



PACIFIC  
ENVIRONMENTAL  
GROUP INC.

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October 12, 1993  
Project 330-06.22

Mr. Mike Whelan  
ARCO Products Company  
P.O. Box 5811  
San Mateo, California 94402

Re: ARCO Service Station 0608  
17601 Hesperian Boulevard  
San Lorenzo, California

Dear Mr. Whelan:

This letter presents the results of conducting aquifer testing, air sparge, soil vapor extraction (SVE), and in-situ bioremediation feasibility testing at the site referenced above (Figure 1). Relevant site background is presented in PACIFIC's *Additional Investigation Work Plan*, dated February 4, 1993, and PACIFIC's *Investigation Report*, dated July 27, 1993. A summary of each test is described below.

#### AQUIFER TESTING

During the week of March 29, 1993, aquifer testing was performed at the site to determine the hydraulic characteristics of the shallow water-bearing zone both on and off site. The testing consisted of step-discharge tests in Wells E-1A and MW-10 to determine appropriate flow rates for 8-hour constant discharge tests to be conducted in each well. The pumping tests were conducted in wells completed in coarser-grained geologic materials (silty sand and sandy silt), at the site (Well E-1A), and downgradient of the site (Well MW-10). The purpose of the tests was to collect aquifer characterization data in order to understand and simulate groundwater flow and hydrocarbon transport phenomena, and for later use in remedial system design.

In addition to the pumping tests, slug tests were performed in Wells MW-14 and MW-23. These wells are completed in finer-grained geologic materials (clays and sandy clays). The purpose of performing the slug tests was to obtain contrasting

hydraulic conductivity data for the finer-grained materials at the lateral and down-gradient hydrocarbon plume boundaries.

The procedures for the aquifer testing were described in a Work Plan dated February 14, 1993, prepared by PACIFIC. The pump used for tests in Wells E-1A and MW-10 was a 3-inch diameter Grundfos, Model JS 10-05, 0.5 horsepower electric submersible, rated at 10 gallons per minute (gpm) at 160 feet of pumping head. At the total discharge head encountered during these tests (less than 40 feet), the pump is capable of discharging more than 15 gpm. The pump was placed in each well such that approximately 11 feet of drawdown was available for testing. All groundwater pumped during the testing program was discharged through the existing site treatment system. Construction details for pumping and monitoring wells used in the testing program are presented in Table 1. Well locations are shown on Figure 2.

Barometric pressure data were obtained from the National Weather Service station located at San Francisco International Airport for the periods during which the 8-hour constant discharge tests were conducted. These data indicated that during the 8 hours of pumping in Well E-1A, barometric pressure decreased by an amount equivalent to 0.1 feet of water, and during the test in Well MW-10, barometric pressure decreased by an amount equivalent to 0.08 feet of water. Since the barometric efficiency of a well usually ranges from 20 to 75 percent (Kruseman and de Ridder, 1990), these pressure fluctuations could have produced up to 0.075 feet of water level *increase* in monitoring wells during the test in Well E-1A, and less during the test in Well MW-10. The small magnitude of the possible fluctuations in conjunction with the direction of the possible effect (increasing water levels) suggests all drawdown observed during the pumping test was due to pumping, or influences other than barometric pressure changes. Therefore, the pumping test data were not corrected for the influence of possible barometric fluctuation effects.

### **Pumping Test Observations**

Plots of drawdown and recovery data for both the step discharge and the constant discharge tests, and slug tests, are included as Figures A-1 through A-22 in Attachment A.

#### **Well E-1A**

The step discharge test for Well E-1A was started at a discharge rate of 0.5 gpm. After pumping at this rate for approximately 20 minutes, the water level well

stabilized at 1.6 feet of drawdown. Forty minutes into the test the flow rate was increased to 1 gpm. This increased drawdown in the well to approximately 2.2 feet. The water level stabilized at approximately 60 minutes into the test. The flow rate was increased to 2 gpm at 70 minutes into the test. This discharge rate created approximately 58 feet of drawdown before the water level stabilized. After 1 hour at 2 gpm, the flow rate was increased to 4 gpm. The well was unable to sustain 4 gpm and dewatered within 10 minutes. These data along with calculated values of specific capacity are summarized in Table 2.

Based on the findings of the step discharge test, a pumping rate of 2 gpm was selected for the 8-hour constant discharge test in Well E-1A. The constant discharge test was performed March 31, 1993. The pumping began at 8:00 AM and within 80 minutes, drawdown stabilized at approximately 5.4 feet. Following 8 hours of pumping, the pump was deenergized and the recovery water levels were monitored.

Water levels during the test were monitored in Wells MW-5, MW-8, MW-9, MW-13, and SP-1/V-4. The distance from the pumping well and maximum drawdowns observed were as follows:

Well Number	Distance (feet)	Maximum Drawdown (feet)
MW-5	36	0.04
MW-8	40	0.05
MW-9	81	0.05*
MW-13	115	0.08*
SP-1	~29	0.15
V-4	~29	0.05

\* = Due to fluctuations of the water levels in Wells MW-9 and MW-13 during the test, it is uncertain that the drawdown observed was due to the pumping of Well E-1A.

#### Well MW-10

The step discharge test in Well MW-10 was started at a discharge rate of 0.5 gpm. After pumping at this rate for approximately 15 minutes the water level in the well stabilized at 0.30 feet of drawdown. Thirty minutes into the test the flow rate was increased to 1 gpm. At approximately 35 minutes into the test the water level in the well stabilized at approximately 0.38 feet of drawdown. The flow rate was

increased again at 70 minutes into the test, to 2 gpm. This flow rate created approximately 0.83 feet of drawdown before the water level stabilized. After 1 hour at 2 gpm the flow rate was increased to 4 gpm. The 4 gpm flow rate created approximately 3.5 feet of drawdown. This pumping rate was maintained for 90 minutes after which the pumping rate was increased to 6 gpm. The well was unable to sustain a flow rate of 6 gpm and dewatered within approximately 10 minutes. These data and calculated specific capacity values are summarized in Table 2.

Based on the findings of the step discharge test, a pumping rate of 4 gpm was selected for the 8-hour constant discharge test. The constant discharge test was performed on April 2, 1993. Pumping began at 8:30 AM and within 20 minutes drawdown stabilized at approximately 3.8 feet. Following 8 hours of pumping the pump was deenergized and the well was monitored while it recovered.

Drawdown during the test was observed in Wells MW-9, MW-11, MW-14, MW-15, and SP-2/V-5. The distance from the pumping well and maximum drawdowns observed were as follows:

Well Number	Distance (feet)	Maximum Drawdown (feet)
MW-9	225	0.03*
MW-11	113	0.08*
MW-14	212	0.02*
MW-15	223	0.03*
SP-2	16	0.35
V-5	16	0.15

\* = Due to the small magnitude of the drawdown observed, it is uncertain that it was due to the pumping of Well MW-10.

### Analysis and Results

Data collected during the recovery of the constant rate pump tests were used to determine transmissivity values for Wells E-1A and MW-10 using the **Cooper and Jacob (1946) straight line technique**. The equations and parameters used in these analyses are shown on the appropriate figure in Attachment A. A summary tabulation of pumping test analytical results is presented below.

Well	Transmissivity	Hydraulic Conductivity	Storage Coefficient
E-1A	112	5.3E-4	N/A
MW-10	310	2.1E-3	N/A
SP-2	130	5.0E-2	0.012
V-5	800	4.0E-2	0.34

N/A = Not available  
Notes: Transmissivity in gallons per day per foot.  
Hydraulic conductivity in centimeters per second.  
Storage coefficient is dimensionless.

The values of hydraulic conductivity determined from this testing program are typical of geologic materials consisting of fine to coarse sand (Kruseman and de Ridder, 1990). Transmissivity values are based on estimated aquifer thicknesses and assume each well used in the test fully penetrated the aquifer. This and other simplifying assumptions (the aquifer is infinite in areal extent, isotropic, homogeneous, and of uniform thickness; the aquifer is confined, or if unconfined, drawdown is small relative to aquifer thickness) necessarily limit the accuracy of the calculated hydraulic parameters to within an order of magnitude.

The test data allowed storativity values to be calculated only for Wells SP-2 and V-5. The value determined for Well SP-2 from this test (0.012) is at the low end of the range typical of unconfined storativity values. Typical unconfined storativity values range from 0.02 to 0.3, (Kruseman and de Ridder, 1990). The value calculated for Well V-5 (truly unconfined) is near the high end of the range typical for unconfined formations. ~~The difference in values is not unexpected since Well SP-2~~ is screened at some depth below the water table. The layer of saturated soils above the well screen behaves similarly to a leaky confining layer; thus producing a corresponding response to pumping, and a correspondingly lower storativity value.

**Specific capacity** is a measure of well yield and indicates the volume of flow a well can support at a given level of drawdown. Table 2 shows the calculated specific capacities for Wells E-1A and MW-10.

### Slug Test Results

Slug tests were performed in Wells MW-14 and MW-23 on April 29, 1993. Falling head and rising head tests were performed on each well. Slug test data was analyzed using a method developed by Bouwer and Rice (1976). This method is designed to determine the hydraulic conductivity within an unconfined aquifer and

is based on the **Thiem equation** of steady state flow to a well. Plots of the falling and rising head test data are presented on Figures A-17 through A-22.

The general equation to derive hydraulic conductivity is:

$$K = [(r_c^2/2Lt)/((1.1/\ln(H/r_w)) + (C/(L/r_w)))] \ln(y_o/y_t)$$

Where:

- $r_c$  = Well casing radius (feet)
- $L$  = Length of saturated well screen (feet)
- $t$  = time (minutes)
- $H$  = Distance from the top of the water table to the bottom of the screened interval open to the aquifer (feet)
- $C$  = Dimensionless parameter correlating to  $L/r_w$
- $r_w$  = Borehole radius (feet)
- $y_o$  = Initial water level (feet)
- $y_t$  = Level at time  $t$  (feet)
- $K$  = Hydraulic conductivity (feet/minute)

Transmissivity is then derived from the relation  $T=K/B$ , where  $B$  (aquifer thickness) is usually treated as being equivalent to parameter  $H$  above, for the purpose of analysis.

The values of transmissivity and hydraulic conductivity for Wells MW-14 and MW-23 calculated using this method are tabulated below. The parameters used in the equation above for each analysis are listed on Figures A-17 through A-22. Note that two falling and rising head tests were performed on Well MW-14; the values displayed below represent the arithmetic mean of the results of the two tests.

Well and Test	Hydraulic Conductivity	Transmissivity
<b>MW-14</b>		
Falling head	2.2E-3	700
Rising head	7.8E-4	240
Mean	1.5E-3	470
<b>MW-23</b>		
Falling head	1.4E-3	320
Rising head	1.7E-3	400
Mean	1.6E-3	360
Notes: Hydraulic conductivity is in centimeters per second. Transmissivity is in gallons per day per foot.		

The calculated values are typical of formations consisting of fine sand or silt. Since the materials in which these wells are screened consist predominantly of clays, secondary permeability characteristics such as cracks in the soil structure, or root-let holes, may be contributing to the higher than expected permeability values.

### Aquifer Test Conclusions

Although the grain-size distribution of geologic materials at and downgradient of the site ranges from clays to silts to fine and coarse sands, the distribution of hydraulic conductivities determined from the aquifer test program did not show a strong correlation to predominant grain sizes noted on the boring logs of the wells tested. Especially noted were higher than expected hydraulic conductivities from slug test results. The shallow, unconfined aquifer appears to be capable of producing 2 to 4 gallons per minute, or more, in the vicinity of the site. The radius of groundwater capture of pumping wells tested is discussed in detail below.

Pumping test results and aquifer geometric factors were used to estimate typical capture zones of Wells E-1A and MW-10. Since capture zone determination depends upon achieving steady-state drawdown conditions, for this analysis, steady state was assumed to be reached after 1 day of pumping. A computer model was employed for this analysis. The model is called AqModel (O'Neill, 1990), and is distributed by WellWare of Davis, California.

The time-dependent head distribution from which the capture zone was determined is based on the Theis analytical solution for flow to a pumping well. The capture zones thus determined have a radius of approximately 30 to 40 feet for Well E-1A, and approximately 70 to 80 feet for Well MW-10. These capture zones are shown as shaded areas on Figure 2.

### AIR SPARGE TESTING

PACIFIC conducted an off-site air sparge (sparge) test on May 4, 1993, and an on-site sparge test on May 5, 1993. The objective of testing was to evaluate the feasibility of using sparge technology at the site. A description of testing, including procedures, results, and conclusions is presented below. Field data sheets are presented as Attachment B.

### Sparge Test Procedures

Sparge testing consisted of two phases. The first phase consisted of injecting a mixture of helium and air at the sparge point (Well SP-1 or SP-2), and the second phase consisted of helium/air injection at the sparge point and SVE at a monitor-

ing point (Well MW-5 or MW-10). The first phase was conducted to verify the radius of sparge influence. The second phase was conducted to determine if the radius of sparge influence could be increased using SVE (SVE may create preferential subsurface flow paths).

Immediately before each test was performed, depth to groundwater was measured in the sparge well. Measurements were used to determine the initial sparge injection pressure (based on the pressure head due to standing water in the sparge well).

Each test was performed by connecting an oil-less air compressor and a compressed helium-gas cylinder to the sparge well. Helium was utilized as a tracer gas to verify the radius of sparge influence. Air was injected to monitor the changes in volatile organic compounds (VOCs) and dissolved oxygen (DO) concentrations. During testing, the injection pressure and flow rate were measured at the sparge well. Helium, VOCs, and DO were also measured at the surrounding monitoring wells. Helium was measured using a portable detector, VOCs were measured using a flame-ionization detector (FID), and DO was measured using a portable meter.

During the second phase of testing, helium/air injection continued as described above, and a 2.5-horsepower regenerative blower was connected to the extraction well head (MW-5 and MW-10) in order to create a vacuum. Extracted soil vapor was treated using vapor-phase activated carbon prior to atmospheric discharge. Helium, VOCs, and DO were also measured at the surrounding monitoring wells during the second phase of testing.

### Sparge Test Results

- o During the first phase of off-site testing, VOCs in Wells V-5, MW-10, and MW-11 were at background levels and remained unchanged during testing. During the second phase, VOCs in Well V-5 increased from 2.5 to 20 parts per million (ppm), VOCs in Well MW-10 remained a background levels, and VOCs were not measured in MW-11.
- o During the first phase of off-site testing, helium levels increased from 0 to 0.18 percent in V-5. Helium was not detected in MW-10 or MW-11 during testing. During the second phase, helium levels in Well V-5 increased from 0.14 to 3.2 percent, helium was not detected in MW-10, and helium was not measured in MW-11.



- o During the first phase of off-site testing, DO concentrations in V-5 increased from 2.1 to 5.1 micrograms per liter (ug/L) and 1.61 to 3.68 ug/L in Well MW-10. DO concentrations remained unchanged in Well MW-11. During the second phase, the DO meter malfunctioned preventing further DO measurement.
- o During the first phase of on-site testing, VOCs in Well V-4 increased from 1,100 to + 10,000 ppm, and from 4.5 to + 10,000 ppm in Well MW-5. VOCs in Well MW-8 were at background levels and remained unchanged during testing. During the second phase, VOCs in Wells V-4 and MW-5 were measured at + 10,000 ppm and remained unchanged. VOCs were not measured in Well MW-8 during the second phase of testing.
- o During the first phase of on-site testing, helium levels ranged from 0 to 5.3 percent in Well V-4. Helium was not detected in Wells MW-5, E1-A, and MW-8. During the second phase, helium levels ranged from 4.9 to 9.3 percent in V-4, and from 0 to 12 percent in Well MW-5. Helium was not measured in Well MW-8 during the second phase of testing.
- o During on-site testing, measurement of DO was not possible; the DO meter malfunctioned during the second phase of off-site testing.
- o The data for both tests indicate that the radius of sparge influence did not encompass the nearest monitoring point (approximately 16 feet).

### Sparge Test Conclusions

The objective of conducting sparge testing was to determine the feasibility of using sparge technology at the site. Given the observed radius of sparge influence and changes in VOC, DO, and helium concentrations, **PACIFIC concludes that the feasibility of using sparge technology at this site is limited. PACIFIC recommends conducting a comparison of alternative groundwater remedial technologies prior to initiation of sparge remedial system design.**

### SOIL VAPOR EXTRACTION TESTING

PACIFIC conducted an off-site SVE test on April 29, 1993, and an on-site SVE test on April 30, 1993. The objective of testing was to evaluate the feasibility of

using SVE technology at the site. A description of testing, including relevant subsurface conditions, procedures, data analysis, results, and conclusions is presented below. Field data sheets and computer modeling worksheets are presented as Attachment C. Certified analytical reports and chain-of-custody documentation are provided as Attachment D.

### **Relevant Subsurface Conditions**

Soils underlying the site consist primarily of clay, sandy clay, sand, and clayey sand. Based on site lithology, permeability to air flow was expected to range between 1 and 10 darcys (1 darcy =  $9.87 \times 10^{-9}$  cm<sup>2</sup>). The boring logs of Wells SP-2/V-5, MW-10, and MW-11 for the off-site test, and Wells SP-1/V-4, MW-5, MW-8, and E1-A for the on-site test are referenced from previous reports prepared by Applied GeoSystems and PACIFIC.

Based on previous quarterly groundwater monitoring reports, depth to groundwater was estimated to be 10 to 12 feet below ground surface across the site.

### **SVE Test Procedures**

Prior to testing, PACIFIC notified the Bay Area Air Quality Management District (BAAQMD) regarding the proposed testing. Copies of these notifications are available on request.

Immediately before each test was performed, depth to groundwater was measured in all applicable groundwater monitoring wells. Measurements were compared with estimated well screen intervals to determine the feasibility of using groundwater monitoring wells as extraction/monitoring points. Based on the available data, it appeared all wells could serve as monitoring points. After accounting for the upwelling of groundwater during vacuum application, it was determined that Wells V-4 and MW-8 (on-site) and Well V-5 (off-site) could serve as extraction points.

Each test was performed by connecting a 2.5-horsepower regenerative blower to the extraction well head in order to create a vacuum. Extracted soil vapor was treated using vapor-phase activated carbon prior to atmospheric discharge. During testing, vacuum influence at the surrounding monitoring well head(s), and extracted soil vapor flow rate and applied vacuum at the extraction well head were measured. Bag samples of extracted soil vapor were also collected during testing, and analyzed for VOCs by EPA Methods 8015 and 8020.

During vacuum application, the measured radial influence was minimal. Therefore, PACIFIC also applied positive pressure in an attempt to collect additional radial influence data. During both tests however, the pressure hose ruptured and the positive pressure tests were terminated.

#### SVE Test Data Analysis

Radial pressure distribution was modeled using field measurements and the steady-state solution to the radial flow equation. Permeability to air flow was calculated using field data and the steady-state solution for the radial volumetric flow rate. Additionally, the radial volumetric flow rate and permeability results were used to generate flow rate versus applied vacuum curves. These curves provided several SVE system design parameters, including effective radius of influence, blower sizing, and maximum design flow rate. Initial petroleum hydrocarbon removal rates were calculated using the soil vapor sample concentration data and maximum design flow rate. Field data sheets are presented as Attachment C. Data analysis calculations and solutions are available upon request.

