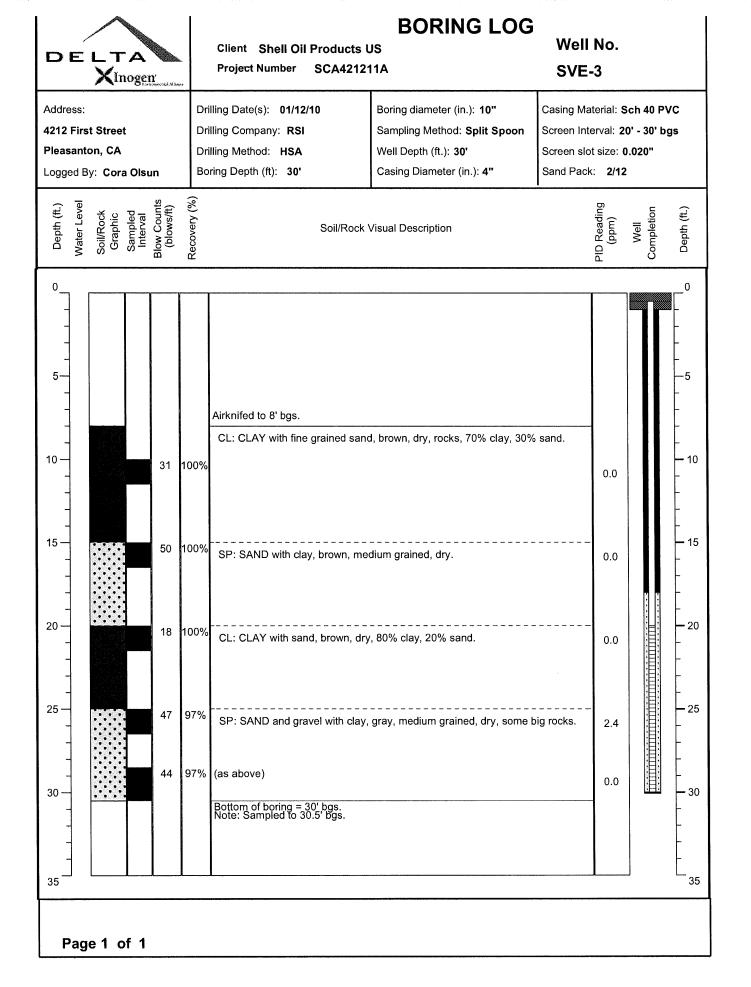
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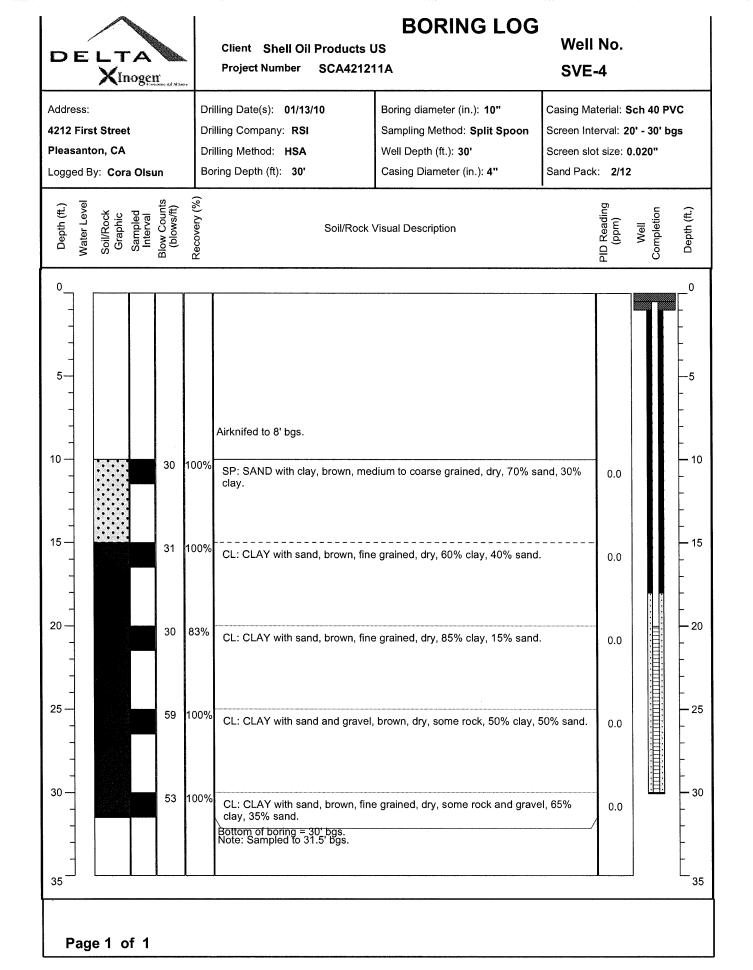
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Project N				F-1		Clien		Shell Oil Products	US	Well No: MW-4		
RANCEIDA			Logged E Driller:	зу:	AP Cross			Loca	tion: Drilled:	4226 First Street 8/24/2006	Page 2 of 3	
		10	Drilling M	lothod:	Gregg HSA/AK	(7')			Dillied:		Location Map	
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I		s, Inc.	Slot Size		0.01	- V C		-	Diamete Depth:	47'		
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	<u> </u>				20	40		▼		fines, low plas	sucity)	

