A Report Prepared for

Texaco Refining and Marketing Inc. 100 Cutting Boulevard Richmond, California 94804

GROUND-WATER REMEDIATION PLAN
FORMER TEXACO STATION NO. 62488000195
2225 TELEGRAPH AVENUE
OAKLAND, CALIFORNIA

HLA Job No. 2251,111.03

by

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November 30, 1989

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#### I INTRODUCTION

This report describes a plan developed by Harding Lawson Associates (HLA), for treatment of ground water containing gasoline constituents at a service station site formerly owned by Texaco Refining and Marketing Inc. As shown on Plate 1, the site is at 2225 Telegraph Avenue in Oakland, California. The service station is currently owned and operated by Exxon Company U.S.A.

HLA previously conducted an investigation at this site that defined a dissolved gasoline plume in the ground water with concentrations of chemical compounds exceeding action levels established by the California Department of Health Services (DOHS). The purpose of this work plan is to outline a remediation program that will remediate the dissolved gasoline plume on-site while maintaining hydraulic control of the ground water in the site vicinity. Specifically, HLA proposes to: extract ground water using three on-site recovery wells, treat the extracted water through contact with granular-activated carbon (GAC), and discharge the clean, treated water into the Oakland sanitary sewer system.

#### II OBJECTIVES OF REMEDIATION

## A. <u>Previous Investigation</u>

In May 1988, Texaco Refining and Marketing Inc. retained HLA to conduct a site investigation of a service station at 2225
Telegraph Avenue in Oakland, California. To date, the investigation has been performed in three sequential phases, results of which were presented in reports issued on the following dates:

- Sensitive Receptor Study May 24, 1988
- Subsurface Investigation July 20, 1988
- Environmental Assessment ' June 22, 1989

To update project status, HLA issues quarterly technical reports to Texaco Refining and Marketing Inc. that describe continuing activities and present findings.

#### B. <u>Summary of Findings and Need for Remediation</u>

The results of HLA's investigation indicate that remedial actions are necessary to address the following conditions at this site:

1. Shallow ground water beneath the site contains dissolved benzene, toluene, ethylbenzene, and xylenes (BTEX) in excess of Drinking Water Action Levels (DWALs) and Maximum Contaminant Levels (MCLs).\* The lateral extent of BTEX in the ground water is not known at this time because of restricted access on Telegraph and West Grand Avenues imposed by the City of Oakland.

<sup>\*</sup> The DOHS issued a revised action list for chemical contaminants of drinking water in a letter dated April 19, 1989 (Appendix A). MCLs are drinking water standards enforced by law under California Code of Regulations, Title 22. DWALs are recommended levels but are not enforced by law.

Subsurface exploration north of the site, on West Grand Avenue, was also restricted because of the existing Bay Area Rapid Transit (BART) tunnel. Distribution of total petroleum hydrocarbons (TPH) as gasoline in the ground water is presented on Plate 2.

2. BTEX and TPH (as gasoline) concentrations in excess of 100 parts per million (ppm) have been detected exclusively in soils at 12 to 13.5 feet below the ground surface. This interval is within the zone of fluctuation of the ground-water table.

A more detailed discussion on site conditions is presented in a subsequent section of this report.

## C. Objective of Proposed Work

HLA proposes to remediate the ground water on site by extracting from three on-site wells, treating the water by passing it through granulated carbon and discharging to the sanitary sewer. The clean-up goals for this remediation will be to continue ground water remediation until ground water samples from monitoring wells have concentrations of BTEX at or less than the existing DWALs or MCLs. We believe these levels to be adequate because the primary exposure pathway of these chemicals is through ingestion of ground water. As explained in our Sensitive Receptor Study, there are no registered drinking water wells within 1/4 mile of the site, and therefore HLA believes that human health is protected. We also believe that the DWAL and MCL are reasonable clean-up levels that can be achieved with available technologies.

Because significant concentrations of BTEX and TPH have only been detected in the soils below 12 feet and the water table is

12 to 14 feet deep, we believe that no remediation of the vadosezone soil is needed.

If an off-site upgradient source of petroleum hydrocarbons is identified during the remediation, changes in the clean-up goals for individual monitoring wells may be requested.

#### III SCOPE OF WORK

The work will be approached as five tasks:

- Task 1 Drill and install one 25-foot-deep, 4-inch-diameter recovery well and modify well heads of MW-6C and MW-6D
- Task 2 Construct a ground-water collection system from three recovery wells to the treatment facility
- Task 3 Install pumps in three recovery wells
- Task 4 Install water treatment system
- Task 5 Start-up and monitor the water treatment system.

#### A. Task 1 - Drill and Install Recovery Well

A recovery well (RW-6A) will be installed, as shown on Plate 3. Existing monitoring wells MW-6C and MW-6D will also be converted into recovery wells.

The well will be drilled with 10-1/2-inch augers to a depth of 25 feet, and constructed of 4-inch-diameter PVC casing (see Well Construction Details, Plate 4). The well screen (0.02-inch slot size) will extend from approximately 10 to 25 feet below grade. The annular space between the casing and the borehole wall will be filled with No. 3 Monterey sand to approximately one foot above the top of the screened interval. A 2-foot-thick bentonite seal will be placed above the sand pack, and the remainder of the annulus will be filled with a cement-bentonite grout to just below the ground surface.

The top of RW-6A will be slightly below the ground surface and equipped to attach to the collection system piping. A lock-ing, 2-foot by 3-foot traffic box will be installed over the well

to house well-head attachments. The existing traffic boxes protecting MW-6C and MW-6D will be replaced with 2-foot by 3-foot rectangular traffic boxes below grade to accommodate well head equipment associated with pneumatic pumping systems.

## B. <u>Task 2 - Construct Ground-water Collection System</u>

A ground-water collection system, in the configuration shown on Plate 3, will be constructed in a 2-foot-deep trench to connect the recovery wells with the treatment compound. A 4-inch-diameter, flush-threaded, PVC pipe will extend from the recovery wells to the treatment system compound. The pipe will contain two pneumatic control lines (a 1/2-inch inside diameter [ID] air source line and a 1/4-inch-ID pilot line), and a 1/2-inch-ID line to transport extracted ground water from the recovery wells to the treatment system.

The bottom 2 inches of the trench, which will contain the pipe, will be backfilled with sand. Then the pipe will be laid in the trench and sand will be placed to 6 inches above the 4-inch pipe and compacted by tamping with vibratory equipment. The trench will then be backfilled with Class II aggregate baserock and compacted to a minimum of 95 percent relative compaction\* to within 2 inches of final grade. A 2-inch asphalt

<sup>\*</sup> Relative compaction refers to the in-place dry density of soil expressed as a percentage of the maximum dry density of the same soil determined by ASTM D1557-78 laboratory test procedure. Optimum moisture is the water content that corresponds to the maximum dry density.

layer will than be placed and compacted in the trench to match the existing grade.

#### C. <u>Task 3 - Install Pumps</u>

Pneumatic injector pumps will be installed inside the three recovery wells (RW-6A, MW-6C, and MW-6D) to facilitate ground-water extraction. Schematic representation of the ground-water extraction system is presented on Plate 5. An air compressor and control panel will be installed in the treatment system compound.

Each well will be equipped with a pneumatic control device that will stop the flow of compressed air to the pump when the water level in the well drops below the pump inlet. The water discharge line at each well head will have a block valve (ball type) and sampling port.

#### D. <u>Task 4 - Install Water Treatment System</u>

Extracted ground water will be processed through a water treatment system consisting of a retention tank, charge pump, and GAC canisters as shown in the schematic representation presented on Plate 6. The ground-water collection system will be connected to a retention tank. A centrifugal pump will be used to move water collected in the retention tank through GAC canisters for treatment prior to discharge to the sanitary sewer. The operation of the pump will be controlled by a high and low level switch in the retention tank; high level will start the charge pump, low level will stop the charge pump, and high-high level (to prevent overflow) will stop the well extraction pumps. Three

GAC canisters will be connected in series with a 1-inch-diameter PVC manifold to redistribute flow. Sampling ports will be installed in the influent line, effluent line, and between each canister. A flow meter totalizer will be installed on the effluent line at a point before the line discharges to the sanitary sewer.

The above treatment system will be installed on an 8-foot by 16-foot reinforced concrete pad enclosed by a fence at the location shown on Plate 3. A low retainer wall will be installed around the perimeter of the pad to provide secondary containment for liquids.

A licensed contractor will install flush-threaded cast iron pipe, which will discharge treated ground water from the treatment system to an on-site sanitary sewer connection after we receive the East Bay Municipal Utilities District (EBMUD) discharge permit. Electrical work will be performed by a licensed contractor. HLA will provide construction management of the installation.

## E. Task 5 - Start-up and Monitor Water Treatment System

We expect that the construction operations described above will take approximately 2 to 3 weeks; a 2- to 3-day full or partial closure of the station services will be necessary to install trench and piping between wells RW-6A and MW-6D. The balance of the work can be done without any station closure. After the ground-water extraction and treatment systems are

installed, HLA will provide testing of the remediation system over a three-week period. During this period, discharge water will be retained in a storage tank on site. After the test, it will be pumped to the sanitary sewer.

Water samples will be collected, as required by an EBMUD ground-water discharge permit, to assess treatment efficiency.

We propose to sample treated effluent on the following schedule:

daily for one week, weekly for the remainder of the month, so get to monthly for the remainder of the quarter, and quarterly

thereafter.