#### SVE Test Results

- o The data for both tests indicate that the vacuum application limit was restricted to a radial boundary which did not encompass the nearest monitoring point.
- o By fitting field data from the off-site test to the steady-state radial flow equation, the effective radius of influence ( $R_e$ ) was determined to be 9.5 feet. The radius of influence was generated by a pressure differential of 40 inches of water. The air flow rate was less than 3 scfm. A value for  $R_e$  could not be determined from the data collected from the on-site test.
- o The intrinsic permeability to air flow ( $k$ ) was estimated from boring logs and field data. Boring log data indicated  $k$  ranges between 1 and 10 darcys, or an average of 5 darcys. Field data applied to the steady-state flow equations determined  $k$  as 3.96 darcys. A value for  $k$  could not be determined from the data collected from the on-site test.
- o Two extracted soil vapor samples were collected from Well V-4 during the on-site test. Sample V4-1 was collected prior to beginning SVE, and sample V4-2 was collected just SVE termination. The certified analytical reports indicate that TPH-g concentra-

tions ranged from 100 to 8,500 ug/L. Benzene, toluene, ethylbenzene, and xylenes were detected in both samples.

- o Three extracted soil vapor samples were collected from Well V-5 during the off-site test. Sample V5-0 was collected prior to beginning SVE, sample V5-1 was collected after approximately 15 minutes, and sample V5-2 was collected just prior to SVE termination. The certified analytical reports indicate that TPH-g concentrations ranged from 6.7 to 13 ug/L. Benzene was not detected in any sample, however, toluene, ethylbenzene, and xylenes were detected in all samples.

### **SVE Test Conclusions**

The objective of conducting a SVE test was to determine the feasibility of using SVE technology at the site. Given the estimated flow rate and extraction well spacing requirements, **PACIFIC concludes that the feasibility of using SVE technology at this site is very limited. PACIFIC recommends conducting a comparison of alternative soil remedial technologies prior to initiation of SVE remedial system design.**

### **IN-SITU BIOREMEDIATION FEASIBILITY TESTING**

PACIFIC initiated an off-site in-situ bioremediation feasibility on March 9, 1993. The objective of testing was to evaluate the feasibility of using in-situ bioremediation technology at the site. A description of testing, including procedures, results, and conclusions is presented below. Certified analytical reports and chain-of-custody documentation are provided as Attachment E.

### **In-situ Bioremediation Testing Procedures**

Six soil samples were collected during the exploratory soil boring program from various locations (Figure 3), and were evaluated to determine if in-situ bioremediation is occurring at the referenced site. PACIFIC contracted BioScreen Testing Services, Inc. to perform a baseline analysis on the six soil samples.

The baseline analysis consisted of nutrient analysis, and moisture content, pH, and microbial testing. Nutrient analysis, and moisture and pH testing is performed to determine if conditions will support in-situ microbial growth. Nutrient analysis consisted of measuring ammonia, nitrate, phosphate, potassium, calcium, magnesium, and iron concentrations. Microbiological testing is performed to determine if bioremediation was occurring at the time a soil sample was collected. Microbi-

ological testing consisted of a heterotrophic plate count, and florescent *Pseudomonas* and hydrocarbon degraders count.

### **In-situ Bioremediation Testing Results**

- o Nutrient Analysis: Ammonia and phosphate were not detected in any sample. Nitrate was not detected in any sample, except for sample B-11 at a concentration of 2.4 ppm. Elevated concentrations of potassium, calcium, magnesium, and iron were detected in all samples:
  - Potassium concentrations ranged from 333 to 756 ppm.
  - Calcium concentrations ranged from 3,100 to 4,340 ppm.
  - Magnesium concentrations ranged from 2,820 to 6,150 ppm.
  - Iron concentrations ranged from 9,460 to 19,200 ppm.
- o Moisture Content and pH: The moisture content and pH of all samples were within the normal range to support microbial growth:
  - Moisture concentrations ranged from 11.76 to 23.82 percent.
  - pH concentrations ranged from 7.24 to 8.28 units.
- o Microbiological Testing: Normal levels of heterotrophic plate count organisms should be in the  $10^5$  to  $10^6$  colony forming units per gram (CFU/gm) range. The results of the heterotrophic plate counts show levels that are below normal, which ranged from non-detected (less than 1,000 CFU/gm) to  $6.2 \times 10^4$  CFU/gm. The florescent *Pseudomonas* and hydrocarbon degraders levels should be in the  $10^3$  and  $10^5$  CFU/gm range, respectively. Florescent *Pseudomonas* were not detected in any sample. Hydrocarbon degraders were not detected in any sample, except for sample B-11 at a concentration of  $4.0 \times 10^3$  CFU/gm.

### **In-situ Bioremediation Testing Conclusions**

Based on the baseline analytical results, it appears that insignificant bioremediation is taking place at this time. Further, column testing was not performed on any sample since the observed characteristics favorable to bioremediation were not sufficient to warrant further study.

It may be possible to enhance in-situ bioremediation with nutrient addition. However, nutrient addition could be severely limited due to site lithology, and would have to be carefully buffered to prevent inorganic precipitation due to the high minerals content observed in all samples. In order to conclude to the feasibility of enhancing bioremediation with nutrient addition, further field study would be necessary. Given the results of the baseline analysis and the site lithology, further field study is not recommended at this time. PACIFIC recommends conducting a comparison of alternative soil remedial technologies prior to initiation of further field study.

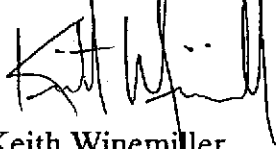
#### SUMMARY

PACIFIC conducted aquifer testing, air sparge, SVE, and in-situ bioremediation feasibility testing at the referenced site. The results of the feasibility testing indicate that air sparge, SVE, and in-situ bioremediation technologies have no or limited feasibility at the site. PACIFIC recommends conducting a comparison of alternative groundwater and soil remedial technologies prior to initiation of any remedial system design. PACIFIC will conduct this comparison in the proposed Remedial Investigation and Feasibility Study.

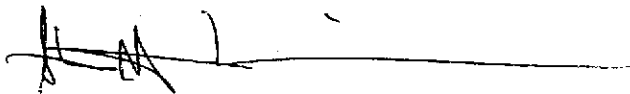
If you have any questions regarding this letter, please call us at (408) 441-7500.

Sincerely,

Pacific Environmental Group, Inc.



Keith Winemiller  
Senior Staff Engineer



Shaw Garakani  
Project Engineer

**REFERENCES**

Cooper, H.H. and C.E. Jacob, 1946, A generalized graphical method for evaluating formation constants and summarizing well field history, *Am. Geophys. union Trans*, vol. 27: 526:534.

Bouwer, H. and R.C. Rice, 1976, A slug test for determining hydraulic conductivity of unconfined aquifers with completely or partially penetrating wells, *Water Resources Res.* vol. 12: 423-428.

Kruseman, G.P and N.A. de Ridder, 1990, *Analysis and Evaluation of Pumping Test Data*, Second Edition, International Institute for Land Reclamation and Improvement, Wageningen, The Netherlands.

O'Neill, G. T., 1990, *AqModel*, Version 2.11 for DOS, Wellware, Davis, California

Attachments: Table 1 - Well Construction Details  
Table 2 - Step Discharge Test Data  
Figure 1 - Site Location Map  
Figure 2 - Well Location Map  
Figure 3 - Soil Boring Location Map  
Attachment A - Aquifer Test Data and Figures  
Attachment B - Air Sparge Test Field Data  
Attachment C - Soil Vapor Extraction Test, Field Data, and  
Computer Modeling Worksheets  
Attachment D - Soil Vapor Extraction Test, Certified Analytical  
Reports, and Chain-of-Custody Documentation  
Attachment E - In-Situ Bioremediation Feasibility Testing,  
Certified Analytical Reports, and Chain-of-  
Custody Documentation

cc: Ms. Juliett Shin, Alameda County Health Care Services Agency  
Mr. Chris Winsor, ARCO Products Company  
Mr. Richard Hiatt, San Francisco Regional Water Quality Control Board

Table 1  
Well Construction Details

ARCO Service Station 0608  
17601 Hesperian Boulevard  
San Lorenzo, California

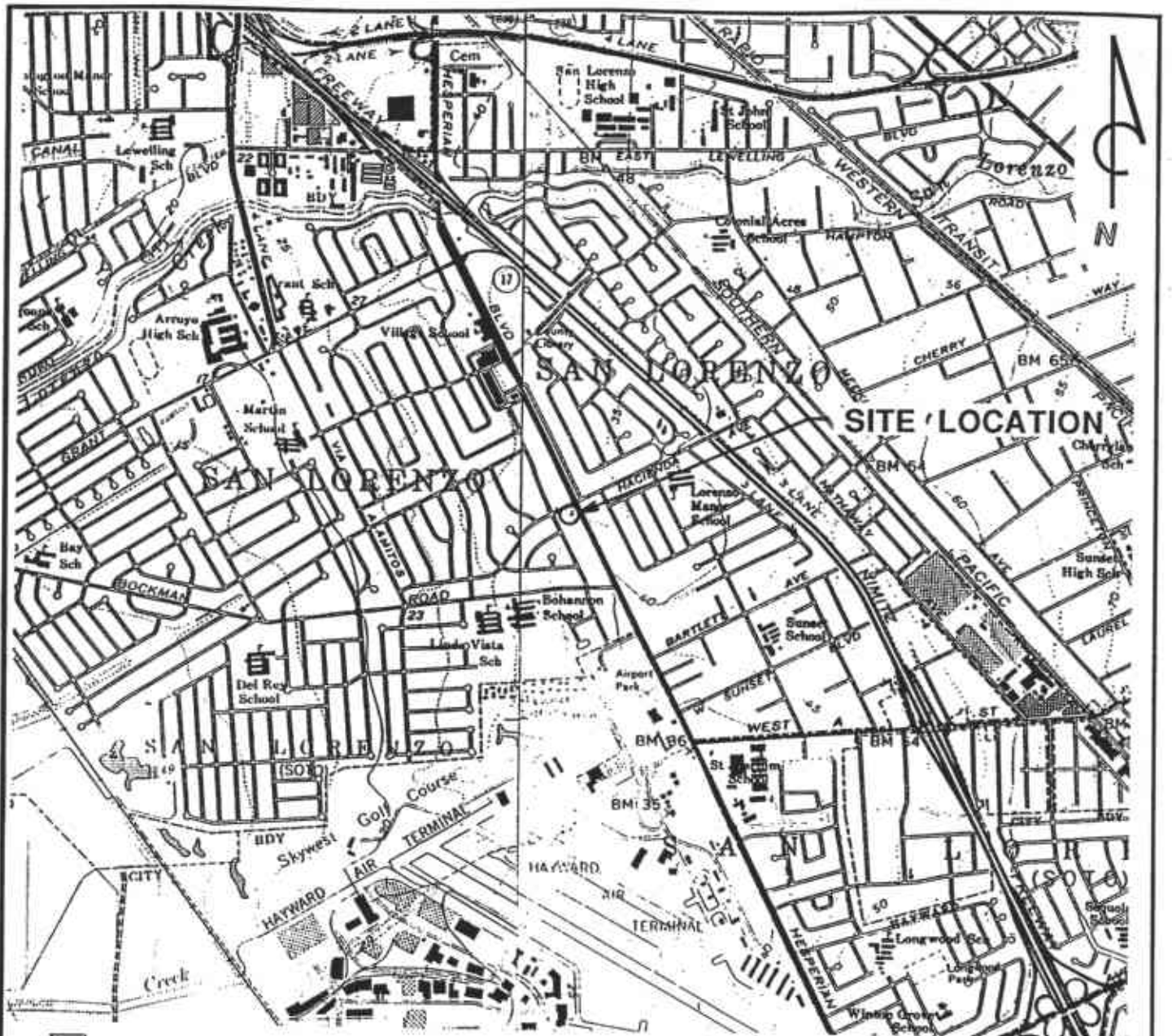
Well Number	Total Depth (feet)	Screened Interval (feet)	Casing Diameter (inches)
E-1A	26	6 - 26	6
MW-5	14	4 - 14	4
MW-8	21.5	6.5 - 21.5	3
MW-9	19.5	6 - 19.5	3
MW-13	23	8 - 23	3
MW-10	23	6 - 23	3
MW-11	19.5	6 - 19.5	3
MW-14	23	8 - 23	3
MW-15	23	8 - 23	3
SP-1	21	20 - 21	2
SP-2	19	18 - 19	2
V-4	15	6 - 15	2
V-5	11	6 - 11	2



**Table 2  
Step Discharge Test Data**

ARCO Service Station 0608  
17601 Hesperian Boulevard  
San Lorenzo, California

Step Number	Discharge Rate (gpm)	Duration of Step (minutes)	Maximum Drawdown (feet)	Specific Capacity (gpm/ft)
<b>Well E-1A</b>				
1	0.5	40	1.58	0.32
2	1	30	2.15	0.47
3	2	60	5.80	0.34
4	4	8	>11	NA
<b>Well MW-10</b>				
1	0.5	30	0.33	1.5
2	1	40	0.41	2.4
3	2	60	0.85	2.4
4	4	90	3.66	1.1
5	6	10	>11.23	NA
gpm = Gallons per minute NA = Not applicable				

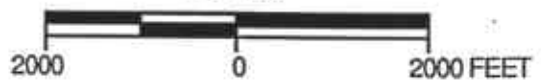


QUADRANGLE LOCATION

**REFERENCES:**

USGS 7.5 MIN. TOPOGRAPHIC MAP  
 TITLED: HAYWARD, CALIFORNIA  
 DATED: 1959 REVISED: 1980  
 TITLED: SAN LEANDRO, CALIFORNIA  
 DATED: 1959 REVISED: 1980

**SCALE**

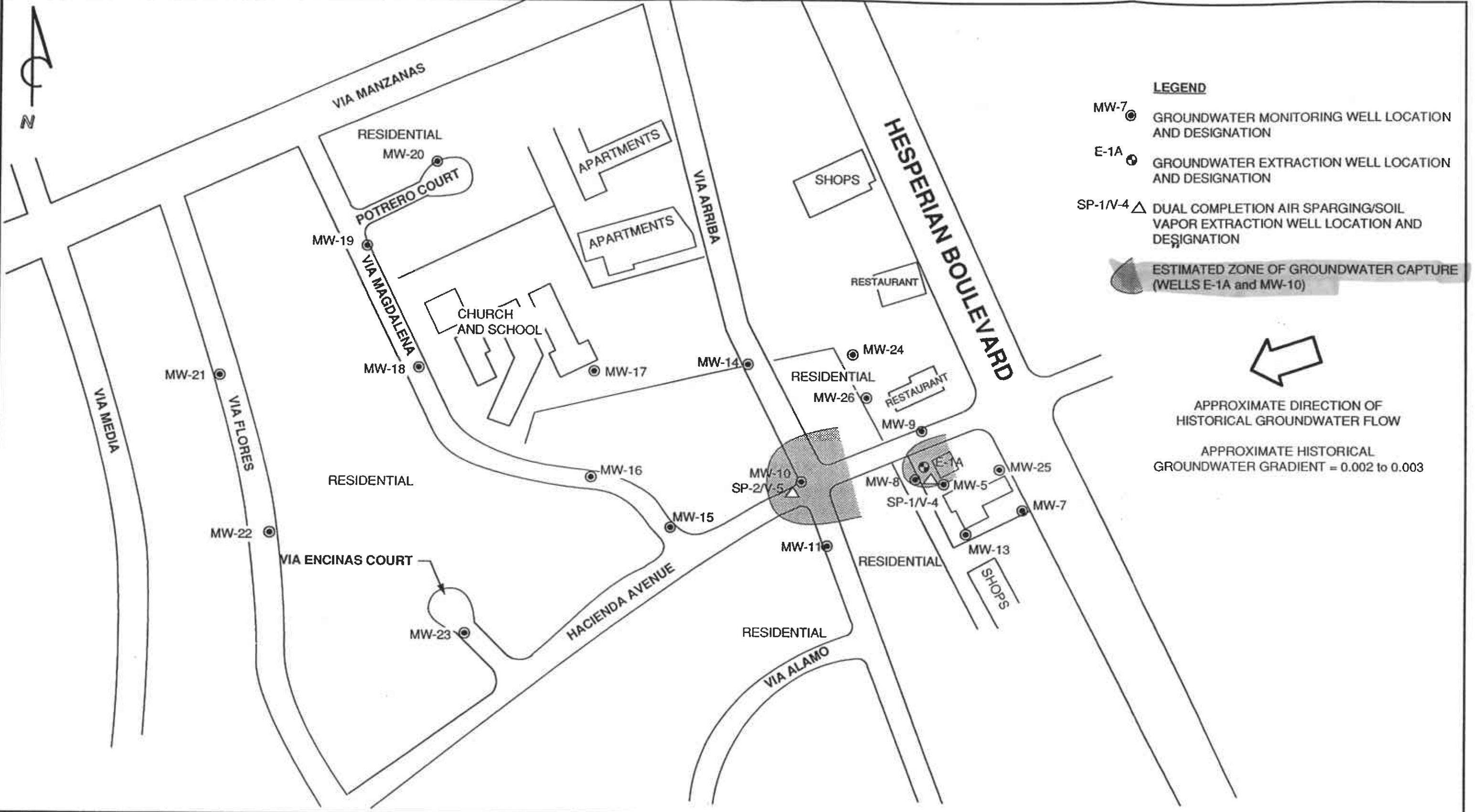


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 San Lorenzo, California

SITE LOCATION MAP

FIGURE:  
 1  
 PROJECT:  
 330-06.22



**LEGEND**

- MW-7 ● GROUNDWATER MONITORING WELL LOCATION AND DESIGNATION
- E-1A ● GROUNDWATER EXTRACTION WELL LOCATION AND DESIGNATION
- SP-1/V-4 ▲ DUAL COMPLETION AIR SPARGING/SOIL VAPOR EXTRACTION WELL LOCATION AND DESIGNATION

ESTIMATED ZONE OF GROUNDWATER CAPTURE (WELLS E-1A and MW-10)