Environmental Consultants, Inc. Well Completion Completion Water Level	Project N Logged E Driller:_ Drilling M Sampling Casing T Slot Size Gravel P	By: Method: g Method: Type:	SJ42-26i AP Gregg HSA/AK SS sch 40 P 0.01 #2/12 sa	(7') VC	Client: Location: Date Drilled: Hole Diameter Hole Depth: Well Diameter Well Depth: Casing Stickul Northing			4226 First Street 8/24/2006 r: 12" 50' r: 4" 47' p: - Easting	8/24/2006 12" 50' 4" 47' - Easting LITHOLOGY / DESCRIPTION		
Sand	wet	27	14 17 24 11 17 20	41 42 43 44 45 46 47 48 50 51 52 53 54 55 56 57 58 59 60			CL	grained sands	ng, 10-20% , 50-70% fi angish bro 5% fines, lo	gravels, 20-30% fine ines wn, hard, 35-45% fine ow plasticity	









BORING LOG Well No. Client Shell Oil Products US **Project Number** SCA421211A **AS-10** XInogen' Address: Drilling Date(s): 01/14/10 Boring diameter (in.): 8" Casing Material: Sch 40 PVC 4212 First Street Drilling Company: RSI Sampling Method: Split Spoon Screen Interval: 47' - 52' bgs Pleasanton, CA Drilling Method: HSA Well Depth (ft.): 52' Screen slot size: 0.020" Boring Depth (ft): 52' Casing Diameter (in.): 2" Sand Pack: 2/12 Logged By: Cora Olsun PID Reading (ppm) Blow Counts (blows/ft) Nater Level Depth (ft.) Soil/Rock Graphic Sampled Interval Completion Depth (ft.) Soil/Rock Visual Description 0 Airknifed to 8' bgs. No recovery. 10 20 100% 0.0 CL: CLAY with fine grained sand, brown, dry. 15 15 28 97% 0.0 CL: CLAY with fine grained sand, gray with red staining, dry, 70% clay, 30% sand. 20 20 16 100% 0.0 CL: CLAY with fine grained sand, brown, dry, 70% clay, 30% sand. 25 25 15 100% CL: CLAY with fine grained sand, brown, dry, 70% clay, 30% sand. 1.4 30 30 36 100% CL: CLAY with fine grained sand and rocks, gray to brown, dry, 80% clay, 5.4 20% sand. 35 35 46 100% 15000 CL: CLAY with fine grained sand, gray, dry, odor, 70% clay, 30% sand. 40 40 56 43% 15000 CL: CLAY with fine grained sand, moist, odor, rock pieces. 45 45 52 87% 40.9 CL: CLAY with fine to medium grained sand, wet, some rocks. 50 50 47 100% (as above) 142 Bottom of boring = 52' bgs. 55 55

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BORING LOG Well No. Client Shell Oil Products US **Project Number** SCA421211D OBS-1 XInogen' Address: Drilling Date(s): 01/13/10 Boring diameter (in.): 10" Casing Material: Sch 40 PVC **4212 First Street** Drilling Company: RSI Sampling Method: Split Spoon Screen Interval: 22' - 47' bgs Pleasanton, CA Drilling Method: HSA Well Depth (ft.): 47' Screen slot size: 0.020" Logged By: Cora Olsun Boring Depth (ft): 47' Casing Diameter (in.): 4" Sand Pack: 2/12 Soil/Rock Graphic Sampled Interval Blow Counts (blows/ft) PID Reading (ppm) Recovery (%) Depth (ft.) Completion Depth (ft.) Well Soil/Rock Visual Description 5 Airknifed to 8' bgs. CL: CLAY with coarse to medium grained sand, gray, dry, 50% clay, 50% 10 10 100% 25 0.0 15 15 35 100% CL: CLAY with medium grained sand, brown, dry, 75% clay, 25% sand. 0.0 20 20 18 97% CL: CLAY, brown to gray, trace fine grained sand, dry. 0.5 25 25 47 100% SP: SAND with clay, brown, medium to coarse grained, dry, some rocks, 0.0 80% sand, 20% clay. 30 30 36 70% 7.3 SP: SAND with clay, brown, medium to coarse grained, dry, some rocks, 60% sand, 40% clay. 35 - 35 59 53% 342 CL: CLAY with coarse to medium grained sand, brown, dry, 60% clay, 40% sand. 40 40 58 80% 86.1 CL: CLAY, brown, trace fine grained sand and gravel, dry. 45 45 87% 58 CL: CLAY with gravel, brown, trace sand, dry. 0.2 Bottom of boring = 30' bgs. 50 50

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