Midstream samples will be obtained weekly to monitor hydrocarbon breakthrough of the first and second canister. The first canister will be removed once breakthrough of the second canister occurs; manifold valves will be manipulated to allow a clean GAC canister to be installed in series, following the remaining two canisters. Monitoring of the second canister in the series will continue on the basis of initial breakthrough times and EBMUD requirements.

We will evaluate the effectiveness of site ground-water remediation quarterly, by sampling and analyzing water from eight on-site and one off-site monitoring wells. Status reports will be submitted quarterly.

#### F. <u>Schedule</u>

We anticipate that the proposed ground-water remediation system will be operational within three months after approval is

obtained from the appropriate regulatory agency to proceed with the work outlined in this work plan (see Anticipated Remediation Schedule, Plate 7). HLA will submit an application to EBMUD for discharging treated ground water to the sanitary sewer; we expect that the permit will require one to two months for processing. During this time, HLA will prepare design drawings and specifications and solicit licensed contractors to install the ground-water collection system, construct components of the water treatment system and compound, install electrical wiring, and connect plumbing.

#### IV SITE CONDITIONS

## A. Location and Topography

The former Texaco service station is located approximately
3.0 miles east of San Francisco Bay near the main business
district of Oakland, California (Plate 1). The surrounding area
consists of commercial/retail business, including a Chevron
service station immediately across Telegraph Avenue, and a Beacon
service station northeast (upgradient) of the site (Plate 8).

Surface elevation at the site is approximately 20 feet above mean sea level. The land surface slopes gently southeast, toward Lake Merritt and the Oakland/Alameda Inner Harbor. This area has been extensively developed, and surface-water runoff is mainly controlled by the municipal storm sewer system.

#### B. <u>Geographic Constraints</u>

The City of Oakland and BART have restricted off-site subsurface access in sidewalks and streets to the north, east, and south of the site; a church occupies the entire adjacent property to the southwest.

#### C. <u>Vadose Zone Soil Condition</u>

Because petroleum hydrocarbon concentrations in excess of 100 ppm have not been detected in the vadose zone soils (above fluctuation in the ground-water level), no soil remediation is necessary.

#### D. Ground-water Gradient

Ground water is encountered at approximately 13 feet. The estimated ground-water flow direction is predominantly southwest, with a gradient of 0.005 foot per foot. As shown on the ground-water surface map (Plate 9), in the northern part of the site, the ground-water flow direction is more toward the west.

#### E. Ground-water Condition

No free product has been observed in any of the monitoring wells. Hydrocarbons dissolved in the ground water are generally limited to the vicinity of the tanks and pump islands, extending southwest. The distribution of these hydrocarbons is presented on Plate 2.

Water from five on-site wells near the tanks and pump islands contains detectable levels of TPH. As of September 1989, the lateral limits of the plume are delineated by wells MW-6G, MW-6A, MW-6F, and MW-6I, which yield samples with non-detectable hydrocarbons (<50 parts per billion [ppb] TPH). Upgradient plume definition is incomplete because of restricted subsurface access.

Hydrocarbons may be migrating to the site from an upgradient source. In water from upgradient well MW-6B, combined concentrations of BTEX have increased from 6.6 ppb to 160 ppb (samplings of October 20, 1988, and July 11, 1989, respectively). Except for downgradient well MW-6E, water from every other monitoring well has exhibited a reduction in BTEX over the same time period.

## F. Underground Fuel Tank Integrity

Three steel underground fuel tanks, installed in 1964, have cathodic protection and contain both leaded and unleaded gasoline (no diesel service). In November 1986, a fiberglass waste oil tank was installed in place of the previously existing waste oil tank. All four tanks and associated transfer lines were certified as "tight"\* on June 30, 1988, by Paradiso Construction Company of Oakland, California (see Appendix B).

## G. Source of Hydrocarbons in Ground Water

Given the positive results in recent tank and line integrity tests, and the limited extent of hydrocarbons in ground water, the most likely source of hydrocarbons is from past tank overfill.

Tank systems certified as "tight" meet the criteria established by the National Fire Protection Association Pamphlet 329.

#### V GROUND-WATER TREATMENT SYSTEM

#### A. <u>Design</u>

Design parameters for the water treatment system are presented in Table 1. Influent concentrations are derived from an average of results from chemical analyses of ground water from the three wells that had the greatest concentration of dissolved hydrocarbon constituents (MW-6C, MW-6D, and MW-6H). Effluent concentrations are established by EBMUD, using the average concentrations in water processed during 1988. Flow rate is estimated from hydraulic calculations discussed in a subsequent section of this report.

Table 1. Water Treatment System Design Parameters

#### <u>Influent Concentrations (ug/l)</u>

	TPH			Ethyl-		
	<u>(as gasoline)</u>	<u>Benzene</u>	<u>Toluene</u>	<u>benzene</u>	<u>Xylenes</u>	<u>Lead</u>
MW-6C	18,000	7,900	430	350	1,100	<0.3
MW-6D	2,200	600	26	58	31	4
MW-6H	<u>660</u>	480	<10	<u> 16</u>	<10	<u>&lt;1</u>
Average	7,000	3,000	160	140	380	<2

#### Effluent Concentrations (uq/1)

	<u>Benzene</u>	<u>Toluene</u>	<u>Ethylbenzene</u>	<u>Xylenes</u>	Lead
EBMUD Specifications	3	31	5	42	2

#### Flow Rates

Unconfined Aquifer
Conductivity 5.0 feet/day
Average Saturated
Thickness = 4.0 feet

Transmissivity = 187 gpd/ft
Storativity (specific yield) 0.04
Q (per well) = 0.3 gpm

Expected Total Flow Rate = 0.9 gpm
Design Flow Rate = 0 to 5 gpm

The proposed treatment system consists of a retention tank, a charge pump, and three GAC canisters connected in series to maximize carbon utilization efficiency (see Plate 6). A 1-inch-diameter PVC manifold will be installed to redistribute flow between the GAC canisters as they are replaced.

The retention tank will serve three purposes:

- 1. Provide a constant head source for the charge pump to draw from, thus maintaining steady operation
- Prevent air in the ground-water collection system from entering the GAC media
- 3. Retain any free product that enters the collection system, which would be skimmed from the tank to prevent fouling of GAC media.

A triple function fluid level control switch will be installed in the retention tank. At high level, the switch will start the charge pump to move water from the retention tank through the GAC media; at low level, the switch will stop the charge pump; and at its overflow level, the switch will shut off the extraction pumps in the recovery wells to prevent tank overflow in the event of a system failure.

#### B. <u>Feasibility</u>

Treatment with GAC media was decided upon after considering air stripping as a remediation technique for this site. Several factors make air stripping less cost effective than the proposed system: low flow rates, mechanical maintenance costs, permitting requirements imposed by the Bay Area Air Quality Management

District, and anticipated monitoring requirements of effluent air.

#### VI HYDROGEOLOGY

#### A. <u>Near-surface Geologic Profile</u>

Seven soil borings (B-1 through B-7) and nine monitoring wells have been drilled on and off site in the vicinity of the subject station. Subsurface materials generally consist of stiff, silty clay (10 to 15 feet thick), underlain by a dense silty sand layer (3 to 8 feet thick). The sand layer is underlain by more silty clay or silt. A shallow subsurface cross-section is presented on Plate 10.

#### B. Aquifer Testing

Hydraulic conductivity was estimated from the results of three single-well slug tests, using monitoring wells MW-6H, MW-6D, and MW-6E, which are generally downgradient of the underground fuel tanks. Water levels were compared with stratigraphic logs of the wells to classify the most permeable stratum adjacent to the screen in the saturated zone as hydraulically confined or unconfined. Table 2 lists the hydraulic conductivity estimates derived from the tests.

Table 2. Slug Test Results

				Estimated Hydraulic
			Thickness	Conductivity
Well	Lithology of Most	Classification	of Stratum	of Stratum
Number	Permeable Stratum	of Stratum	<u>(feet)</u>	(feet/day)
MW-6D	sand	confined	2	5.9
MW-6E	sand, fine-grained	confined	2.5	1.2
мм-6н	sand, medium-grained	unconfined	6	4.8

#### C. Flow Rates

Initially, the expected flow rate from the three recovery wells is 0.3 gallons per minute (gpm) per well, using the parameters presented in Table 1. As ground-water extraction continues, flow rates are expected to decrease.