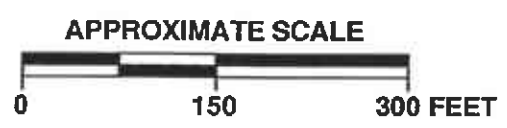


APPROXIMATE DIRECTION OF HISTORICAL GROUNDWATER FLOW

APPROXIMATE HISTORICAL GROUNDWATER GRADIENT = 0.002 to 0.003



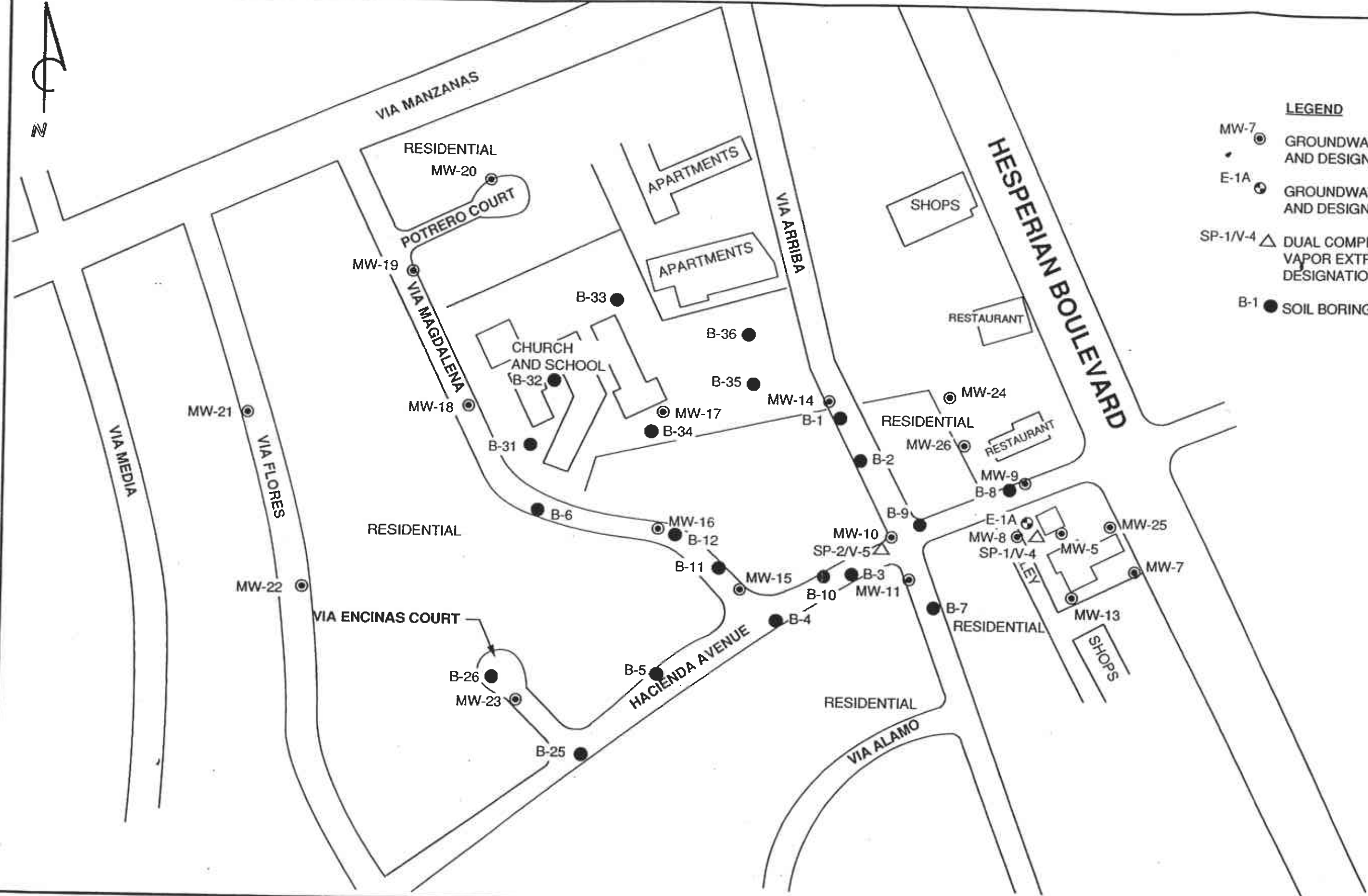
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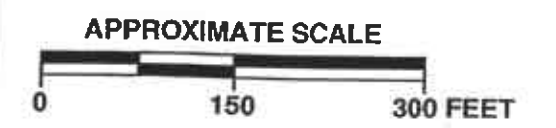
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WELL LOCATION MAP

FIGURE: 2  
PROJECT: 330-06.22



- LEGEND**
- MW-7 ● GROUNDWATER MONITORING WELL LOCATION AND DESIGNATION
  - E-1A ● GROUNDWATER EXTRACTION WELL LOCATION AND DESIGNATION
  - SP-1/V-4 △ DUAL COMPLETION AIR SPARGING/SOIL VAPOR EXTRACTION WELL LOCATION AND DESIGNATION
  - B-1 ● SOIL BORING LOCATION AND DESIGNATION



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SOIL BORING LOCATON MAP

FIGURE:  
**3**  
 PROJECT:  
 330-06.22

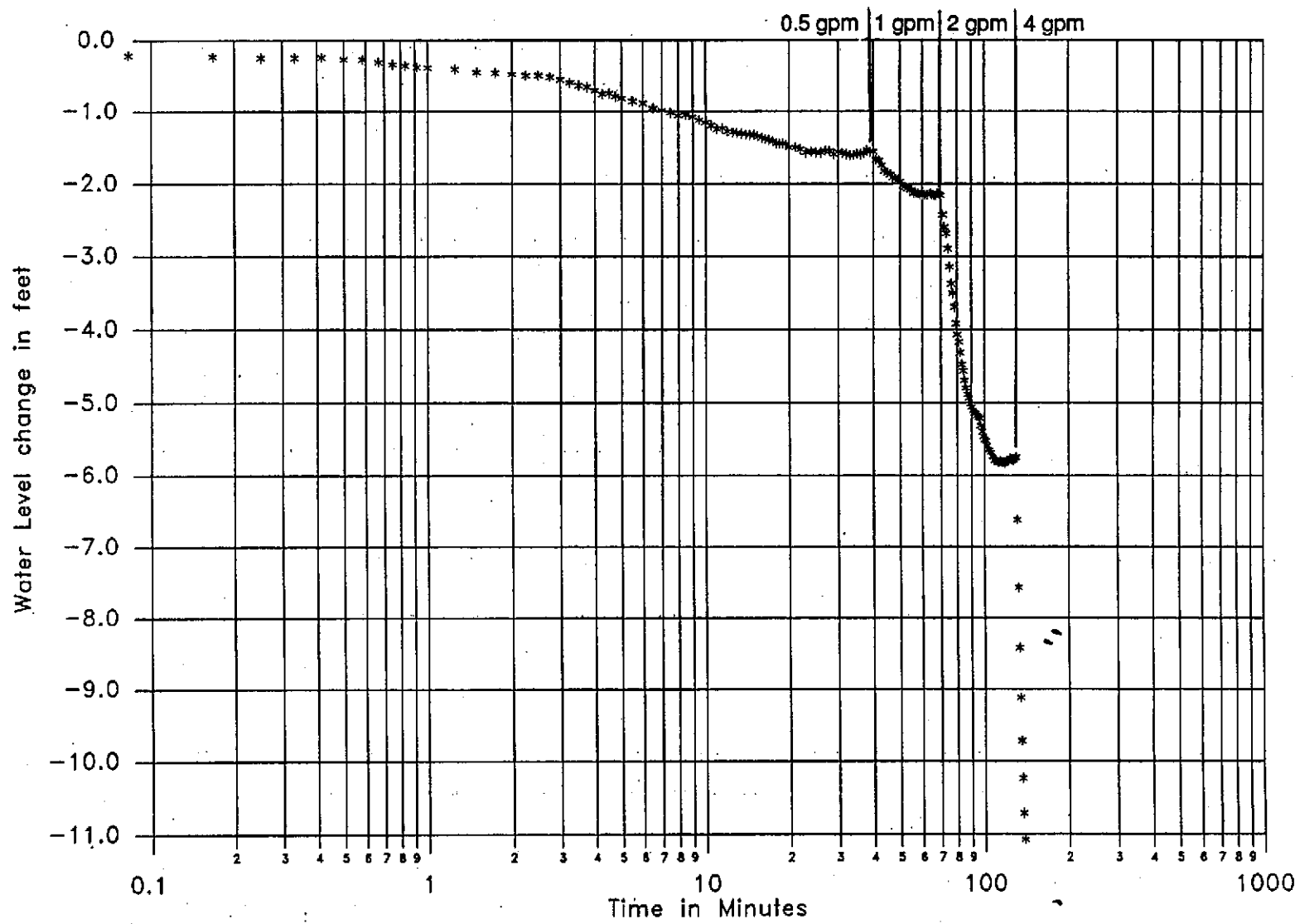
**ATTACHMENT A**  
**AQUIFER TEST DATA AND FIGURES**

**Table A-1  
Pump Test Data Summary**

ARCO Service Station 0608  
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San Lorenzo, California

Well Identification	Step Number	Discharge Rate (gpm)	Duration of Step (minutes)	Drawdown (feet)	Specific Capacity (gpm/feet)
E-1A	1	0.5	40	1.6	0.31
	2	1	30	2.2	0.45
	3	2	60	5.8	0.34
	4	4	10	>11.07	NA
E-1A	Constant Discharge	2	480	5.4	0.37
MW-10	1	0.5	30	0.30	1.67
	2	1	40	0.38	2.63
	3	2	60	0.83	2.41
	4	4	90	3.5	1.14
	5	6	10	>11.22	NA
MW-10	Constant Discharge	4	480	3.8	1.05

gpm = Gallons per minute  
NA = Not available

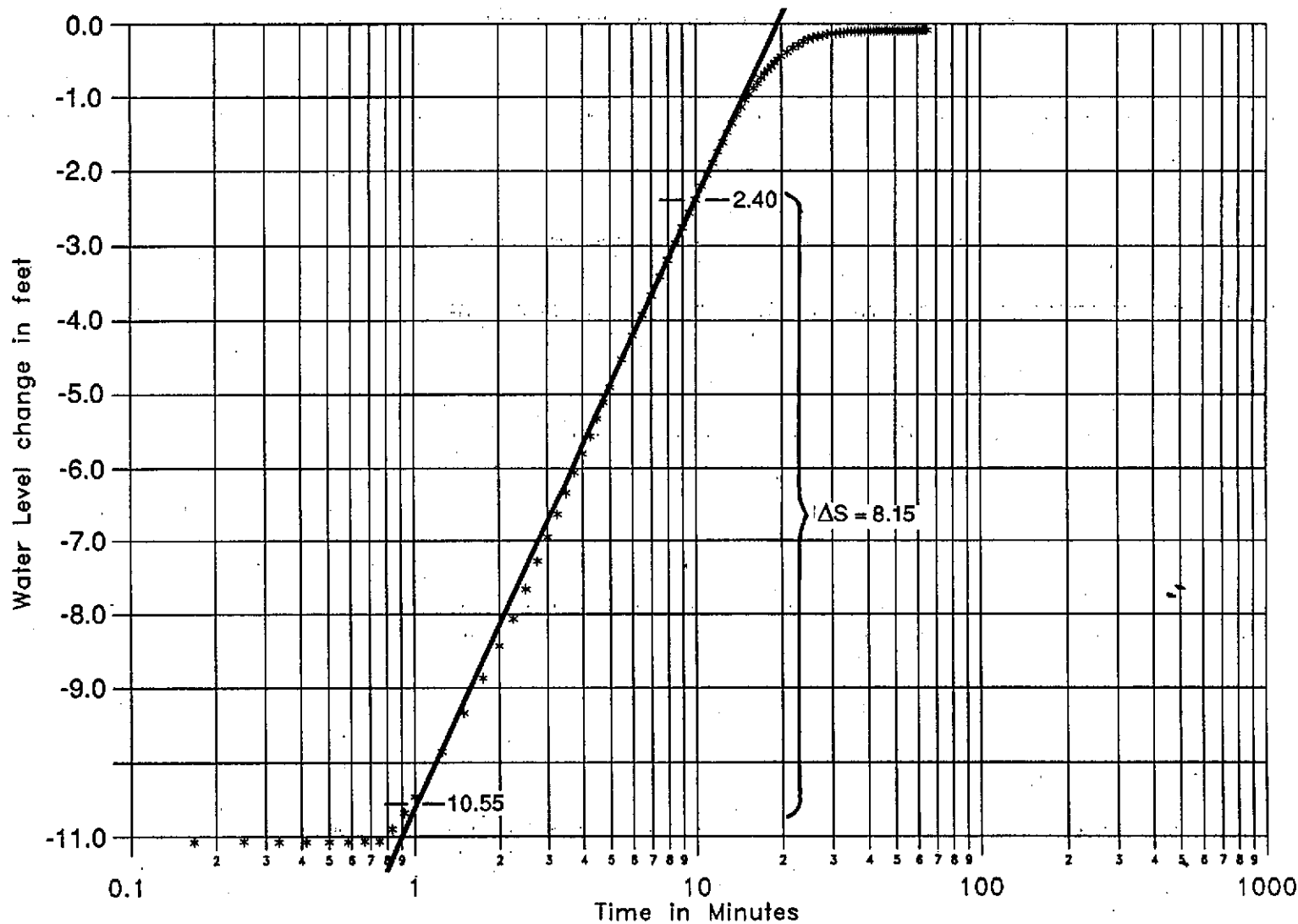


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STEP DISCHARGE TEST IN WELL E-1A

FIGURE:  
**A-1**  
PROJECT:  
330-06.23



JACOB:  
 $T = 264Q$   
 $\frac{\Delta S}{8.15}$   
 $= \frac{264 (4\text{gpm})}{8.15}$   
 $\approx 130 \text{ gpd/ft}$   
 $\approx 17.4 \text{ ft}^2/\text{d}$   
 $K = T/B$   
 $B = 10'$   
 $K = 1.74 \text{ ft/d}$   
 $= 6 \times 10^{-4} \text{ cm/sec}$



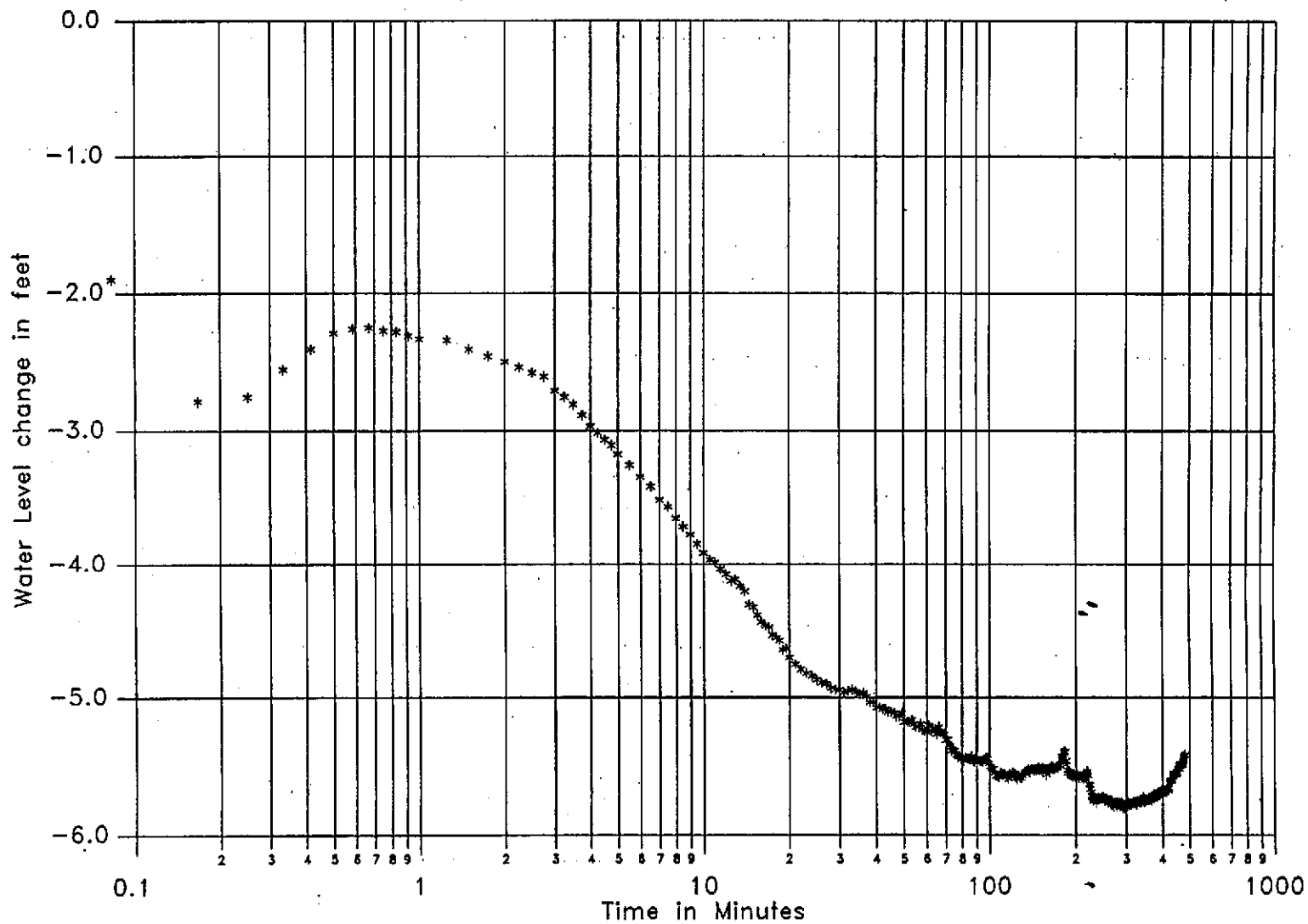
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STEP DISCHARGE TEST RECOVERY IN WELL E-1A

FIGURE:  
**A-2**  
 PROJECT:  
 330-06.23



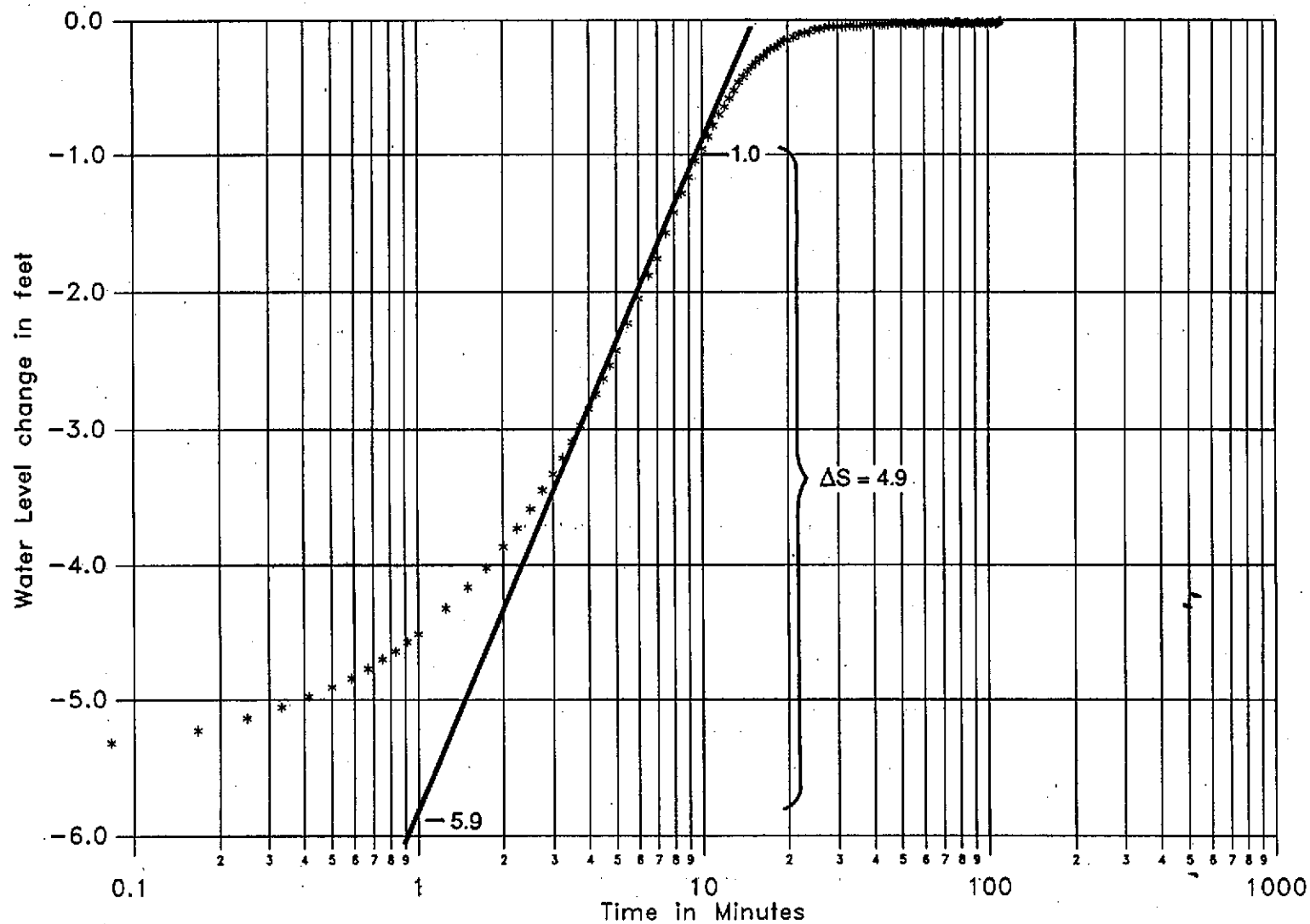


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CONSTANT DISCHARGE TEST IN WELL E-1A

FIGURE:  
**A-3**  
PROJECT:  
330-06.23



JACOB:  
 $T = 264Q$   
 $\frac{\Delta s}{4.9}$   
 $= \frac{264 (2 \text{ gpm})}{4.9}$   
 $= 100 \text{ gpd/ft}$   
 $= 13 \text{ ft}^2/\text{d}$   
 $K = T/B$   
 $B = 10'$   
 $K = 1.3 \text{ ft/d}$   
 $= 4.7 \text{ E-4 cm/sec}$

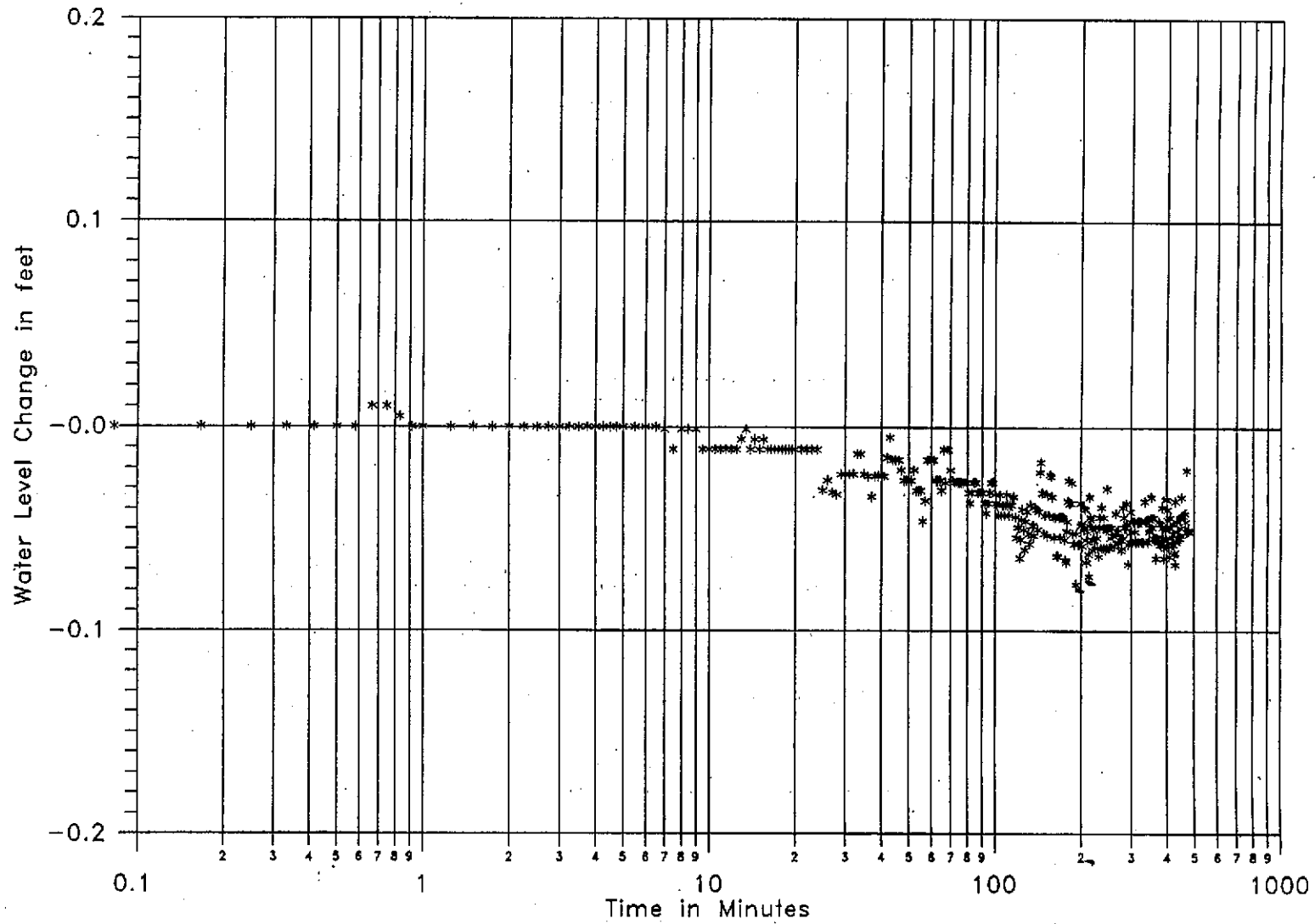


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CONSTANT DISCHARGE TEST RECOVERY PLOT IN WELL E-1A

FIGURE:  
**A-4**  
 PROJECT:  
 330-06.23

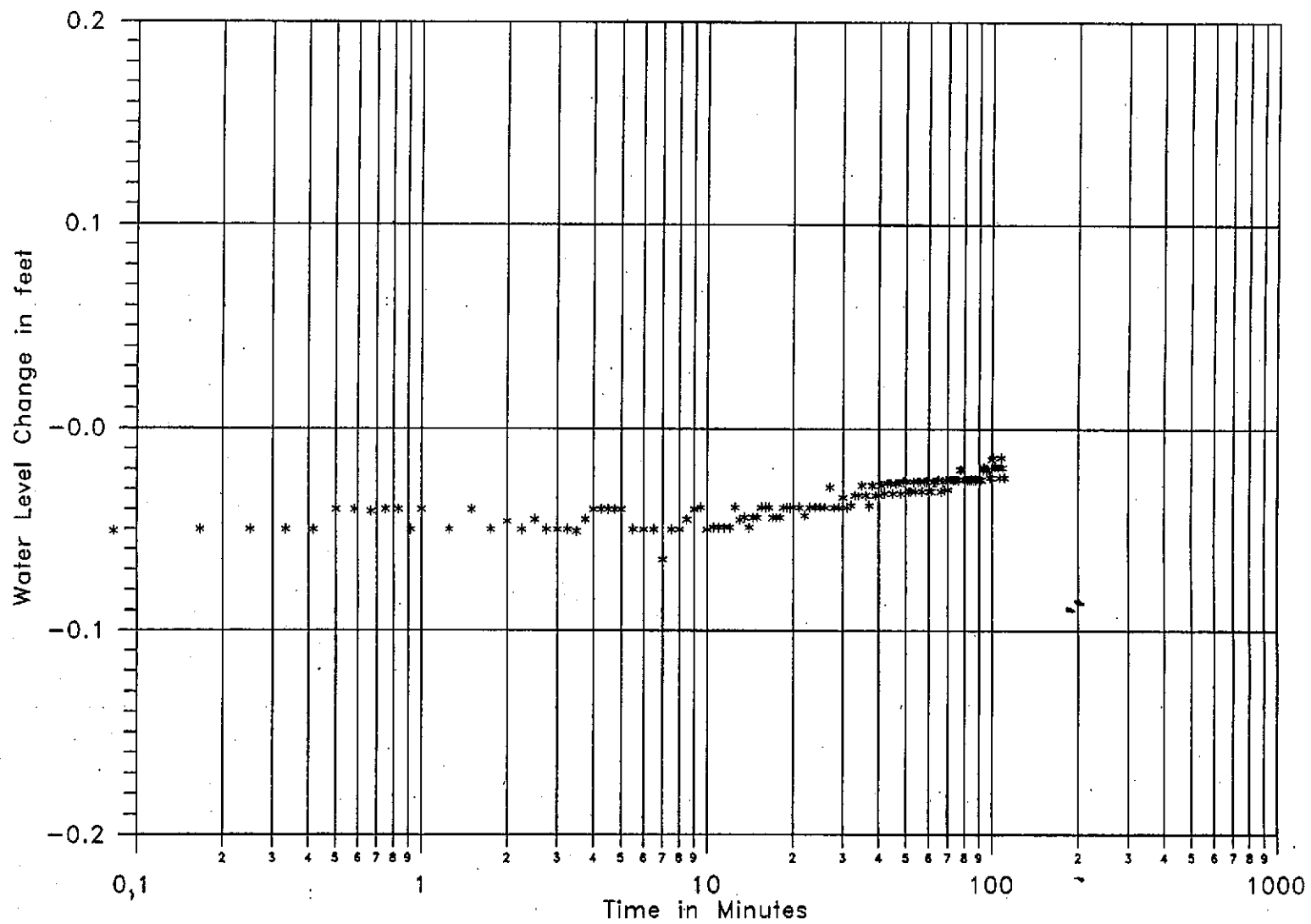


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DRAWDOWN IN WELL V-4 DURING THE CONSTANT DISCHARGE TEST IN WELL E-1A

FIGURE:  
**A-5**  
PROJECT:  
330-06.23

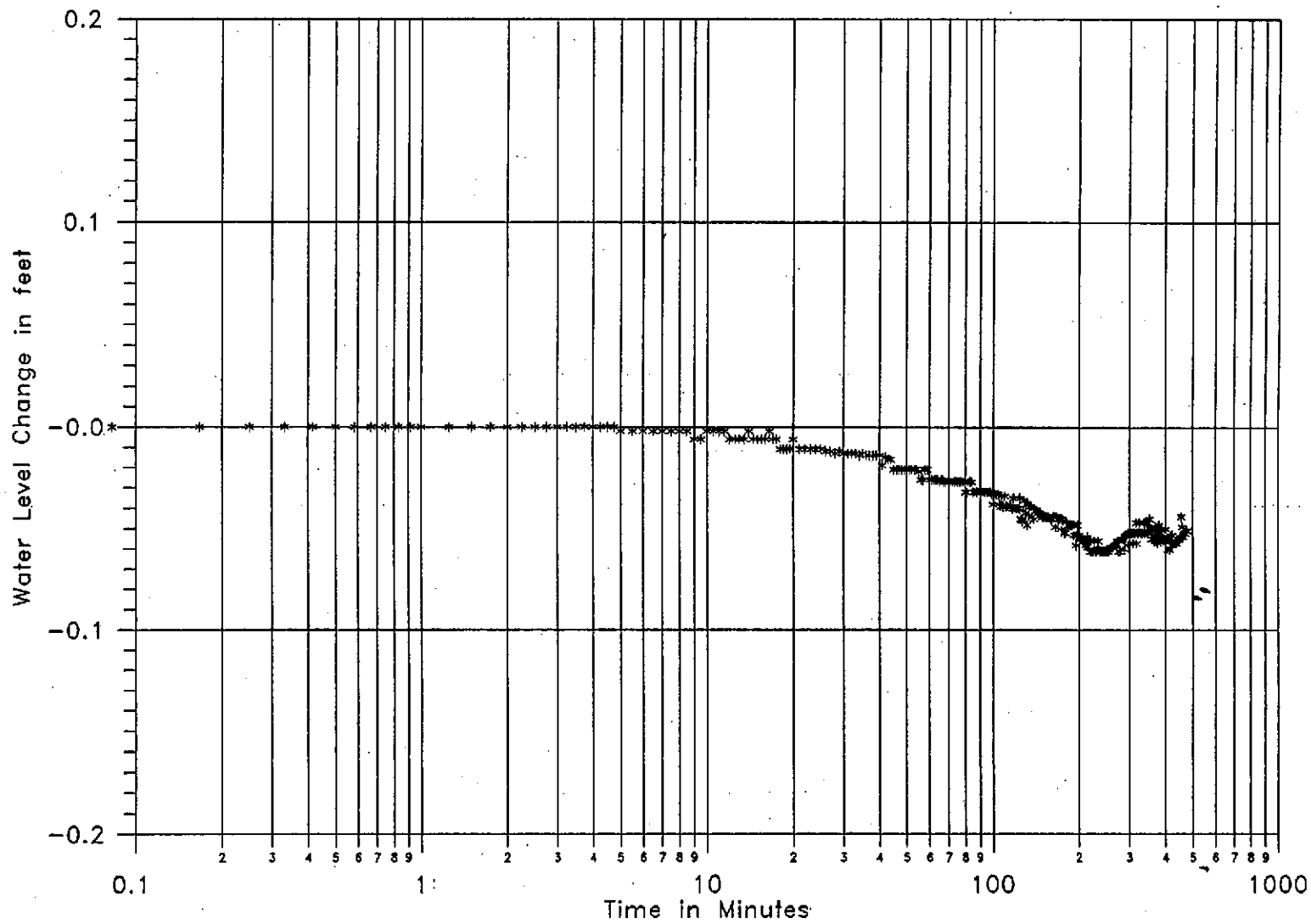


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DRAWDOWN IN WELL V-4 DURING THE CONSTANT DISCHARGE TEST RECOVERY IN WELL E-1A

FIGURE:  
**A-6**  
PROJECT:  
330-06.23

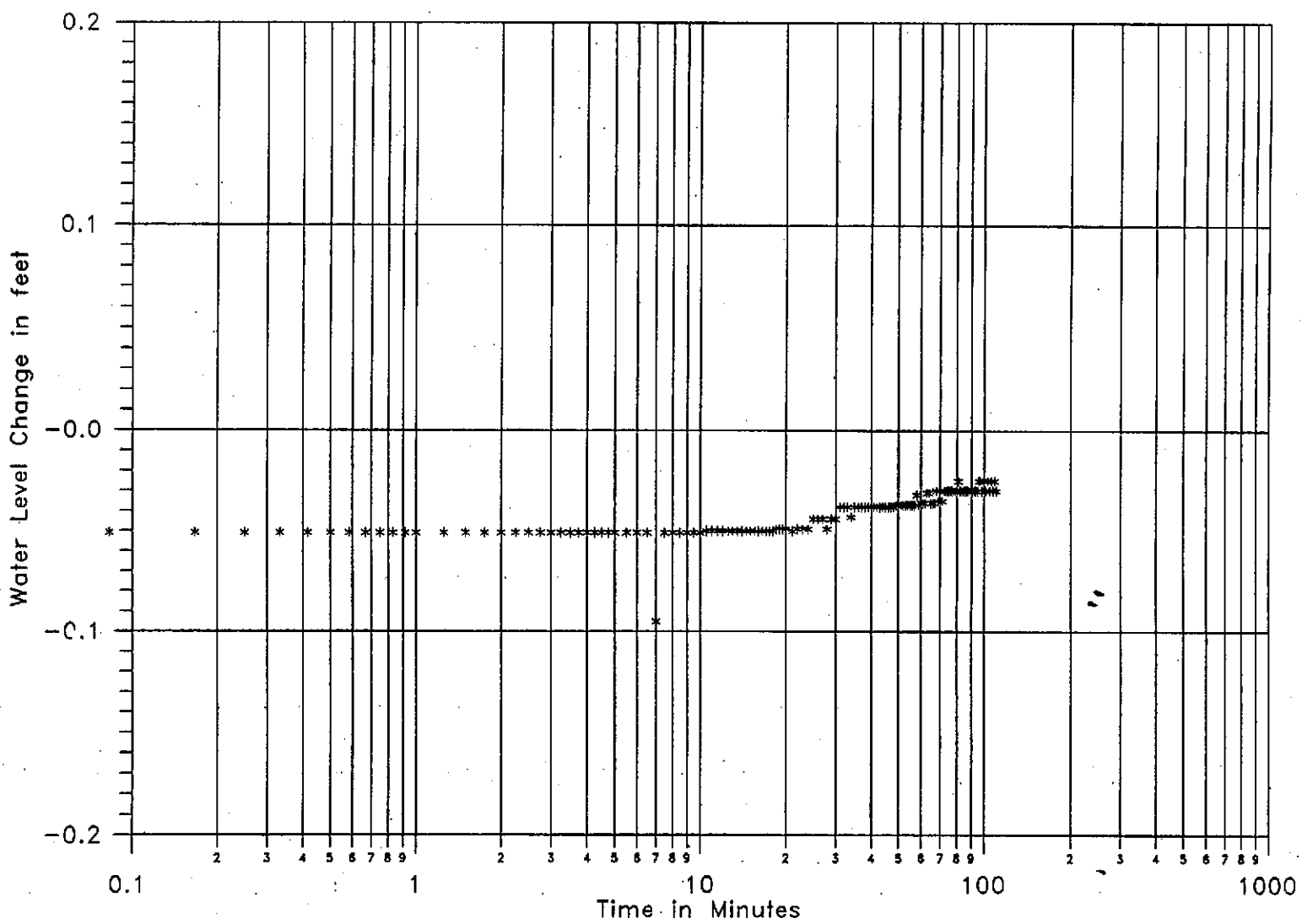


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DRAWDOWN IN WELL MW-5 DURING THE CONSTANT DISCHARGE TEST RECOVERY IN WELL E-1A