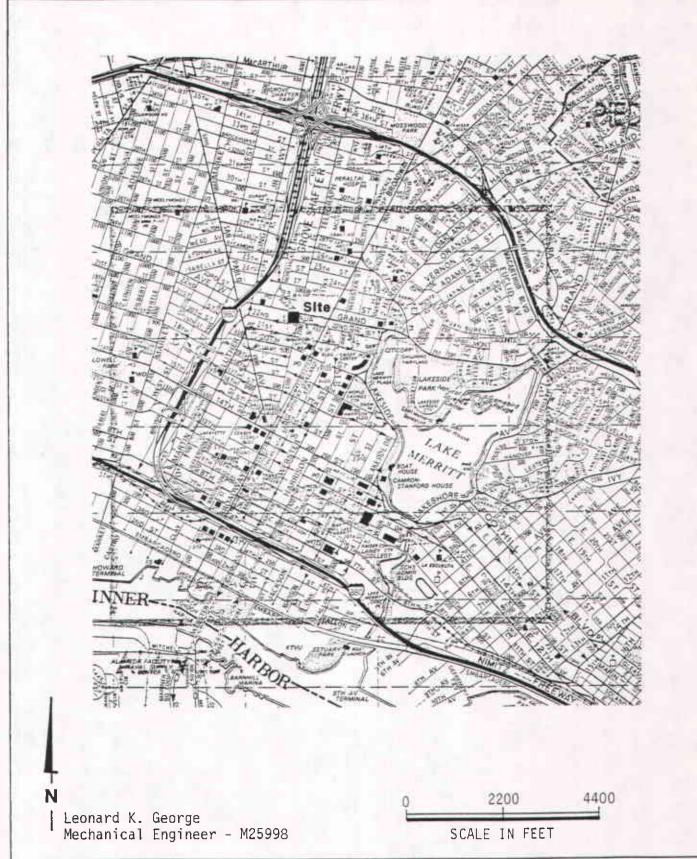
## D. <u>Capture Zone</u>

As shown on Plate 11, cones of depression about each well, with radii of approximately 50 feet, are expected to develop after two months of operation. These cones will provide an imposed gradient toward the wells of about 0.05 foot per foot, which is 10 times greater than the natural hydraulic gradient across the site. In these calculations, we used the flow rate parameters presented in Table 1. The combined cones of depression (capture zone) of RW-6A, MW-6C, and MW-6D should provide hydraulic control over the site to recover the dissolved hydrocarbon plume as it is currently defined.

#### E. <u>Feasibility</u>

Three recovery wells were compared with the installation of a recovery trench. Capture zone calculations indicate that the proposed recovery wells will eventually dewater the low conductivity aquifer at this site; the additional expense to install a recovery trench would not provide increased hydraulic control.







Engineers and Geoscientists

Site Location Map

Former Texaco Service Station 2225 Telegraph Avenue

Oakland, California

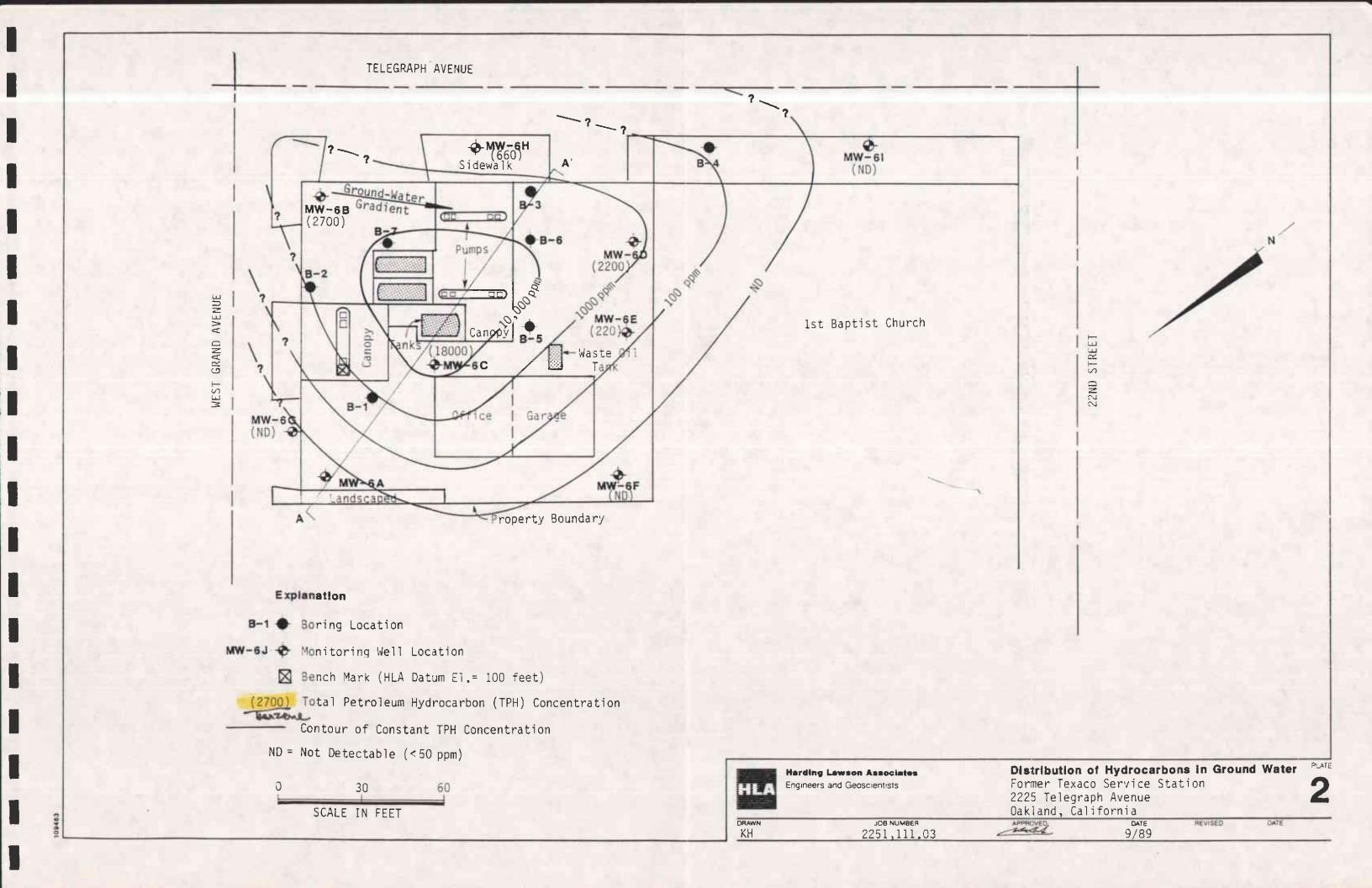
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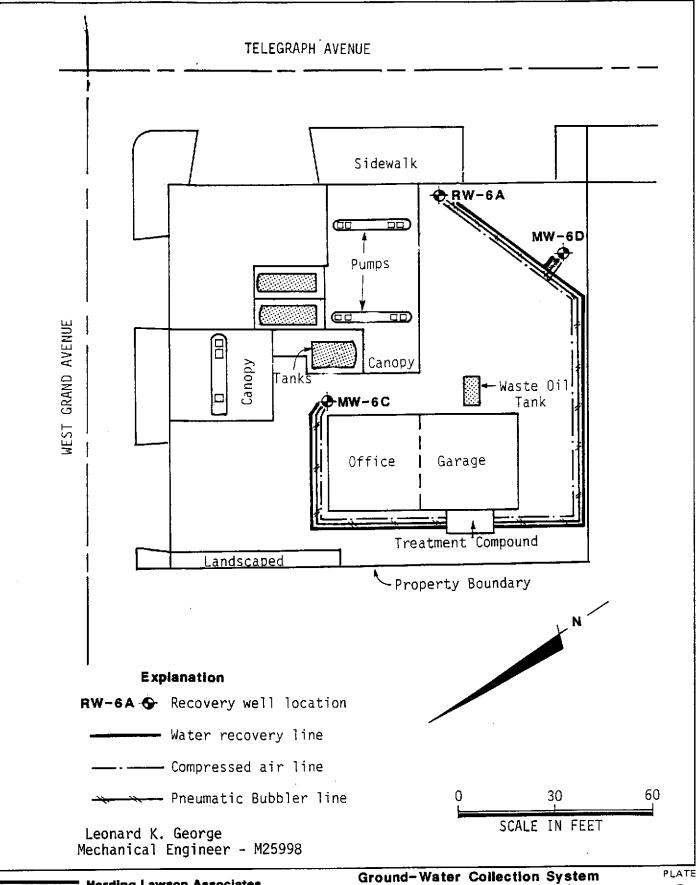
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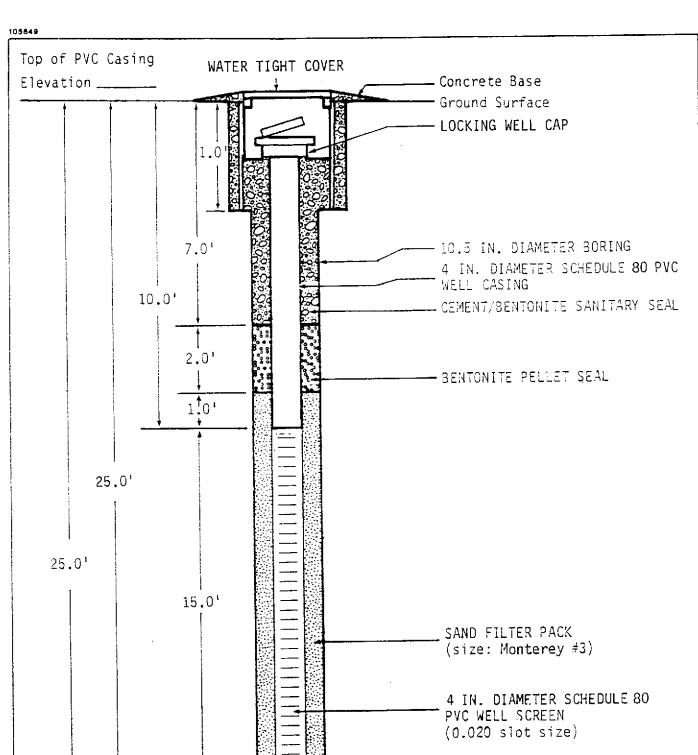
Engineering and Environmental Services Former Texaco Service Station 2225 Telegraph Avenue Oakland, California

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Harding Lawson Associates

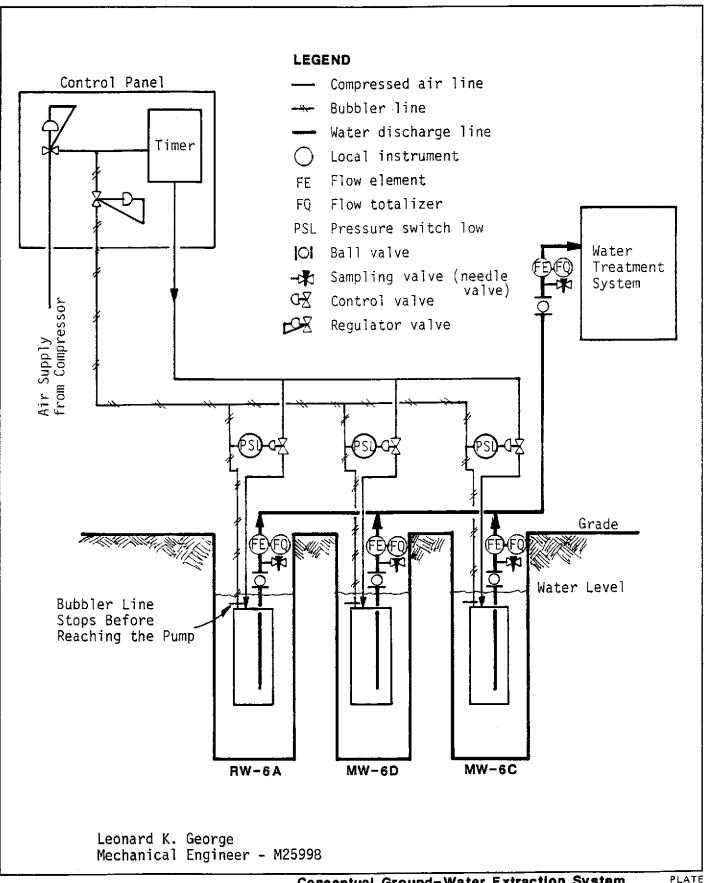
Engineers and Geoscientists

Proposed Well Construction Detail Former Texaco Service Station 2225 Telegraph Avenue

Oakland, California

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NOT TO SCALE





Engineering and **Environmental Services**  Conceptual Ground-Water Extraction System

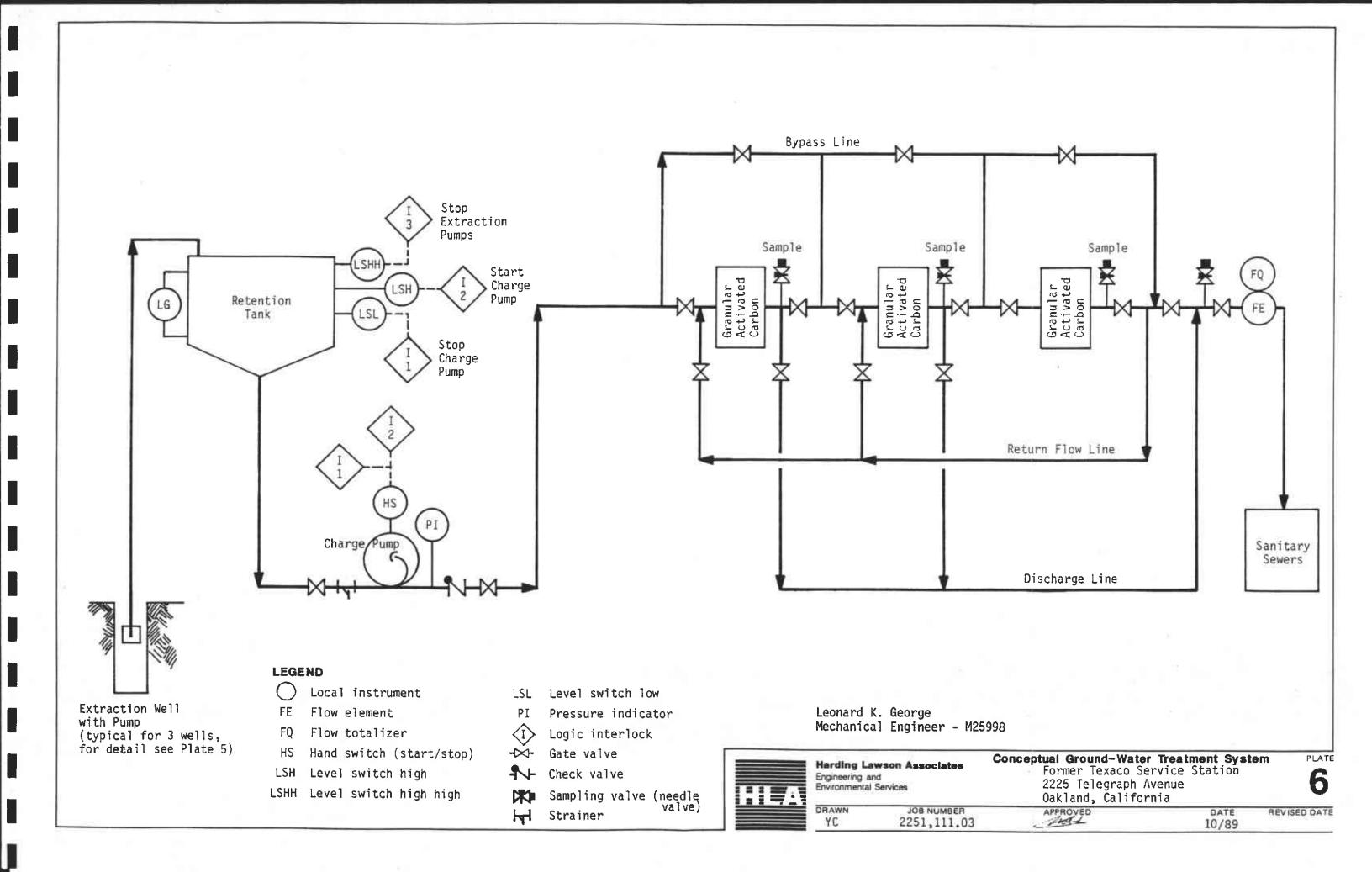
Former Texaco Service Station 2225 Telegraph Avenue

Oakland, California

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DATE 9/89

REVISED DATE





Harding Lawson

Anticipated Remediation Schedule
Former Texaco Service Station
2225 Telegraph Avenue
Oakland, California
APPROVED
APPROVED
11/89

HEVISED DATE

89 90 Nov Dec Jan Feb Mar Apr May Jun Jul Aug 2 Resources Task Name EBMVP Permit Applictn HLA Engineering/Construction HLA Design Specs HLA Contract HLA Prefabricate HLA/SUBCON Install Fndtn. HLA/SUBCON Install Trench HLA/SUBCON Install Treat. Plant HLA/SUBCON Monitor System HLA Online HLA

(Slack)

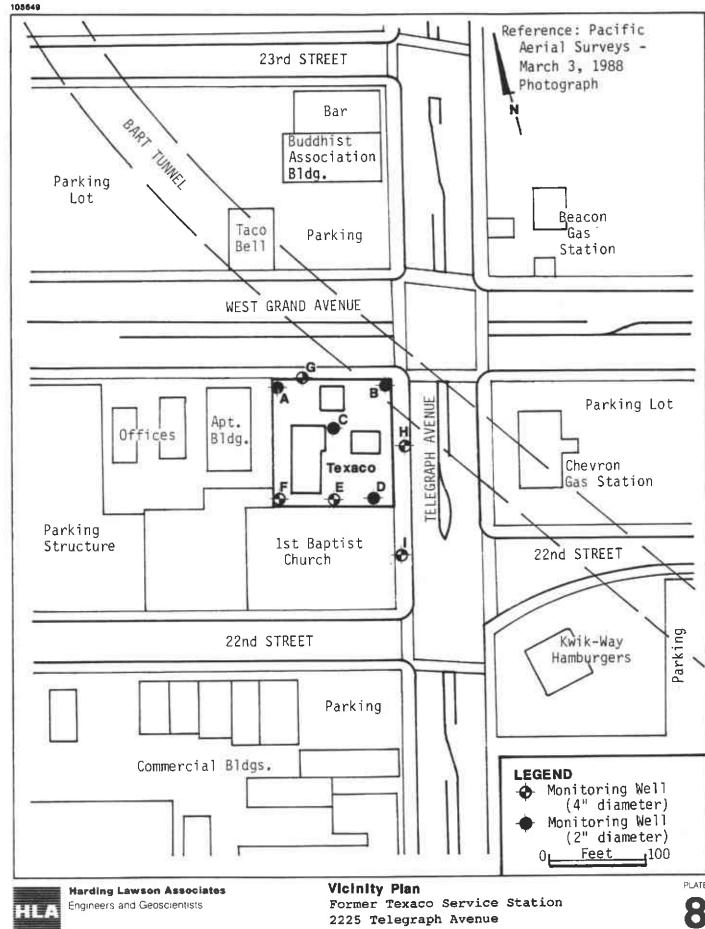
Detail Task ≡≡≡≡≡ Summary Task (Started) ==≡≡≡ (Started)