FIGURE:  
**A-7**  
PROJECT:  
330-06.23

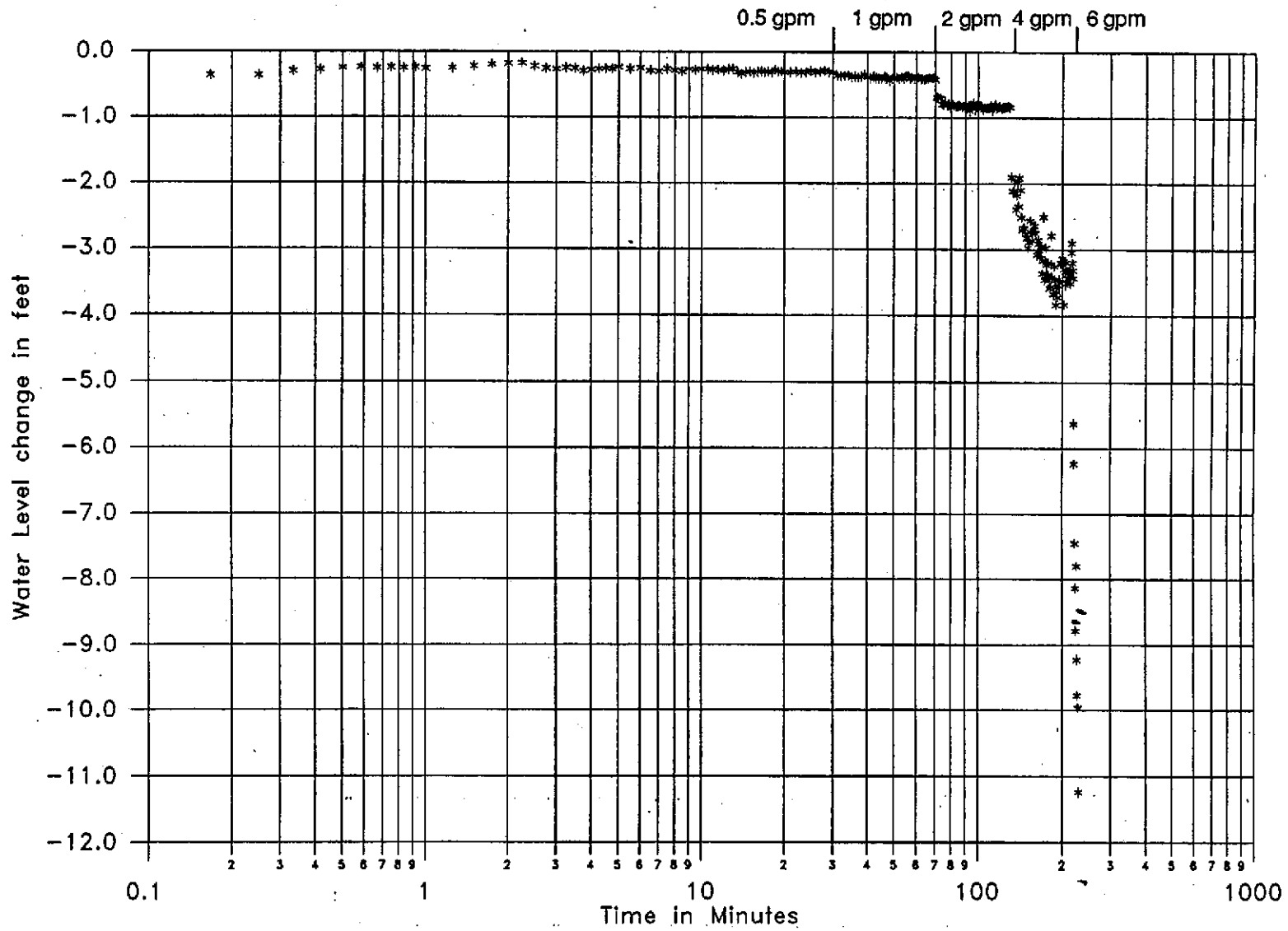


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DRAWDOWN IN WELL MW-5 DURING THE CONSTANT DISCHARGE TEST RECOVERY IN WELL E1-A

FIGURE:  
**A-8**  
 PROJECT:  
 330-06.23

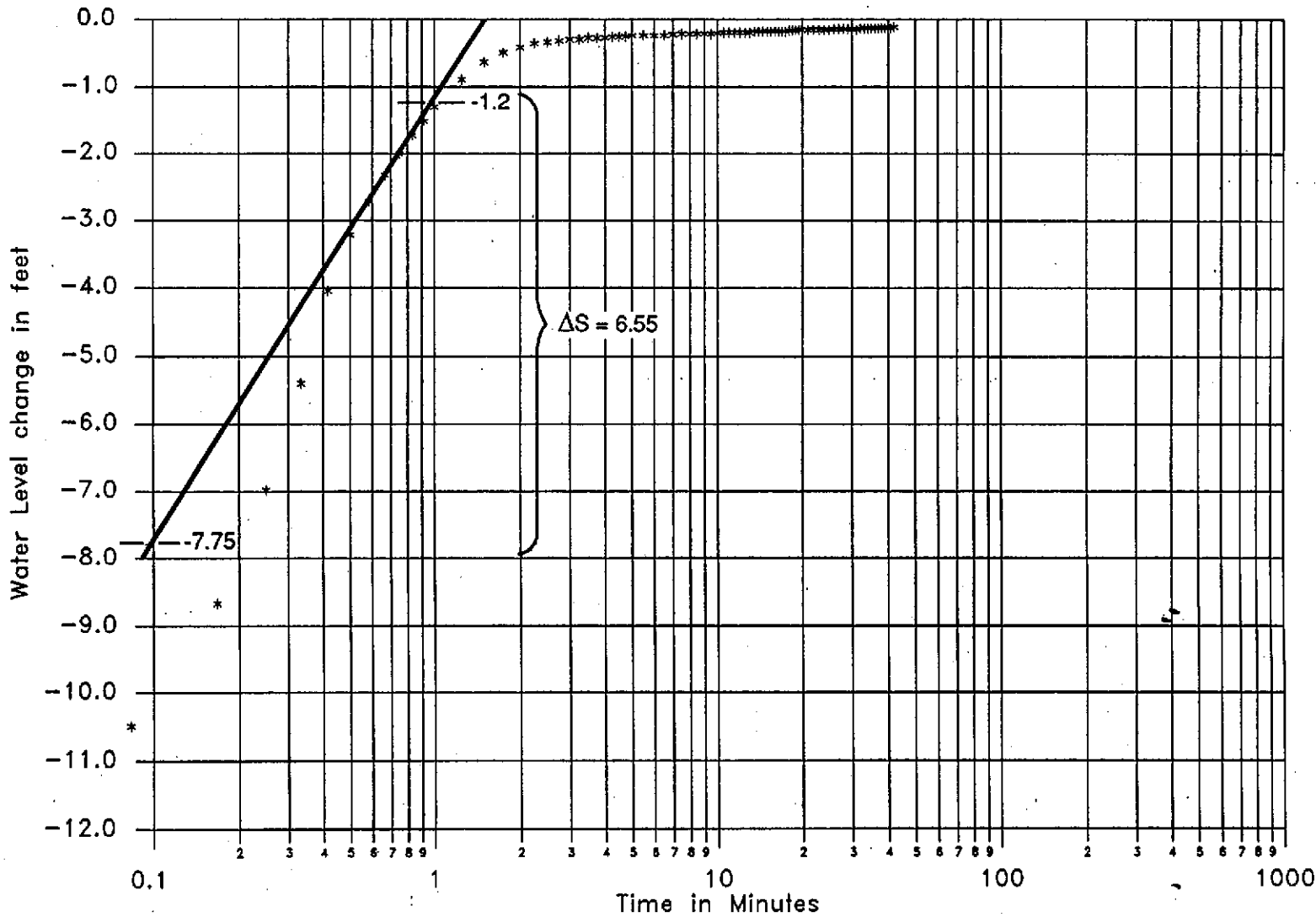


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STEP DISCHARGE TEST IN WELL MW-10

FIGURE:  
**A-9**  
PROJECT:  
330-06.23



JACOB:

$$T = \frac{264Q}{\Delta s}$$

$$= \frac{264 (6)}{(6.55)}$$

$$= 242 \text{ gpd/ft}$$

$$= 35 \text{ gpd/ft}^2$$

$K = T/B$

$B = 7'$

$$K = 4.6 \text{ ft/d}$$

$$= 1.6 \text{ E-3 cm/sec}$$



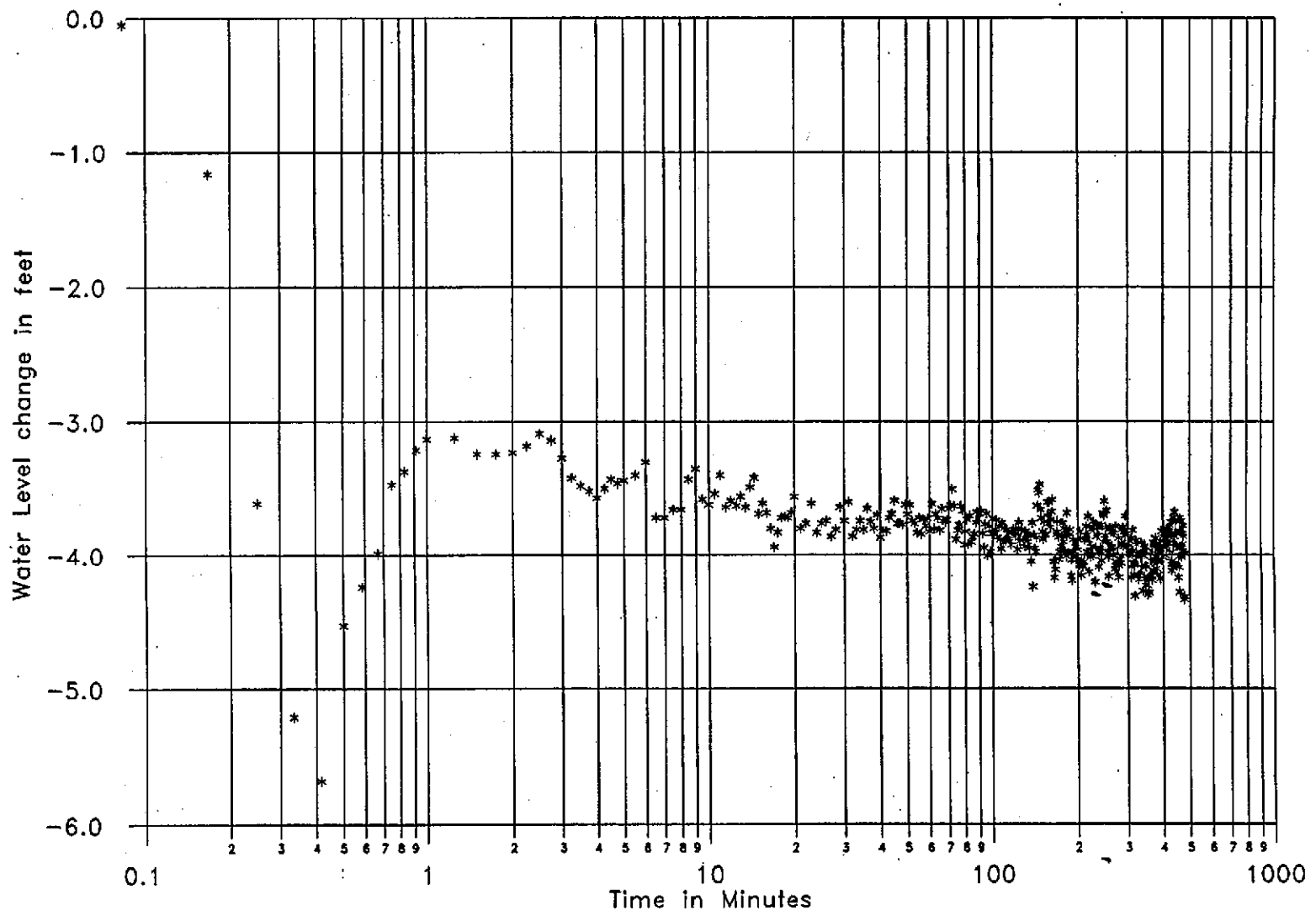
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STEP DISCHARGE TEST RECOVERY IN WELL MW-10

FIGURE:  
**A-10**  
PROJECT:  
330-06.23



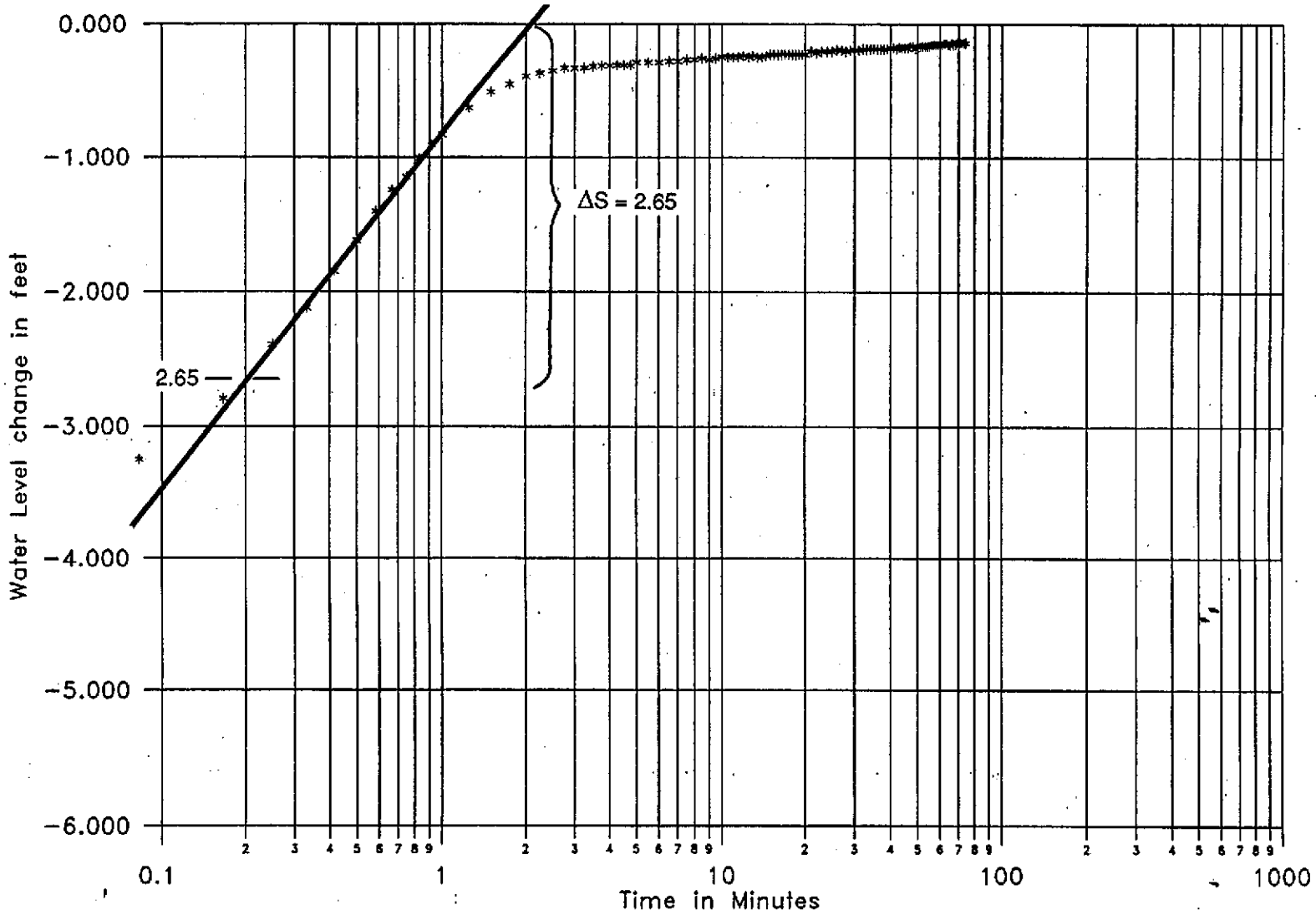


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CONSTANT DISCHARGE TEST IN WELL MW-10

FIGURE:  
**A-11**  
 PROJECT:  
 330-06.23



JACOB:

$$T = \frac{264Q}{\Delta s}$$

$$= \frac{264(4)}{2.65}$$

$$= 400 \text{ gpd/ft}$$

$$= 53 \text{ ft}^2/\text{d}$$

$K = T/B$

$B = 7'$

$$K = 7.6 \text{ ft/d}$$

$$= 2.7 \text{ E-3 cm/sec}$$

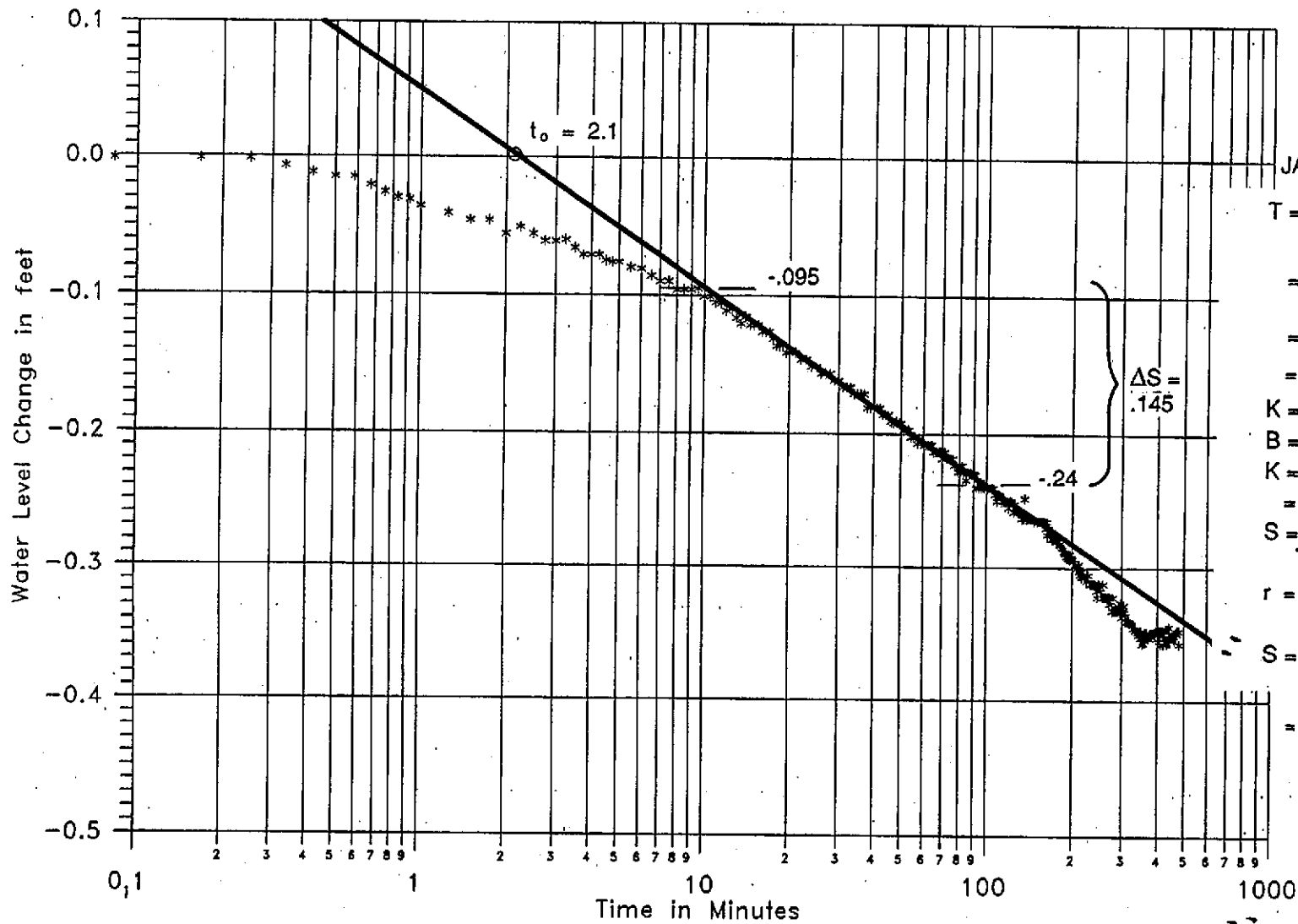


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CONSTANT DISCHARGE TEST RECOVERY IN WELL MW-10

FIGURE:  
**A-12**  
PROJECT:  
330-06.23



JACOB:

$$T = \frac{264Q}{\Delta s}$$

$$= \frac{264(4)}{(.145)}$$

$$= 7.5 \text{ E3 gpd/ft}$$

$$= 970 \text{ ft}^2/\text{d}$$

$$K = T/B$$

$$B = 7$$

$$K = 140 \text{ ft/d}$$

$$= 5 \text{ E-2 cm/sec}$$

$$S = \frac{2.25 T t_0}{r^2}$$

$$r = 16' t_0 = 2.1 \text{ min}$$

$$S = \frac{2.25 (970 \text{ ft}^2/\text{d}) \left( \frac{2.1}{1440} \text{ d} \right)}{(16 \text{ ft})^2}$$

$$= 1.2 \text{ E-2 (no units)}$$



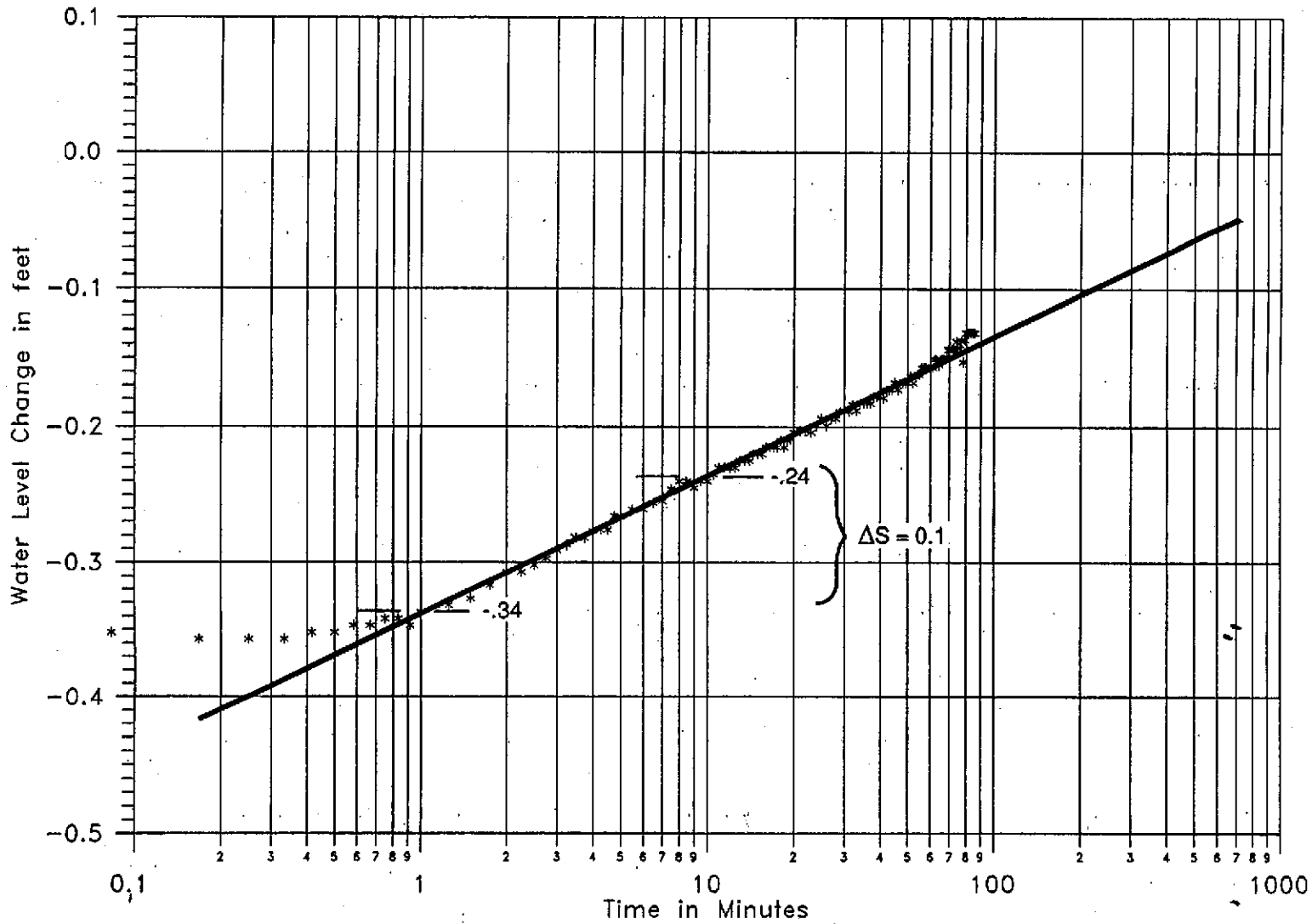
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DRAWDOWN IN WELL SP-2 DURING THE CONSTANT DISCHARGE TEST IN WELL MW-10

FIGURE:  
**A-13**

PROJECT:  
330-06.23



JACOB:

$$T = \frac{264Q}{\Delta s}$$

$$= \frac{264(4)}{0.1}$$

$$= 1 \text{ E}4 \text{ gpd/ft}$$

$$= 1,400 \text{ ft}^2/\text{d}$$

$$K = T/B$$

$$B = 7'$$

$$K = 200 \text{ ft/d}$$

$$= 7 \text{ E-}2 \text{ cm/sec}$$

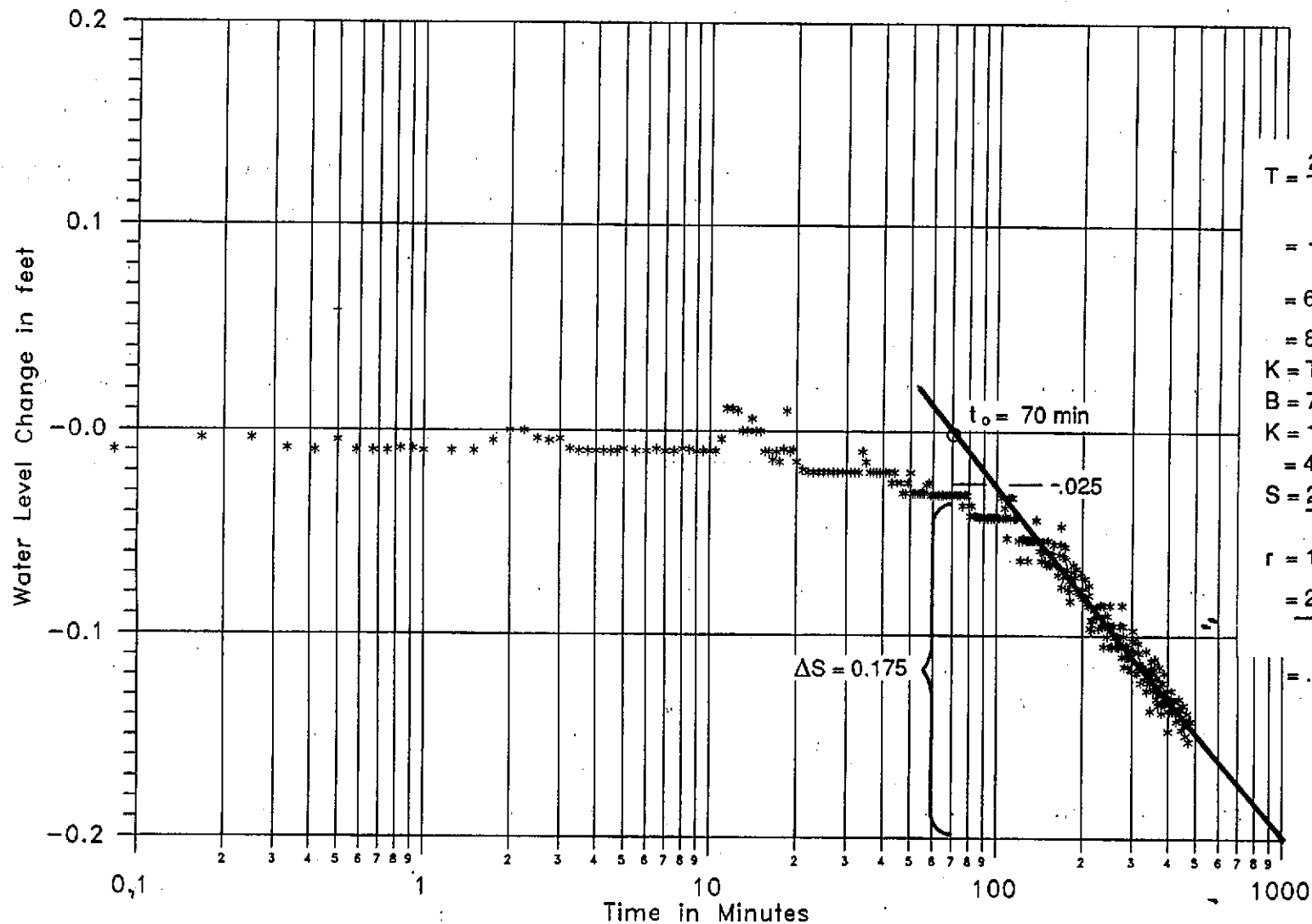


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DRAWDOWN IN WELL SP-2 DURING THE CONSTANT DISCHARGE RECOVERY IN WELL MW-10

FIGURE:  
**A-14**  
PROJECT:  
330-06.23



JACOB:

$$T = \frac{264Q}{\Delta S}$$

$$= \frac{264(4)}{0.175}$$

$$= 6 \text{ E}3 \text{ gpd/ft}$$

$$= 806 \text{ ft}^2/\text{d}$$

$$K = T/B$$

$$B = 7'$$

$$K = 115 \text{ ft/d}$$

$$= 4 \text{ E-}2 \text{ cm/sec}$$

$$S = \frac{2.25 T t_0}{r^2}$$

$$r = 16' \quad t_0 = 70 \text{ min}$$

$$= \frac{2.25 (806 \text{ ft}^2/\text{d}) \left( \frac{t_0}{1,440} \text{ d} \right)}{(16 \text{ ft})^2}$$

$$= .34$$

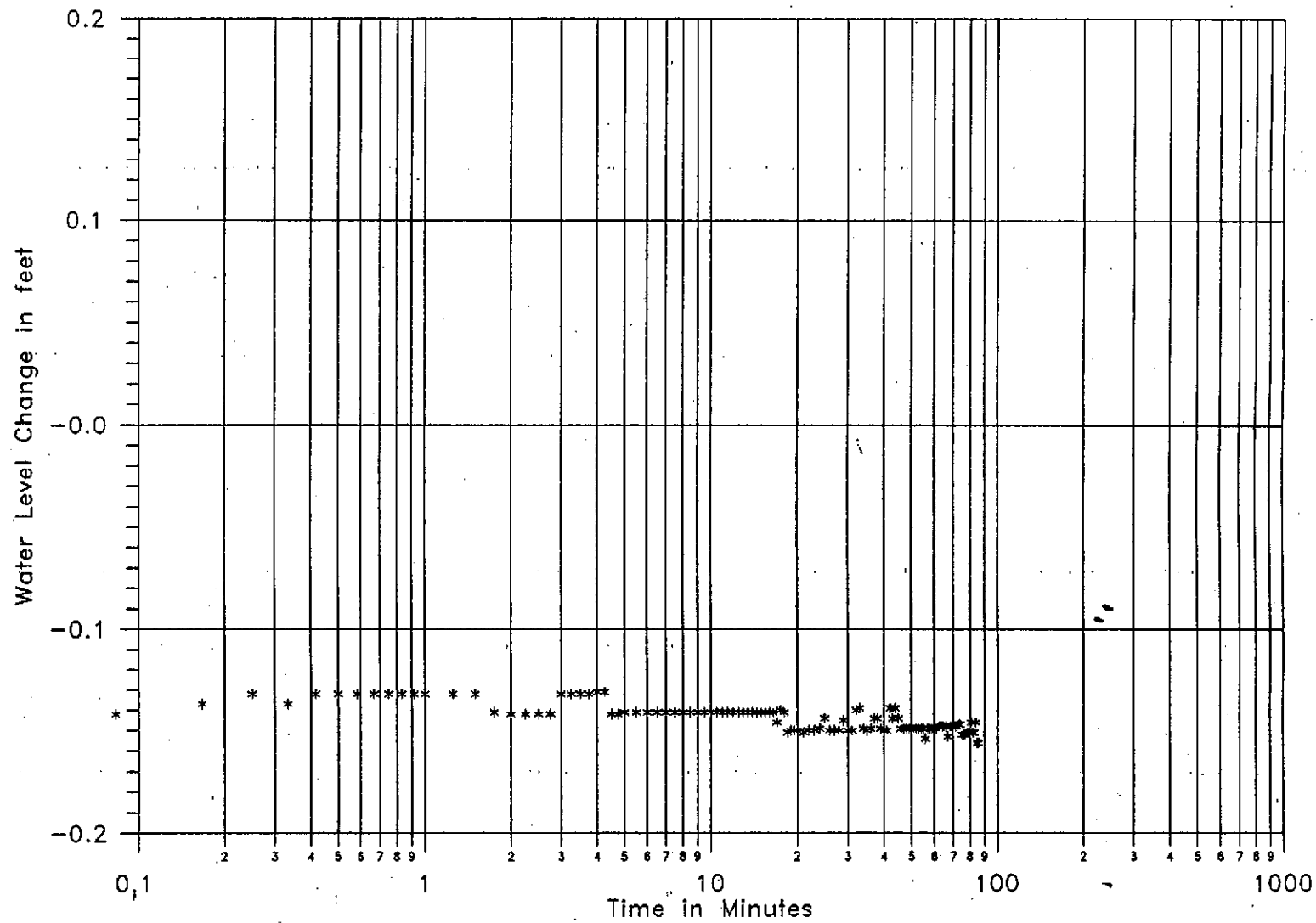


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DRAWDOWN IN WELL V-5 DURING THE CONSTANT DISCHARGE TEST IN WELL MW-10

FIGURE: A-15  
PROJECT: 330-06.23

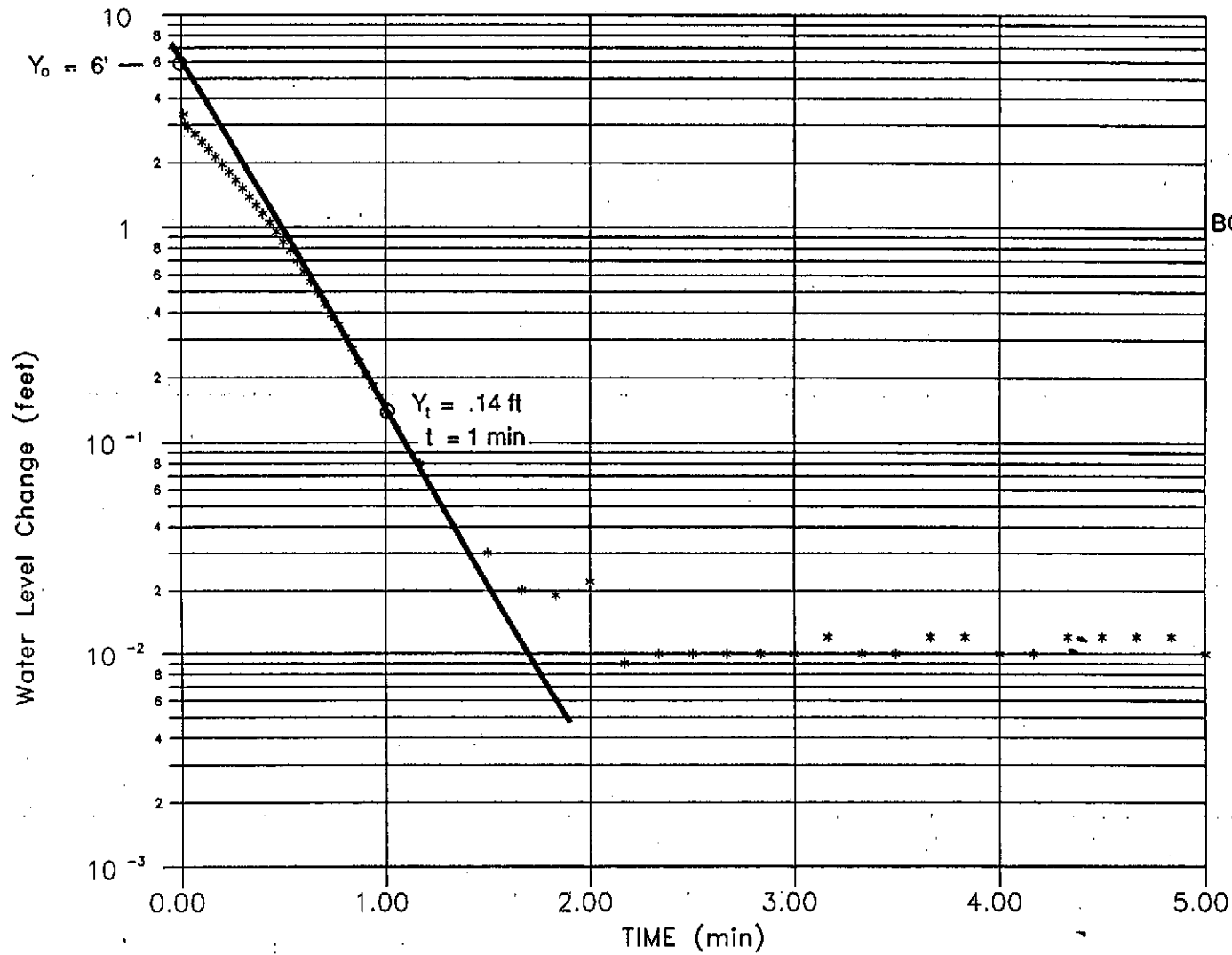


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DRAWDOWN IN WELL V-5 DURING THE CONSTANT DISCHARGE RECOVERY IN WELL MW-10