Milestone ▶▶▶ Conflict

... Resource delay

≡≡= (Slack)

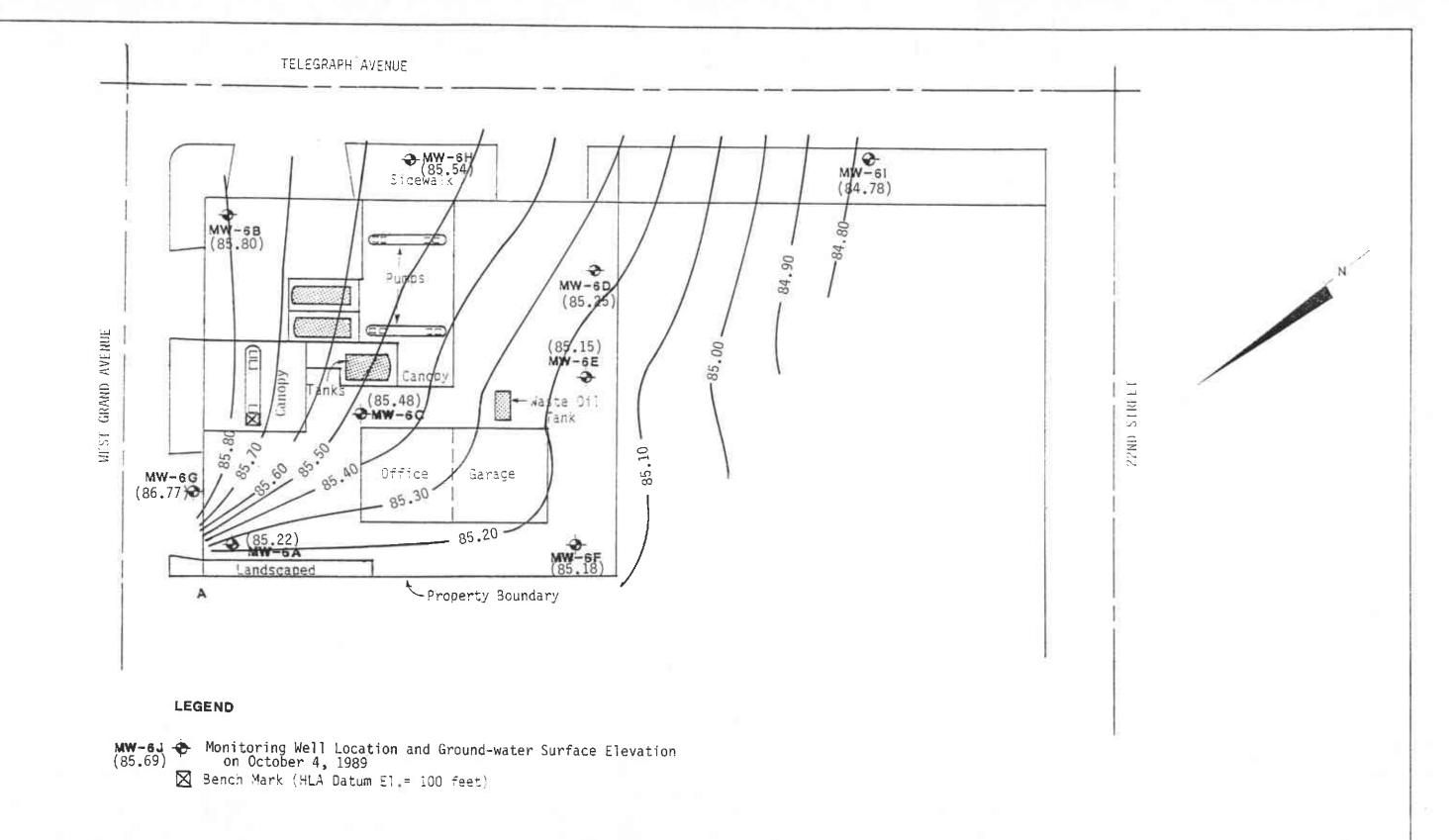
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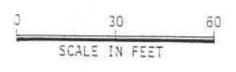


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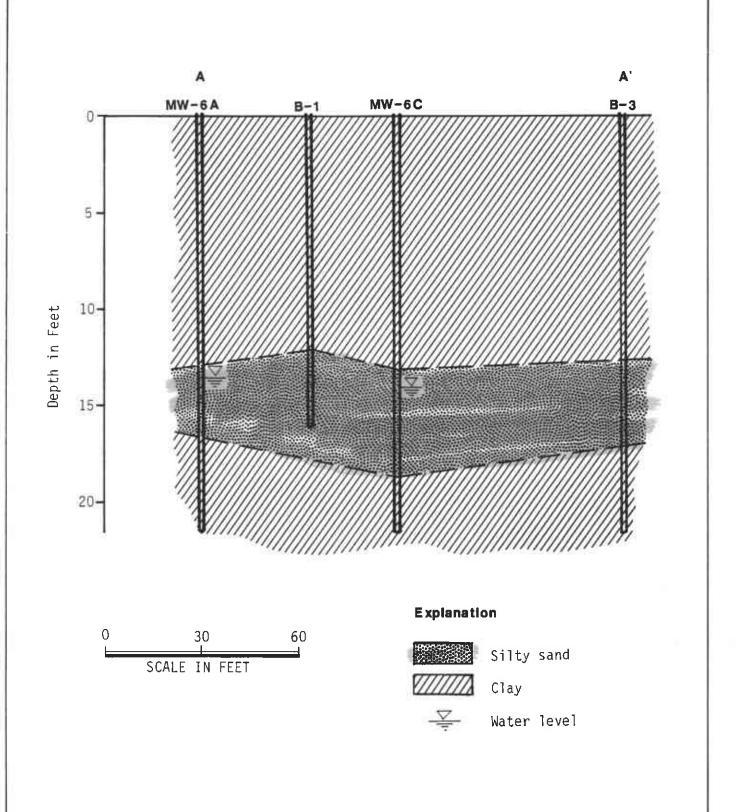
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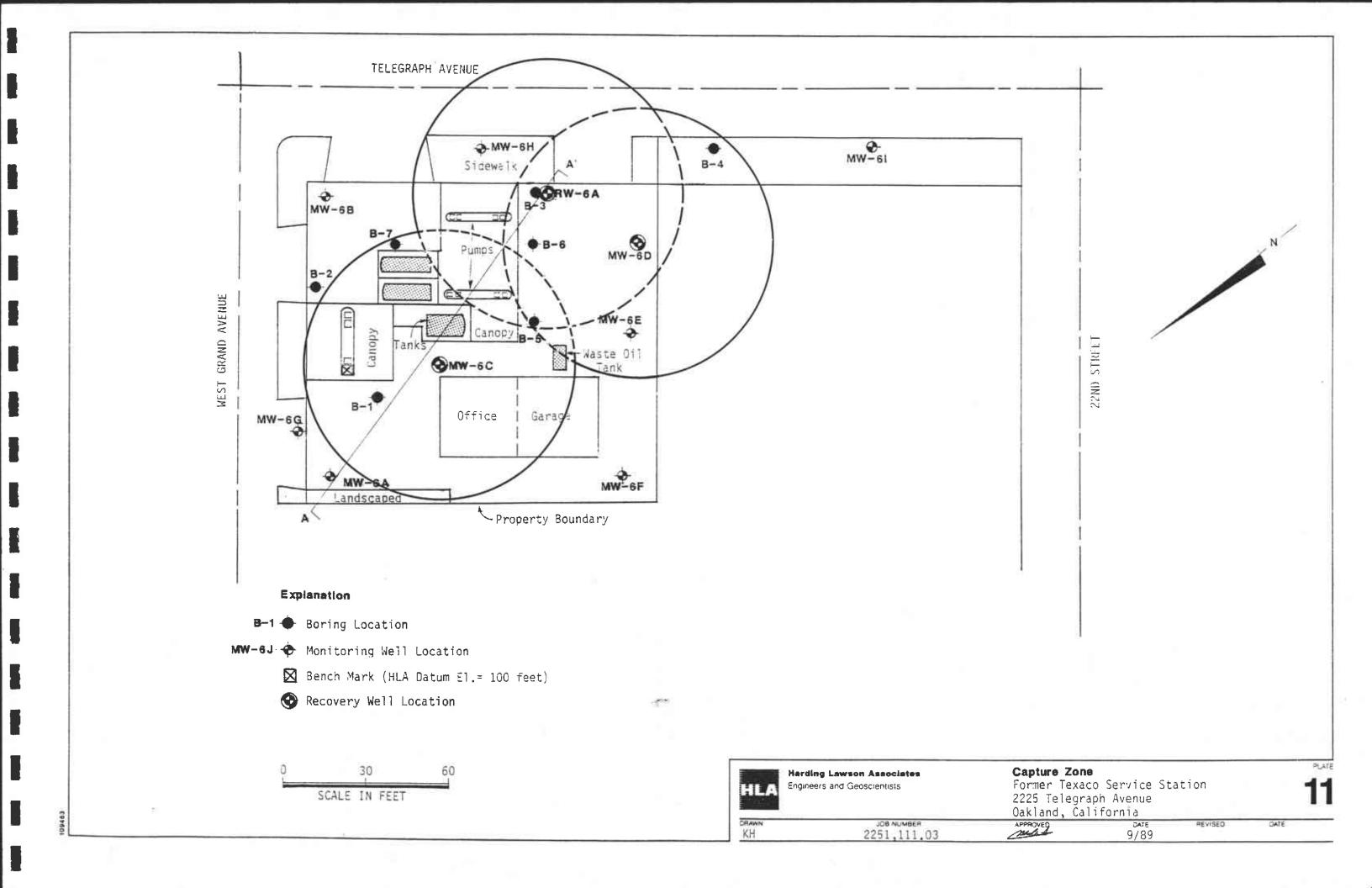
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# Shallow Subsurface Cross Section A-A'