FIGURE:  
**A-16**  
PROJECT:  
330-06.23



**BOWER PARAMETERS**  
(equation in text)

$r_c = 0.125 \text{ ft}$

$r_w = 0.33 \text{ ft}$

$D = 15 \text{ ft}$

$L = 15 \text{ ft}$

$Y_0 = 6 \text{ ft}$

$Y_t = 0.14 \text{ ft}$

$t = 1 \text{ min}$

$H = 15 \text{ ft}$

$C = 2.5$



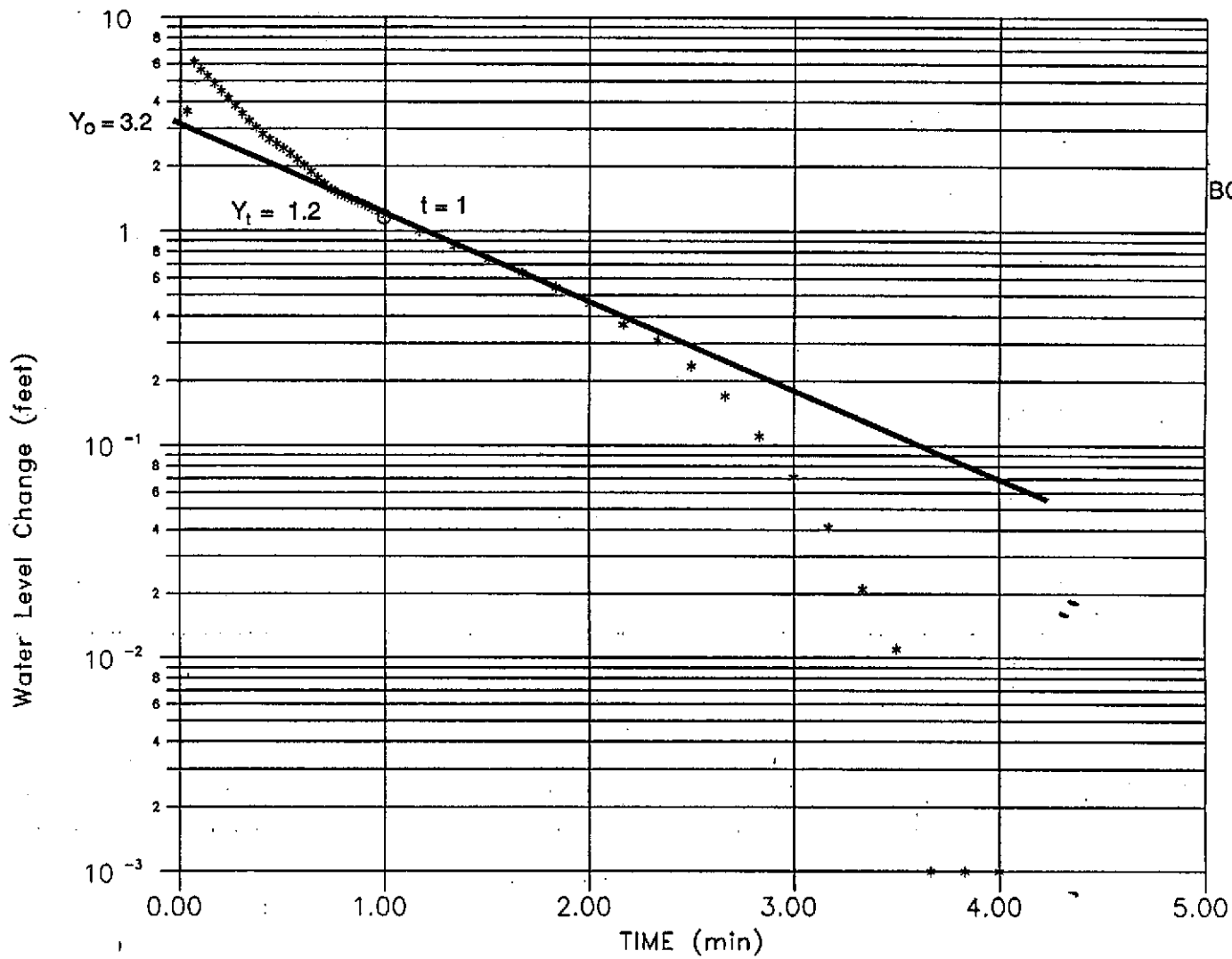
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WELL MW-14 FALLING HEAD TEST #1

FIGURE:  
**A-17**

PROJECT:  
330-06.23



**BOWER PARAMETERS**  
(equation in text)

- $r_c = 0.125$  ft
- $r_w = 0.33$  ft
- $D = 15$  ft
- $L = 15$  ft
- $Y_0 = 3.2$  ft
- $Y_t = 1.2$  ft
- $t = 1$  min
- $H = 15$  ft
- $C = 2.5$



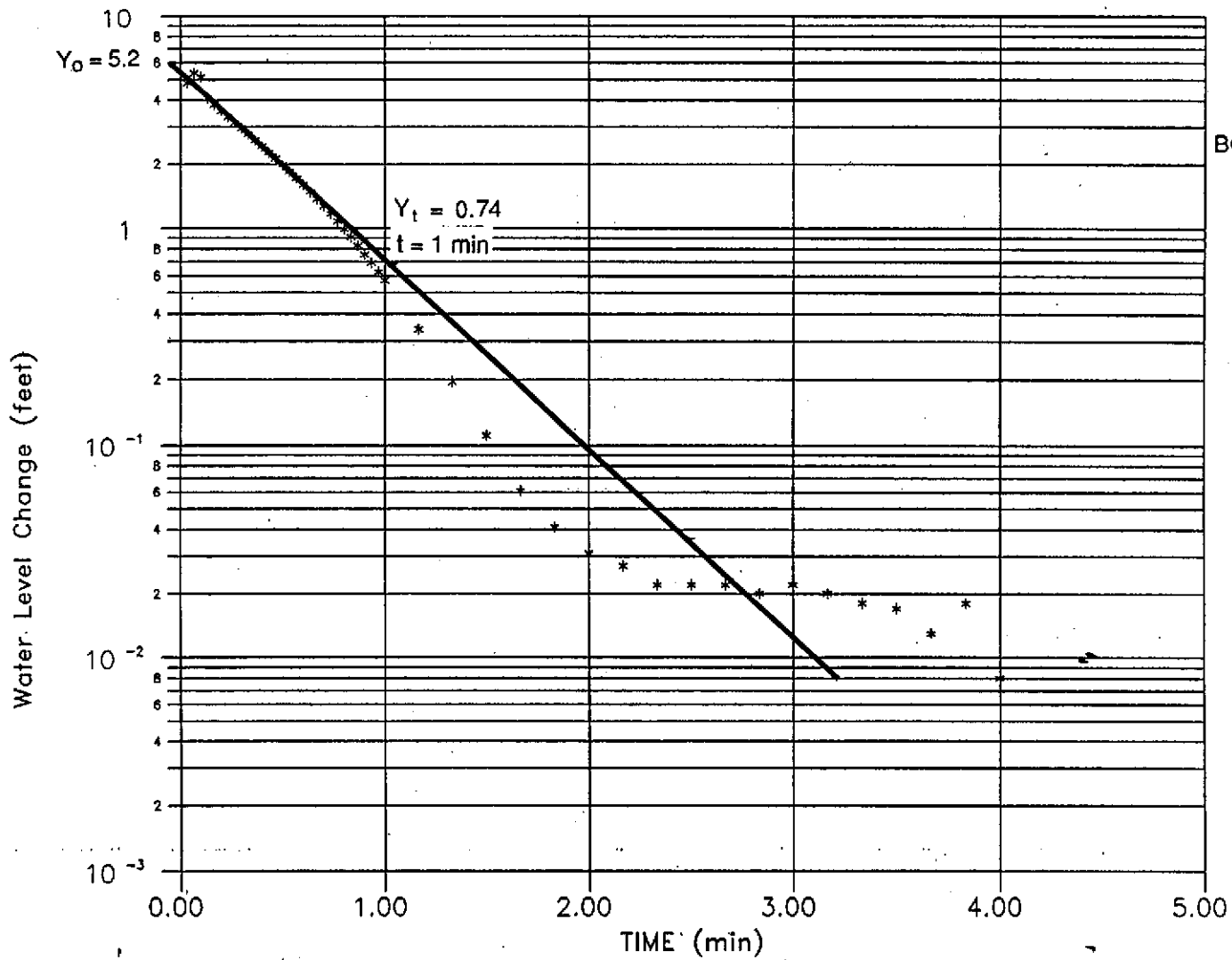
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WELL MW-14 RISING HEAD TEST #1

FIGURE: **A-18**  
PROJECT: 330-06.23





BOWER PARAMETERS  
(equation in text)

$r_c = 0.125 \text{ ft}$

$r_w = 0.33 \text{ ft}$

$D = 15 \text{ ft}$

$L = 15 \text{ ft}$

$Y_0 = 5.2 \text{ ft}$

$Y_t = 0.74 \text{ ft}$

$t = 1 \text{ min}$

$H = 15 \text{ ft}$

$C = 2.5$

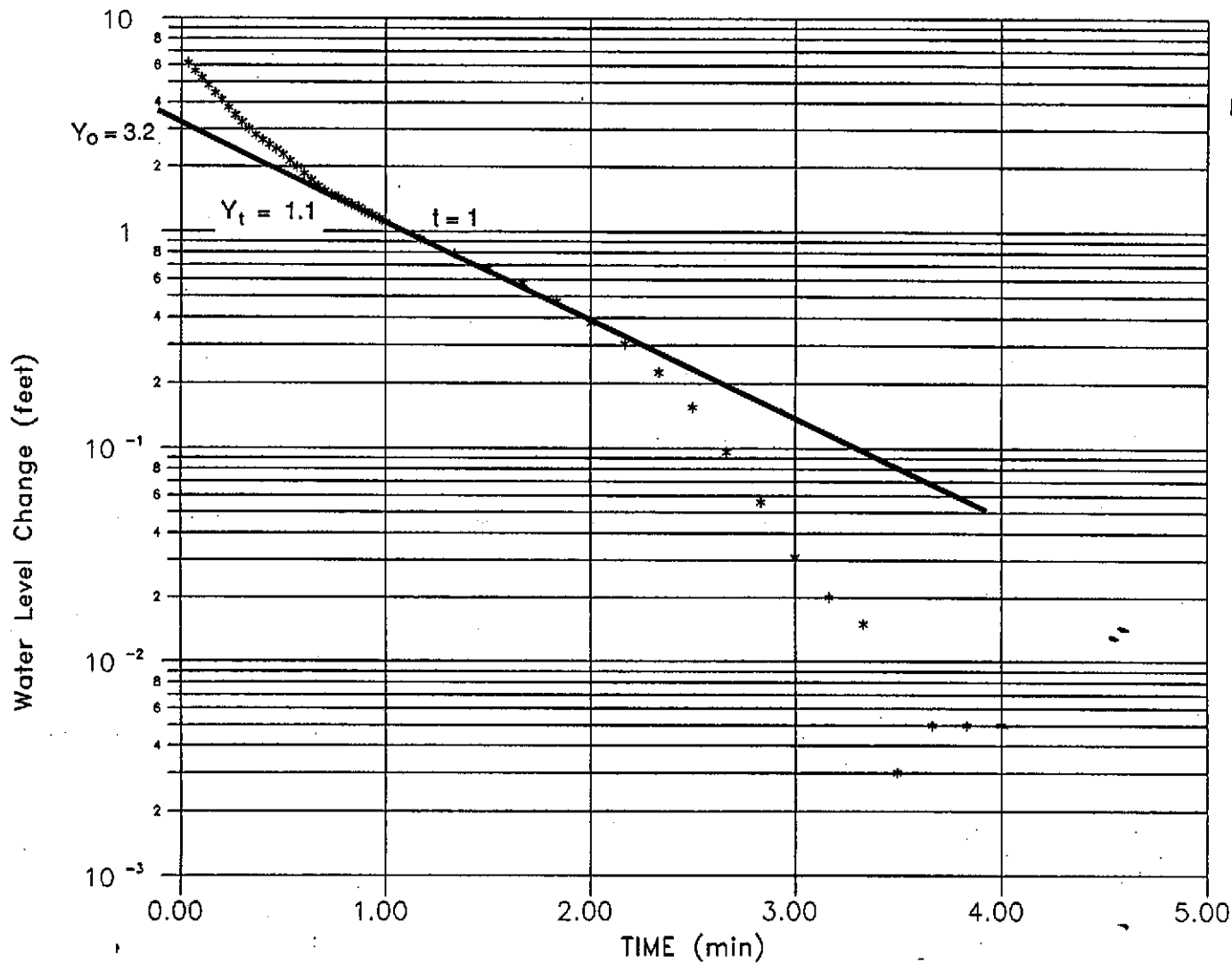


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WELL MW-14 FALLING HEAD TEST #2

FIGURE: A-19  
PROJECT: 330-06.23



**BOWER PARAMETERS**  
(equation in text)

$r_c = 0.125$  ft

$r_w = 0.33$  ft

$D = 15$  ft

$L = 15$  ft

$Y_0 = 3.2$  ft

$Y_t = 1.1$  ft

$t = 1$  min

$H = 15$  ft

$C = 2.5$

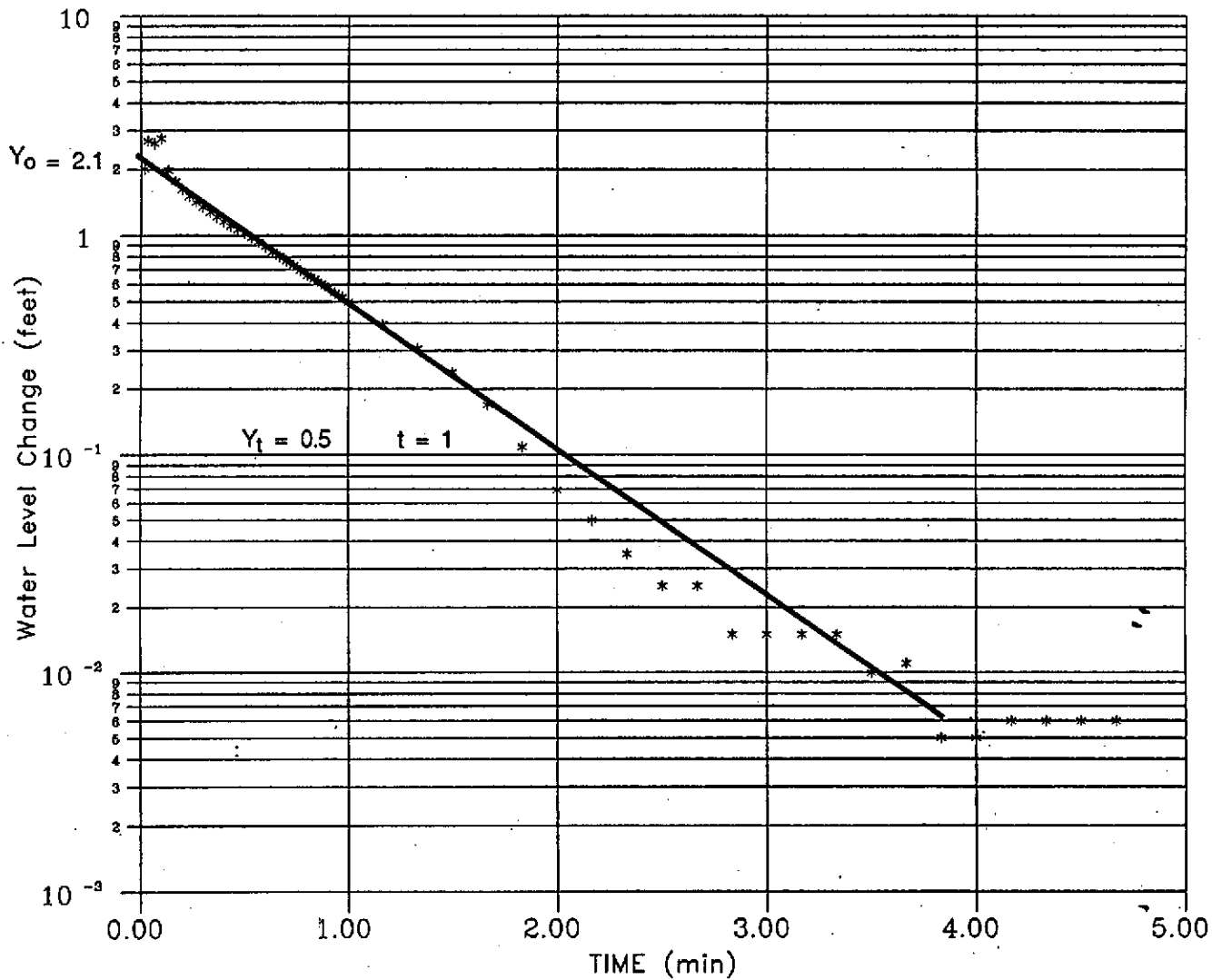


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WELL MW-14 RISING HEAD TEST #2

FIGURE:  
**A-20**  
PROJECT:  
330-06.23



**BOWER PARAMETERS**  
(equation in text)

$r_c = 0.125$  ft

$r_w = 0.33$  ft

$D = 11$  ft

$L = 11$  ft

$Y_0 = 2.1$  ft

$Y_t = 0.5$  ft

$t = 1$  min

$H = 11$  ft

$C = 2$

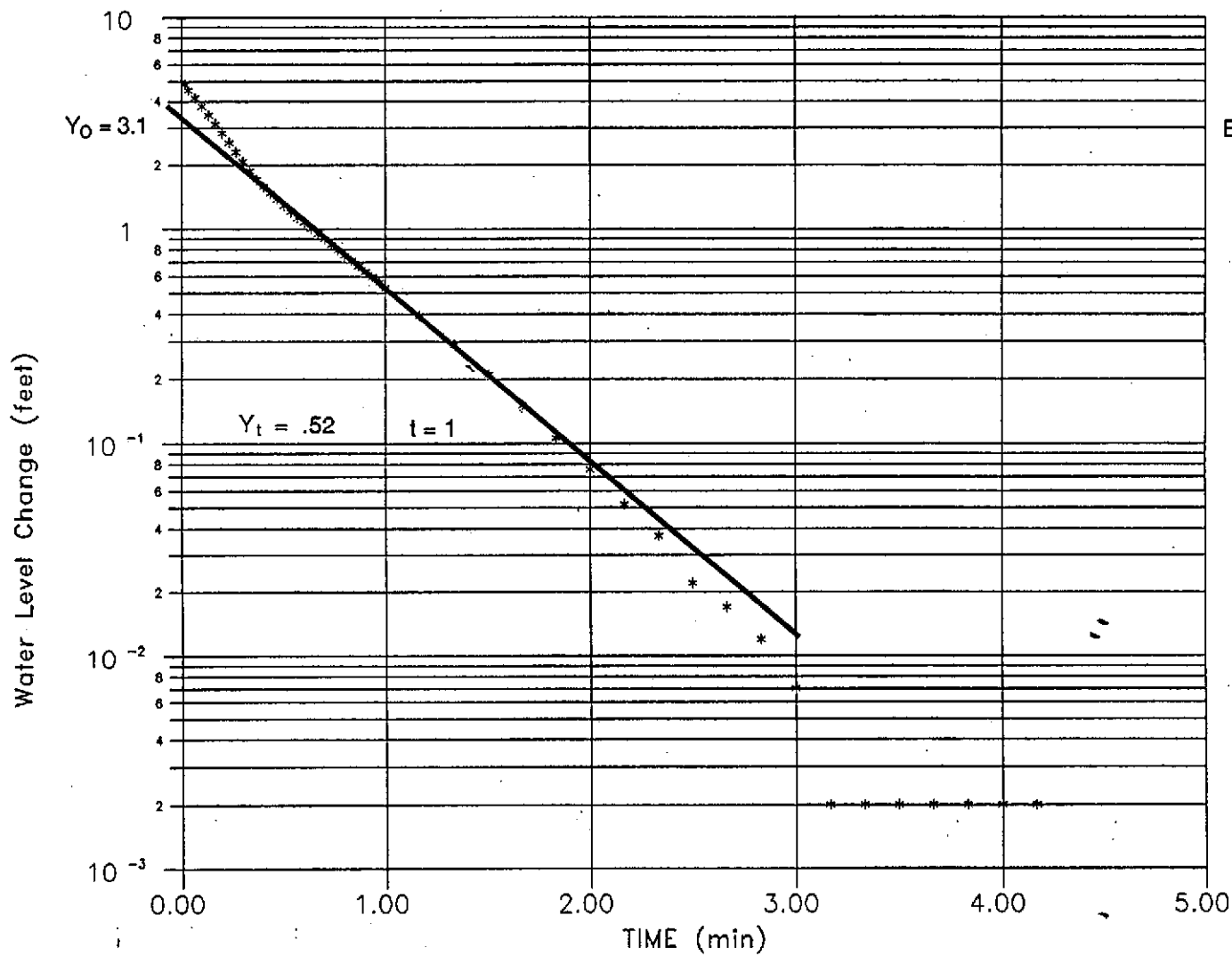


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WELL MW-23 FALLING HEAD TEST

FIGURE:  
**A-21**  
PROJECT:  
330-06.23



**BOWER PARAMETERS**  
(equation in text)

$r_c = 0.125$  ft

$r_w = 0.33$  ft

$D = 11$  ft

$L = 11$  ft

$Y_0 = 3.1$  ft

$Y_t = 0.52$  ft

$t = 1$  min

$H = 11$  ft

$C = 2$



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WELL MW-23 RISING HEAD TEST

FIGURE:  
**A-22**

PROJECT:  
330-06.23

**ATTACHMENT B**

**AIR SPARGE TEST FIELD DATA**

# ON-SITE SPARGE TEST

WELL	DTW	TIME	DISTANCE
V-4	10.95	12:45	0
AS-1	10.92	"	41
MW-5	11.36	"	16
MW-8	9.98	" "	33

## STARTING PRESSURE

DT SCREEN 20'  
 DTW 10.98

9.08' ~ 5 psi

AIR COMPRESSOR @ 20 psi

Helium @ 100 psi

Q = 410



PACIFIC ENVIRONMENTAL GROUP, INC.