Former Texaco Service Station 2225 Telegraph Avenue Oakland, California 10

APPROVED DATE REVISED DATE 9/89



# APPENDIX A

DOHS REVISED ACTION LIST FOR CHEMICAL CONTAMINANTS OF DRINKING WATER

GEORGE DEUKMEJIAN, Governor

APR 24 1989

April 19, 1989

DEPARTMENT OF HEALTH SERVICES

2151 BERKELEY WAY BERKELEY, CALIFORNIA 94704

TO: ALL INTERESTED PARTIES

LIST OF ACTION LEVELS FOR CONTAMINANTS OF DRINKING WATER

For your information and reference, I am forwarding a copy of this Department's most recently revised action level list for chemical contaminants of drinking water. The recently adopted maximum contaminant levels (MCLs) have been deleted. This list also reflects changes in the action levels of some of the contaminants.

A list of the recently adopted MCLs is also enclosed for your reference.

For further information concerning these lists, you may contact me at the technical staff office of the Public Water Supply Branch in Berkeley (415) 540-2172.

Sincerely,

David P. Spath, Ph.D., Chief

Chemical Standards and

Technology Unit Public Water Supply Branch

Enclosures

# State of California Department of Health Services

# Drinking Water Action Levels Recommended by the Department of Health Services

# April 1989

Chemical	parts	Action Level per billion (ppb)	
Pesticides	<u> </u>	Per danted (Ppe)	
Chlorinated Hydrocarbon			
Aldrin	Limit	of Quantification	(0.05)
a-Benzene Hexachloride (a-BHC)	0.7	2	,
b-Benzene Hexachloride (b-BHC)	0.3		
Chlordane	0.1		
Dieldrin		of Quantification	(0 05)
Heptachlor	0.01	or guarithmeteron	(0.05)
Heptachlor Epoxide	0.01		
Pentachlorophenol	30.0		
i en caomiot oblienot	30.0		
Organophosphate			
Dimethoate	140.0		
Diazinon	14.0		
Ethion	35.0		
Malathion	160.0		
Methyl Parathion	30.0		
Parathion	30.0		
Trithion	7.0		
Carbamate			
Aldicarb	10.0		
Baygon	90.0		
Carbaryl	60.0		
•			
Phthalamide			
Captan	350.0		
Amides			
Diphenamide	40.0		
Fumigants			
Dibromochloropropane	1.0		
1,2-Dichloropropane	5.0		
Chloropicrin	50.0	(37.0)*	
CHTOTODICTIL	50.0	(37.0)=	
Miscellaneous			
Terrachlor	0.9		
(Pentachloronitrobenzene)	J. J		
( = encapitabloitabletisetie)			

Herbicides CIPC 350.0 (isopropyl N (3-chlorophenyl) carbamate) 700.0 Alachlor Limit of Quantification (0.2) Purgeable Halocarbons Methylene Chloride 40.0 Tetrachloroethylene 5.0 1,1-Dichloroethane 5.0 Trichlorofluoromethane 150.0 (Freon 11) 1,1,2-Trichloro-1,2,2-tri-1,200.0 fluoroethane (Freon 113) Cis-1,2-Dichloroethylene 6.0 Trans-1,2-Dichloroethylene 10.0 Purgeable Aromatics 1,2-Dichlorobenzene 130.0 (10)\* 1,3-Dichlorobenzene 130.0 (20)\*(Action Level for 1,2-Dichlorobenzene and 1,3-Dichlorobenzene is either for a single isomer or for the sum of the 2 isomers) Toluene 100.0 (Action Level for Xylene is either for a single isomer or the sum of the 3 isomers) Phenols 2,4-dimethylphenol (400.0)\*Phenol (1.0) \* (For Chlorinated Systems) Aldehydes Formaldehyde 30.0

# Department of Health Services Recently Adopted Maximum Contaminant Levels For Contaminants In Drinking Water

# April 1989

### California Code of Regulations Title 22

Constituent	Maximum Contaminant Level
Inorganic (Section 64435)	mg/l
Aluminum	1.
Radioactivity (Section 64441)	pCi/l
Uranium	20
Organic (Section 64444.5)	<u>mg/l</u>
Atrazine	0.003
Bentazon	0.018
Benzene	0.001
Carbon Tetrachloride	0.0005
1,4-Dichlorobenzene	0.005
1,2-Dichloroethane	0.0005
1,1-Dichloroethylene	0.006
1,3-Dichloropropene	0.0005
Ethylbenzene	0.680
Ethylene Dibromide	0.00002
Molinate	0.02
Monochlorobenzene	0.030
Simazine	0.01
1,1,2,2-Tetrachloroethane	0.001

Thiobencarb	0.07
1,1,1-Trichloroethane	0.200
1,1,2-Trichloroethane	0.032
Trichloroethylene	0.005
Vinyl Chloride	0.0005
Xylenes	1.750*

<sup>\*</sup>MCL is for either single isomer or the sum of the isomers.

APPENDIX B

TANK SYSTEM TIGHTNESS TEST

# Lata Chart for Tank System Tightness Test

PN6827

PLEASE PRINT Property 💹 OWNER Tank(s) Acciess Regresentative Telephone Малле Address Representative Telephone 2. OPERATOR **Name** 3. REASON FOR TEST (Explain Fully) 4. WHO REQUESTED TEST AND WHEN Company or Affiliation Cate Address Telephone Identify by Direction Capacity Brand/Supplier Grada Approx. Age Steet/Fibergiass TANK INVOLVED EAST REGULAR Use additional lines #20 ENTER OK Saved Times for manifolded tanks #3 WEST 10 K ことといるのの 74 NOR アニシガル チェイス 6. INSTALLATION CONCRETE REMOTE DATA Size, Titefill make, Drop North inside driveway, Rear of station, etc. Concrete, Black Top, Suction, Remote, Make if known Earth, etc. tubes, Remote Fills Ali Size. Manriotded 1,3 0 7. UNDERGROUND is the water over the tank? WATER ☐Yes ⊠No Depth to the Water table Arranged by . 8. FILL-UP Name Тенеріпопе **ARRANGEMENTS** Extra product to "top off" and run tank tester. How and who to provide? Consider NO Lead. Terminal or other contact Company Name Telephone Paradiso Construction Company 9. CONTRACTOR. MECHANICS, any other contractor Involved 10. OTHER INFORMATION OR REMARKS Additional information on any items above. Officials or others to be advised when testing is in progress or completed. Visitors or observers present during test, etc. Tests were made on the above tank systems in accordance with test procedures prescribed for 11. TEST RESULTS as detailed on attached test charts with results as follows: VES 30-88 006 UES ナロユ 13. This is to certify that these tank systems were tested on the date(s) shown. Those indicated as 2. SENSOR National Fire Protection Association Pamphlet 329. CERTIFICATION Date ELGADILLO Paradiso Construction Co Serial No. of Thermal Sensor 9220 G Street, Oakland, CA 94603

	GRAPH GRANT	O'AKLAN CHY	D CACIFORNIA DATE OF THE POST	A 6-30-8
15. JANK TO TEST  H EAST  Identity by position  REGULAR  Brand and Grade	RIEF DIAGRAM OF TANK FIELD	16. CAPACITY  Hominal Capacity 6 00 (  Gellor  By nicel accurate capacity chart available	Tank Manufacturer's Cr	Data
17. FILL-UP FOR TEST  Shick Water Bottom Lefore Fill-up  18. SPECIAL CONDITIONS AND PROCEDURES TO TEST THIS	Tank Districtor  S TANK   Water in tank   M-Line(s)	Inventory	96'	Total Gallona ea. Reading  6048  410
,	If the start table in tank or		WATER	100
See manual sections applicable. Check below and record procedure in log (27).  Use maximum allowable test pressure for all lests.  Four pound rule does not apply to doublewalled lanks.  Crimplete section below:	19. TANK MEASUREMENTS FOR TSTT ASSEMBLY  Bottom of lank to grade*	OR 136 In	Transfer total to line 25a 21. VAPOR RECOVERY SYSTEM	Stage I Stage II
1. Is four pound rule require 17 Yes No	Add 30" for "T" probe asey	1//	24b. COEFFICIENT OF EXPANSION RECIPROGAL METHOD	GASOLINE
2. Height to 12" mark from bottom of tankin.  3. Pressure at bottom of tankP.8.I.	20. EXTENSION HOSE SETTING Tank top to grade* Extend hose on suction tube 8" or more below tank top	40 in.	Type of Product    Hydrometer Employed	69.8.
4. Pressure of top of tank.  Depth of burial  Tank die.  P.S.I.  136 in.	"If Fill pipe extends above grade, use top o  22. Thermat-Sensor reading after circular  23. Digits per *F in range of expected ch	15593 69170 -	Difference (*/-) Observed A.P.I. Gravity Reciprocal 1413 Page 1 66	+2.2.,(+2.0
Tank die.	COEFFICIENT OF EXPANSION ( 24a. Corrected A.P.I. Gravity Observed A.P.I. Gravity		Total quantity in full tank (16 or 17)	Volume change in this tank per "F Transfer to Line 26a.
	Hydrometer employed		24c. FOR TESTING WITH WATER	see Table C & D
	Corrected A.P.I. Gravity @ 60°F, From Table A		Water Temperature siter Circulation Table C	*F
The above calculations are to be used for dry soil conditions to establish a positive pressure advantage, or when using the four pound rule to compensate for the presence of subsurface water in the tank area.	Coefficient of Expansion for involved Product From Table 8		Coefficient of Water Table D  Added Surfactant? Yes ht Transfor C	
Refer to N.F.P.A. 30, Sections 2-3.2.4 and 2-7.2 and the tank manufacturer regarding allowable system test pressures.	25. (a)  Total quantity in full tank (16 or 17)  26. (a) 4, 198 198	× (b)  Coefficient of expansion for involved product	= (C) Volume change in this tank por *F  :: + 0 1 2 4 7 79	gallons (. 8/29)

KEGULAR 11 .0129 DETOR 15 42 593 42.8 608 300 4,055 4,080 1300 ,380 520 +10 11 11 670 7/08 11 150 . 1030 R'<u>†150</u> . 1 1045 " 11 1100 1034 2 10 t,011 4,011 205 7026 200 4,032 1215 11 2010 701

#### P-T Tank Test Data Chart Additional Info

	$\Omega$	4,006.	
1. Nel Volume Change al	Cosclusion of Pre	cision Testgph	$O_{l}$
Signature of Tester	Julia	Morbade	40-
Date:	-30~8-1	8	

R/Statement:
MIank and product handling system has been tested tight according to the Precision Test Criteria as established by N.F.P.A. publication 329. This is not intended to indicate permission of a leak.

OR

Tank and product handling system has failed the tank tightness test according to the Precision Test Criteria as established by N.F.P.A. publication 329.

It is the responsibility of the owner and/or operator of this system to immediately advise state and local authorities of any Implied hazard and the possibility of any reportable pollution to the environment as a result of the indicated failure of this system. Heath Consultants Incorporated does not assume any responsibility or flability for any loss of product to the environment.

Tank Owr	er/Operator			
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	EGRAPH/GFAND	OAKLAND	CALIFORNIA 6-50-88
	BRIEF DIAGRAM OF TANK FIELD	16. CAPACITY  Horninal Capacity  By most accurate capacity chart svaliable  GO 1	From  Station Chart  Tank Manufacturer's Chart  Company Enginearing Dala
17. FILL-UP FOR TEST  Stick Water Bottom Defore Fill-up In Gallons	Tunk Diamotor In.	Inventary	Gallons Total Gallons  Gallons  GO 48  489 0FF  100
18. SPECIAL CONDITIONS AND PROCEDURES TO TEST T  See manual sections applicable. Check below and record procedure in log (27).  Use maximum allowable test pressure for all tests.	HIS TANK	xcevation	WATER — 0  Transfer total to line 25s
Four pound rule dose not apply to doublewelled tanks.  Complete section below:  1. Is four pound rule required?  Yes W. No	TSTT ASSEMBLY  Bottom of tank to grade*	139 in	21. VAPOR RECOVERY SYSTEM Stage II Stage II  24b. COEFFICIENT OF EXPANSION RECIPROCAL METHOD
2. Height to 12" mark from bottom of lank  5  9.8.		<u>43</u> In.	Type of Product
1. Pressure at top of tank  ———————————————————————————————————	I. Thermal-Sonsor reeding after circula	of the	Temperature of Sample
Tank dia.	23. Digita per *F in range of expected ch COEFFICIENT OF EXPANSION 24a. Corrected A.P.I. Gravity	digits *	Observed A P.J. Gravity  Reciprocal 1501 Page # (20)  CO 58 1504 4,0279255  Total quantity in Reciprocal Volume change in this tank per *F
Water lable NOTES:	Observed A.P.i. Gravity  Ilydrometer employed  Observed Sample Temperature	Н	Yearsfer to Line 26s.  24c. FOR TESTING WITH WATER Roo Table C & D
The above calculations are to be used for dry soil conditions to establish a positive pressure advantage, or when using the four pound	Corrected A.P.I. Gravity  @ 60°F, From Yable A  Coefficient of Expansion for involved Product From Table B		Water Temperature after Circulation Table C*  Coefficient of Water Table D
rule to compensate for the presence of subsurface water in the tank srea.  Refer to N.F.P.A. 30, Sections 2-3.2.4 and 2-7.2 and the tank manufacturer regarding allowable system test pressures.	Transfer GOE to 1 Ine 25b.  25. (a)  Total quantity in full tank (18 or 17)  26. (a) 4 0 2 7 9 7 5 5	x (b) Coefficient of expansion for involved product 326	Added Surfactant? Yes Ro Transfer COE to Line 250.  = (C) Volume change in this tank por *F  = (O/73555) This is (,O/74)

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910	15/Santon K	caoline		<u> </u>	42	 	-		452				
0925	Start live &	wel test	Z	44.0	42	.170	1315	41115	459	+7	+,087	<u> </u>	
0940	Start light &	Iwel test	3	44.4	42	1315	,490	1175	4772	+13	+,161	+,014	
955	11	11	uf	44.5	42	,490	1655	1.165	476	+4	+,050	+1115	{
1010	ıl		5	44.6	42	.455	830	1,175	486	+10	17174	t.05/	
1025	u	11		44.5	47	450	1620	+.170	495	+9	+.112	+,058	
1040	11	21			47	1620	795	+.175	501	+6	+.074	7.101	
1055	<u> </u>	14	_ <u>;</u>	445	47	700	270	+ 17 1	٠-١٦	+11	+ 13/2	1 ~ 2 (	
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1140	Start low le	vel test	4	15.8	12	.265	1525	7260	558	119_	1234	+.024	+,024
1/55		wel text			12	1525	,720	+1195	573	+15	1,186	4,009	+033
1210	и	cſ	13	14.9	12	720	.90x3	1,180	590	+/7	4,211	7,031	+,∞2
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## P-T Tank Test Data Chart Additional Info

2, Statement:

Tank and product handling system has been tested tight according to the Precision Test Criteria as established by N.F.P.A. publication 329. This is not intended to indicate permission of a leak.

OR

Tank and product handling system has falled the tank tightness test according to the Precision Test Criteria as established by N.F.P.A. publication 329.

It is the responsibility of the owner and/or operator of this system to immediately advise state and local authorities of any implied hazard and the possibility of any reportable pollution to the environment as a result of the indicated failure of this system. Heath Consultants incorporated does not assume any responsibility or liability for any loss of product to the environment.

Tank Owner/Operator
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14. TEKACO 222 Hame of Supplier, Owner or Dealer.	25 750 Addre	EGRAPH/GA.	AND CHYLA		N/A 6-30-8
# 3 DEST  HOLEADED  Brand and Grode	 	MEF DIAGRAM OF TANK FIELD	16. CAPACITY  Nominal Capacity / O C Gallor  By most accurate capacity chart available / O C	From .  Station Chart  Tank Manufacture:  Company Engines  El_Charts supplied with tallions	ring Oats In FTT
17. FILL-UP FOR TEST Stick Water Bottom before fill-up	(Qaltons	Yank Dlemeter In.	Inventory	96" TOPOFF	Total Gallons es. Reading  /0184
18. SPECIAL CONDITIONS AND PROCEDI  800 manual sections applicable. Check below and record	,	TANK		NATER	10194
Use maximum allowable test pressure for all tests. Four pound rule does not apply to doublewalled tanks. Complete saction below:		19. TANK MEASUREMENTS F TSTT ASSEMBLY Bottom of tank to grade*	147	Transfer total to line 21. VAPOR RECOVERY SYSTEM	
1. Is four pound rule required?	Y•• № □	Add 30" for "T" probe assy	177	24b. COEFFICIENT OF EXPANS RECIPROCAL METHOD	Granite
2. Height to 12" mark from bottom of lank	167_in.	20. EXTENSION HOSE SETTIN	51	Type of Froduct	ATSM 6.
Pressure at lop of lank     Pressure at lop of lank		Extend hose on suction tube 6" or more below tank top	of Mile.	Temperature in Tenk After Circulation	78.0.,
Depth of burial	147_in.	22. Thermal-Sensor reading after circula	68 12 9	Difference (+/-)	57.9
Tank dia.  Water table  NOTES:	96 m	23. Digital per °F in range of expected ch COEFFICIENT OF EXPANSION 24a. Corrected A.P.I. Gravity Observed A.P.I. Gravity	(Complete after circulation)	Reciprocal 1503 Page # 1505  Total quantity in Acciprocal full tank (16 or 17)	S - 6.782435/ Volume change in this tank per 'F Transfor to Line 26a
		Hydrometer employed		24c. FOR TESTING WITH WATE	R see Table C & D
		Corrected A.P.I. Gravity  @ 60° F, From Table A		Water Temperature after Circulation Table C	*F
The above calculations are to be used for dry soil establish a positive pressure advantage, or when using titude to compensate for the presence of subsurface wall area.	he four pound	for Involved Product From Table 6 Transfer COE to Line 25b.		Table D	
Refer to N.F.P.A. 30, Sections 2-3.2.4 and 2-7.2 amenufacturer regarding allowable system test pressure		25. (a)  Total quantity in full tank (10 or 17)  26. (a) (2 7 8 7 1/ 2)	x (b)  Coefficient of expansion for involved product	Volume change in this tank	gallons ( 17.17)

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1.100	767-0.	~~~~		<u> </u>	1844				15				
1400	1st SerA07	Keading			42		1.16		050				
1415	Start Migh	level test		42.8							1062	7007	
1430	Cont-light	level tost		42.8		1440			056		1,062	1012	j
1445	1 0	- 11	4	42.8	42	,490	515	7,055	056	<u>+0</u>	+,000	<u> </u>	ļ,
1430 1445 1500	<u>,!</u>	p	5	43,1	42	.545	.630	+,085	058	+2	+,042	4013	·
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1530	И	u	7	43,0		,705		1,070		+4	T083	7613	
1545	4	14	8	431	42	,775				43	+062	+.018	
1600	11	11	9	43.1	42		9110			+4	1.083	1,002	
	Drop to	low level											
1615	1st Pour le	vel Roading	10		12			•	075				
1/270	Stat a.	Level test	11	13,4	12	,260	1360	1/60	Q80	15	-7104	7004	7004
1645	Con't low	- Sevel test	12 13	13,2	12	1360	450	1.090	084	14	5083	1.001	1,003
1700	a		13	13,1	12	1450	1535	4,085	088	+4	1,083	1.002	4,005
1715	at .	//	14	13.1	12	,535	1615	4,080	091	+3	1,062	+,018	1.023
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#### P-T Tank Test Data Chart Additional Info

	Λ	11 4 4 04	<u>ح</u> ب
1. Net Volume Change al	Conglusion of Pr	rclsign Test	agh 🔿 .
· \	يتوريد الألا	$+\Lambda h(h_{\alpha \alpha})$	0. ( <i>10</i> -
Signature of Tester		As overhice	سرم کارسانی
Date: ( - 3)	0-8-8	<u> </u>	

Statement:
Tank and product handling system has been tested tight according to the Precision Test Criteria as ustablished by N.F.P.A, publication 329. This is not intended to indicate permission of a leak.

OR

☐ Tank and product handling system has failed the tank tightness test according to the Procision Test Criteria as established by N.F.P.A. publication 329.

It is the responsibility of the owner and/or operator of this system to immediately advise state and local authorities of any implied hazard and the possibility of any reportable pollution to the environment as a result of the indicated failure of this system. Health Consultants Incorporated does not assume any responsibility or liability for any loss of product to the environment.

fank Owner/Operator	
Fank Owner/Operator	

14. TELACO 2225 TELES  Name of Supplier, Owner or Dealer  Ad	RAPH GRAND	OAKLAND		6-30-88
15. TANK TO TEST  15. TANK TO TEST  15a. I  LIGHTING BOOK TH  WASTE OL WATER FEST  Brandand Grade	BRIEF DIAGRAM OF TANK FIELD	capacity chart available	Froin Station Chart Tank Manufacturer's Company Engineerin Charts supplied with Other	g Data
17. FILL-UP FOR TEST  Stick Water Bottom before FIH-up  10 W* Gallone	Tank Diameter	Inventory	Gellons  1/8	Total Gallons es. Reading  5.50
18. SPECIAL CONDITIONS AND PROCEDURES TO TEST THE See manual sections applicable. Check below and record procedure in log (27).  Use maximum altowable test pressure for all tests. Four pound rule does not apply to doublewalled tanks.  Complete section below:	IS TANK	FOR G I	Transfer total to line 25 21. VAPOR RECOVERY SYSTEM	560   Stage
1. is four pound rule required?  DOUBLE WALLED TANK  2. Height to 12" mark from bottom of tank  3. Pressure at bottom of tank  4.	20. EXTENSION HOSE SETTIN Tank top to grade*  Extend frose on suction tube 6" or more below tank top	124_in. 16	24b. COEFFICIENT OF EXPANSIO RECIPROCAL METHOD  Type of Product Hydrometer Employed Temperature in Tenk After Circulation  Temperature of Sample	H
Depth of burial  Tank dis.  P.81.  944 in.  48 in.	22. Thermal-Sensor reading after circular 23. Digits per *F in range of expected circular COEFFICIENT OF EXPANSION 24a. Corrected A.P.I. Gravity	lation LS57/c Gdiplis 6 Gdiplis F Gdiplis Gdiplis Gdiplis Gdiplis	Observed A.P.I. Gravity	·····
Water tableIn. NOTES:	Observed A.P.i. Gravity  I hydrometer employed  Observed Sample Temperature  Corrected A.P.I. Gravity  © 60°F, From Table A	H	24c. FOR TESTING WITH WATER  Water Temperature after Circulation Table C	Transfer to Line 26s.
The above calculations are to be used for dry soil conditions to establish a positive pressure advantage, or when using the four pound rule to compensate for the presence of subsurface water in the tank area.  Refer to N.F.P.A. 30, Sections 2-3.2.4 and 2-7.2 and the tank	Coefficient of Expansion for Involved Product From Yable 8	× (b) • 0001200	Coefficient of Water Table 0  Added Surfactorit? Yes to Transfe	r COE to Line 25b.
manufacturer regarding allowable system test pressures.	Total quantity in full tank (18 or 17)	Coefficient of expension for involved product		This (OOOZ)

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1305	Pump Primed & Runni					<del></del>				TÁCTOG	0/1	9002
1000	1 may	5						15		1111-56		
1315	1st Senson Reading			42				576	<u> </u>			
1234	Start lightwel to	- 2	42.1		3/0	77()	+110	1.10	1-21	+ 000	+,003	
1340	Cont hill lultes!	- 3	1/2 0	1119	1270	7770	F000	6.111	1.77	1.007	7,007	
1070	con supremy less	4	1/2 0	1/2	10/0	100	+000	100	1777	+ 000	-009	
1400	<i>h</i>	5	1/2.0	1/2	1010	140	+ 000	711	100	+,006	7,008 7,006	
17/5	1 + 0 0 1		7-40	40	1470	1210	1,000	<u> </u>	404	1,000	1006	
1/	Drop to low leve		<b> </b>			<del></del>			ļ			
1130	1st hur level Boad			12			<u> </u>	732			- warender Strake S	
1445	Start four level test		12.0	17	.320	1320	7,000	776	1444	7009	7,009	7,009
1500	Cont low-level test	8	12.1	12	,320	,325	7,005	81(	+35	4,007	7002	7011
1515		9	12.0	12	1325	1325	1000	844	<u>+33</u>	+,007	7,007	-1018
1530	4 4	10	12.0	12	1325	,325	t.000	875	+31_	4.006	-,006	5,024
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#### P-T Tank Test Data Chart Additional Info

2 Statement:

Tank and product handling system has been tested tight according to the Precision Test Criteria as established by N.F.P.A. publication 329. This is not intended to indicate permission of a leak.

OR.

Tank and product handling system has failed the tank lightness test according to the Precision Test Criteria as established by N.F.P.A. publication 329. It is the responsibility of the owner and/or operator of this system to immediately advise state and local authorities of any implied hazard and the possibility of any reportable pollution to the environment as a result of the indicated failure of this system. Heath Consultants Incorporated does not assume any responsibility or liability for any loss of product to the environment.

Tank	Owner/Operator	
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	56RAP	4 REA	SON FOR TEST RE	QUIRE	D F	0 MW. MO	DALE	Ø.	PROP	BRTY	/	Telephone h
FORM!	LOCATION		T REQUESTED BY:	BN M	14 2A		Position		Older No.	•	Billing Address	
88 QL	20 222 rf. of	7 CON	CIAL INSTRUCTIONS: 17 ITRACTOR OR COMPANY CHANIC(S) NAME TITANK TEST TO BE		41614	KS F F.D MAKE AND		wo A	DRAD 15	SO CON	57. Co C	MKLANI
11 IDENTIFY	4	MAI 10 WEA		TEMPI	ERATURE IN T	ANKS	F 'C	COVER	CONCR	EVE	APPROXIMAT BURIAL DEPT	
EACH LINE AS TESTED	12 TIM (MILIT.		1) LOG OF TEST PR AMBIENT TEMP WEATHER,	PERATURE		AFTER	BEFORE	16 VOLUME READING AFTER	NET CHANGE	cc	16 TEST RE	
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	1040	<u>ر</u>	BLEED B	ACK	50	Ø	1034	,060	4,026			

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CENTER 1115	STARTLINE TEXT		50	
DIPER 1/30	CONTILINE TEST	10	0 1030	1029 -1001
UNLEADED 1145	11 4	1,,,	079	,028 7,00/
1260	1	49 5	0 1028	1028 +,000
1215	4	50 5	1028	1000
1220	BLEED BACK	50	2) 102.8	,048 +,020
# FAST 1240	S-10 1 1+ 75	-		
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13/0	UNTILINE LEST	1	0 ,033	1032 7001
1325	il 4	49 5 50 50		103/ 709/
1346	11 11	50 51		103( +000
1345	BLEED BLCK	50 d		.03/ tood
	1	30 0	700	.054 1023

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Attention: Mr. R. R. Zielinski

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QUALITY CONTROL REVIEWER

Leonard K. George

Senior Engineer