Project No:

330-06.19

Figure No:

1/3

Date:

5/4/93

Drawn By:

KW


Title:

ON-SITE SPARGE

WELL	TIME	DO	FID	HE
MW-5	1:30	METER BROKEN ↓	4.5	0.00
	1:35		6	0.00
	1:40		9.5	0.00
	1:45		8.0	0.00
	1:50		6.0	0.00
	1:55		NA	0.00
	2:00		4.0	0.00
	2:05		NA	0.00
	2:10		4.5	0.00
	2:15		NA	0.00
	2:20		30	0.00
	2:25		+10000	0.00
	2:30		+10000	0.00
	2:35		+10000	0.00

WELL	TIME	DO	FID	HE
V-4	1:30		100	0
	1:35		4000	0.26
	1:40		7000	0.50
	1:45		1750	0.66
	1:50		2100	0.69
	1:55		NA	NA
	2:00		1750	0.97
	2:05		2000	0.25
	2:10		NA	NA
	2:15		NA	NA
	2:20		+10000	5.3
	2:25		+10000	NA
	2:30		+10000	4.3
	2:35		+10000	4.9

WELL	TIME	FID	HE
MW-8	1:30	BKG	0.00
	1:35	"	0.00
	1:40	"	0.00
	1:45	"	0.00
	1:50	"	0.00
	1:55	"	0.00
	2:00	"	0.00
	2:05	"	0.00
	2:10	"	0.00
	2:15	"	0.00
	2:20	"	0.00
	2:25	"	0.00
	2:30	"	0.00
	2:35	"	0.00

 PACIFIC ENVIRONMENTAL GROUP, INC.	Project No: 330-06.19	Figure No: 2/3	Date: 5/4/97
	Drawn By: KDW		

Title: OFFSITE SPARGE TEST DATA - HELIUM ONLY

TIME	HE		FID	
	MW-5	N-4	MW-5	V-4
3:25	0	NA		
3:26	2.1	NA	+10000	
3:28	1.5	NA	+10000	
3:30	NA	5.6		+10000
3:34	NA	8.1	+10000	
3:35	4.5	NA		
3:37	2.5	"		
3:39	5.5	"		
3:41	4.7	"		
3:43	NA	9.3		+10000
3:45	NA	8.4		
3:47	8.8	NA		
3:49	NA	8.2		
3:50	9.3	NA	+10000	+10000
3:52	11	"		
3:54	NA	6.9		
3:55	NA	5.9	+10000	+10000
3:57	12	NA		
3:58	11	NA	+10000	+10000
4:00	NA	8.4		
4:02	14	NA		
4:04	NA	7.0		
4:06	11	NA	+10000	+10000
4:08	NA	6.2		
4:10	12	NA		

TEST 2 W/SVE @ MW-5

GRAB P, Q = SAME

SVE P = 40"  
Q = 3

DO METER INOPERATIVE  
DTW IRRELEVANT

HE USAGE

He<sub>0</sub> = 2750  
He<sub>20</sub> = 2200  
He<sub>32</sub> = 1875  
He<sub>37</sub> = 1650  
He<sub>42</sub> = 1275  
He<sub>47</sub> = 1075



PACIFIC ENVIRONMENTAL GROUP, INC.

Project No:

330-06.19

Figure No:

3/3

Date:

5/4/93

Drawn By:

KW

Title:

ON-SITE GRABBE - PHASE 2



SPARGE WELL - AS-2

DTW @ 12:00 = 10.8'

MONITORING WELLS

WELL	DISTANCE FROM AS-2	DTW
V-5	<1'	10.1' *
MW-10	16.5'	9.8'
MW-11	113.8'	10.8'

\* BOTTOM OF CASING

STARTING PRESSURE

$$\begin{aligned} \text{DT SCREEN} &= 18' \\ \text{DTW} &= 10.8' \\ \hline &= 7.2' \text{ STANDING WATER} \end{aligned}$$

$$7.2' = 3.1 \text{ PSI} \Rightarrow \text{MINIMUM REQUIRED}$$

TEST BEGAN AT 1:00.

AIR COMPRESSOR SET AT 20 PSI

HELIUM TANK SET AT 100 PSI

COMBINED PRESSURE AT 10 PSI

FLOW SET AT <10 SCFM

OVER



PACIFIC  
ENVIRONMENTAL  
GROUP, INC.

Project No:

330-06.19

Figure No:

1/3

Date:

5/3/93

Drawn By: xw

Title:


OFF-SITE SPARGE TEST DATA

WELL DIST TIME DTW DO FID HE EST P

WELL	DIST	TIME	DTW	DO	FID	HE	EST	P
AS-2	0	1:00	10.8	NA	NA	NA	<10	10 psi
		1:05	NA	"	"	"	"	7
		1:15	"	"	"	"	"	"
		1:30	"	"	"	"	"	"
V-5	<1	1:00	10.1	2.1	BK6	0.	NA	NA
		1:05	9.4	NA	"	0.16	"	"
		1:15	NA	"	"	0.08	"	"
		1:30	9.83	5.1	"	0.14	"	"
MW-10	16.5	1:00	9.8	1.62	BK6	0	NA	NA
		1:05	9.8	NA	"	"	"	"
		1:15	NA	"	"	"	"	"
		1:30	9.6	3.68	"	"	"	"
MW-11	13.8	1:00	10.8	2.4	BK6	NA	NA	NA
		1:05	NA	NA	"	"	"	"
		1:15	"	"	"	"	"	"
		1:30	10.55	"	"	"	"	"

NOTES:

NO INCREASE IN FID/HE READINGS AFTER 30 MINUTES IN EITHER WELLS - TEST DISCONTINUED.  
 STARTED TEST @ 1:00  
 BK6 ~ 3 PM

 PACIFIC ENVIRONMENTAL GROUP, INC.	Project No: 330-06.19	Figure No: 2/3	Date: 5/3/93
	Drawn By: <i>kw</i>		

Title: HELVUM-ONLY TEST - OFF-SITE SPARGE

SCARBE SVE  
EST Q P Q P

WELL DIST TIME DTW FID HE DO

WELL	DIST	TIME	DTW	FID	HE	DO	EST Q	P	Q	P
AS-2	0	2:00	NA	NA	NA	*	<5	12 Ppi	NA	NA
		2:05	"	"	"	"	"	7	"	"
		2:15	"	"	"	"	"	"	"	"
		2:30	"	"	"	"	"	"	"	"
V-5	<1	2:00	9.83	+2.5	0.14	*	NA	NA	<5	70"
		2:05	9.90	+5.5	0.39	"	"	"	"	"
		2:15	9.98	+7.5	2.7	"	"	"	"	"
		2:30	9.95	+20	3.2	"	"	"	"	"
MW-10	16.5	2:00	9.7	BKG	0	*	NA	NA	NA	NA
		2:05	9.7	"	"	"	"	"	"	"
		2:15	9.58	"	"	"	"	"	"	"
		2:30	9.58	"	"	"	"	"	"	"
MW-11	113.8	2:00	10.54	NA	NA	*	NA	NA	NA	NA
		2:05	10.55	"	"	"	"	"	"	"
		2:15	NA	"	"	"	"	"	"	"
		2:30	10.55	"	"	"	"	"	"	"


NOTES:

NO INCREASE IN MW-10 - TEST DISCONTINUED

STARTED TEST @ 2:00

BKG ~ 3ppm

\* - DO METER FAILED - SAMPLE COULD NOT BE MEASURED


 PACIFIC ENVIRONMENTAL GROUP, INC.  
 Project No: 320-06.19  
 Figure No: 3/3  
 Date: 5/3/93  
 Drawn By: KJW  
 Title: HELIUM/SVE AT AS-2 OFF-SITE SCARBE

**ATTACHMENT C**

**SOIL VAPOR EXTRACTION TEST  
FIELD DATA AND  
COMPUTER MODELLING WORKSHEETS**

TEST WELL - V-5

DTW @ 10:30AM = 9.87

MONITORING WELLS:

WELL	DIST FROM V-5	DTW
MW-10	16.5'	9.63'
MW-11	113.8'	NA

BEGIN TEST AT 11:00AM

SVE PRESSURE @ 40"  
FLOW @ <3 SCFM

TIME	VACUUM PRESSURE	
	MW-10	MW-11
11:00 *	0.00	0.00
11:05	0.01	0.00
11:10	0.01	0.00
11:15 *	0.01	0.00
11:20	0.01	0.00
11:25	0.01	0.00
11:30	0.01	0.00
11:35	0.01	0.00
11:40 *	0.01	0.00
11:45	0.01	0.00

\* SAMPLE COLLECTED FOR 8015/8020 ANALYSIS

PRESSURE APPLICATION ATTEMPTED TO PROVIDE ADDITIONAL DATA - HOSE RUPTURED.

WELL SCREEN NOT SUBMERGED AT VACUUM APPLICATION

POOR RESULTS - STEP TEST NOT CONDUCTED



PACIFIC ENVIRONMENTAL GROUP, INC.

Project No:

530-06.19

Figure No:

Date:

4/29/93

Drawn By:

KW

Title:

OFF-SITE SVE TEST DATA

TEST WELL - V-4

DTW @ 1:30 = 11' EVEN

MONITORING WELLS

WELL	DISTANCE FROM V-5	DTW
MW-5	16'	11.45'
MW-8	33'	9.10'
E1-A	20''	9.85'

BEGIN TEST AT 2:00 PM

SVE PRESSURE @ 40"  
FLOW C < 3 SCFM

TIME	VACUUM MW-5	PRESSURE MW-8	E1-A
2:00 *	0.00	0.00	0.00
2:05	0.00	0.00	0.00
2:10	0.00	0.00	0.00
2:15	0.00	0.00	0.00
2:30	0.00	0.00	0.00
2:45 *	0.00	0.00	0.00

\* SAMPLE COLLECTED FOR 2015/2020 ANALYSIS

PRESSURE APPLICATION ATTEMPTED TO PROVIDE ADDITIONAL  
DATA - HOSE RUPTURED.

WELL SCREEN NOT SUBMERGED AT VACUUM APPLICATION  
POOR RESULTS - STEP TEST NOT CONDUCTED



PACIFIC  
ENVIRONMENTAL  
GROUP, INC.

Project No:

330-06.19

Figure No:

Date:

4/30/13

Drawn By: KW

Title:

ON-SITE SVE

Pacific Environmental Group  
 Project: 330-06.22  
 August 27, 1993

### EFFECTIVE RADIUS OF INFLUENCE

This program is designed to determine an effective radius of influence of a vapor extraction well. Data from feasibility tests or an operating system may be entered. A best fit curve is generated to fit raw field data.

For more detail on this technic please read:

Timothy E. Buscheck, P.E. and Thomas R. Peargin, R.G., November 1991, Proceedings of the Petroleum Hydrocarbons and Organic Chemicals in Groundwater: Prevention, Detection, and Restoration, Houston, Texas Summary of a Nation-Wide Vapor Extraction System Performance Study

n := 1                    Number of monitoring points  
 m := 1                    Number of data points per well  
 i := 0 ..n                Matrix array size for pressure data  
 j := 0 ..m - 1            Matrix array size for number of data points per well  
 P (i,j) = Well vacuum pressure, inches of H2O  
 Pn (i,j) = Normalized well vacuum pressure, inches of H2O  
 R (i) = Radial distance from extraction well to monitoring point, feet

#### FIELD DATA

Well Pressure (inches of water)

Extraction Well

Well MW-10

P     := 40  
 0,j

P     := 0.01  
 1,j

Radial Distance

R :=

i

0.0
17

Pacific Environmental group  
 Project: 330-06.22  
 August 27, 1993

Calculate the normalized vacuum:

$$Pn_{i,j} := \frac{P_{i,j}}{P_{0,j}} \quad Pn = \begin{bmatrix} 1 \\ 2.5 \cdot 10^{-4} \end{bmatrix}$$

Calculate the average values for normalized data:

$$Pave_i := \sum_j \frac{Pn_{i,j}}{m} \quad Pave = \begin{bmatrix} 1 \\ 2.5 \cdot 10^{-4} \end{bmatrix}$$



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 Project: 330-06.22  
 August 27, 1993

LINEAR REGRESSION OF VACUUM DATA

Covert an equation of the form  $Y = e^{ax} + b$  into linear form:

$$\ln(y) = ax + \ln(b)$$

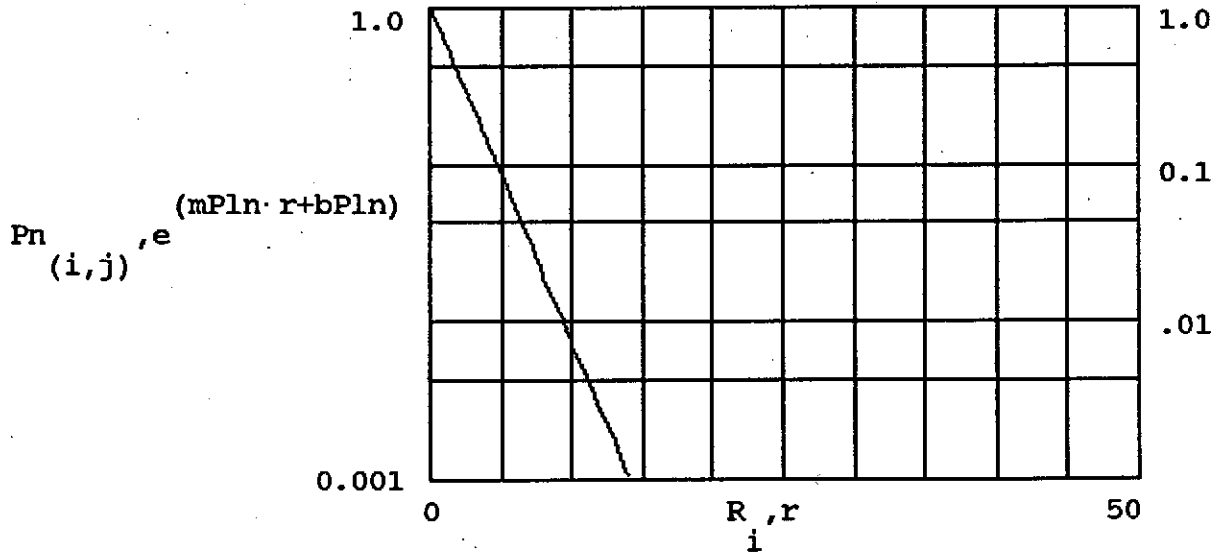
$$\text{Plog}_i := \ln \left[ \begin{matrix} \text{Pave} \\ i \end{matrix} \right] \quad \text{Plog} = \begin{bmatrix} 0 \\ -8.294 \end{bmatrix}$$

Calculate the slope, y - intercept and the correlation coefficient:

mPln := slope(R,Plog)	mPln = -0.488	linear regression slope
bPln := intercept(R,Plog)	bPln = 0	linear regression intercept
rPln := corr(R,Plog)	rPln = -1	correlation coefficient

Plot the field data and the regressed curve in semi-log form:

$$r := 0 \dots 50$$



Calculate the effective radius of influence at 1% of total vacuum:

$$Re := \frac{\ln(0.01) - bPln}{mPln} \quad Re = 9.439 \quad \text{Feet}$$

Project: 330-06.22  
Date: August 27, 1993

This program uses the radial flow equation, to estimate soil permeability given flow rate, well vacuum, radius of influence and well construction data. Once a value for permeability is determined, a plot of flow rate versus vacuum is generated. Field step test data is shown on the same plot for comparison.

Define Major Parameters for extraction well.

$n := 1$  Number of data sets (including step test data)

$i := 0 \dots n - 1$  Range Variable Used for Calculations

$H := 1 \text{ ft}$  Screened Interval

$\mu := 0.000018 \cdot \left[ \frac{\text{kg}}{\text{m} \cdot \text{sec}} \right]$  Air Viscosity

$Patm := 1 \text{ atm}$  Atmospheric Pressure - Absolute

$Rw := 5 \text{ in}$  Well Radius

$Ri := 9.5 \text{ ft}$  Radius of Influence

$Q_i := 3 \text{ cfm}$  Flow Rate

$Pwg_i := 40 \text{ in}_H2O$  Well Pressure - Gauge

Convert Gauge Pressure to Absolute:

$$Pw_i := Patm - Pwg_i$$

Solve Radial Flow Equation for k (permeability)

$$k_{\text{darcy}_i} := \left[ \frac{Q_i}{H} \right] \cdot \left[ \frac{\mu}{\pi} \right] \cdot \frac{\ln \left[ \frac{Rw}{Ri} \right]}{Pw_i \cdot \left[ 1.0 - \left[ \frac{Patm}{Pw_i} \right]^2 \right]}$$

$$\frac{k_{\text{darcy}_i}}{\text{darcy}} = 3.96$$

Note: Permeability in darcies (1 darcy =  $1 \cdot 10^{-8} \text{ cm}^2$ )

Pacific Environmental Group  
 Project: 330-06.22  
 Date: August 27, 1993

Compute Average Permiability, Kave:

$$Kave := \sum_i \frac{k_{darcy_i}}{n} \quad \frac{Kave}{darcy} = 3.96$$

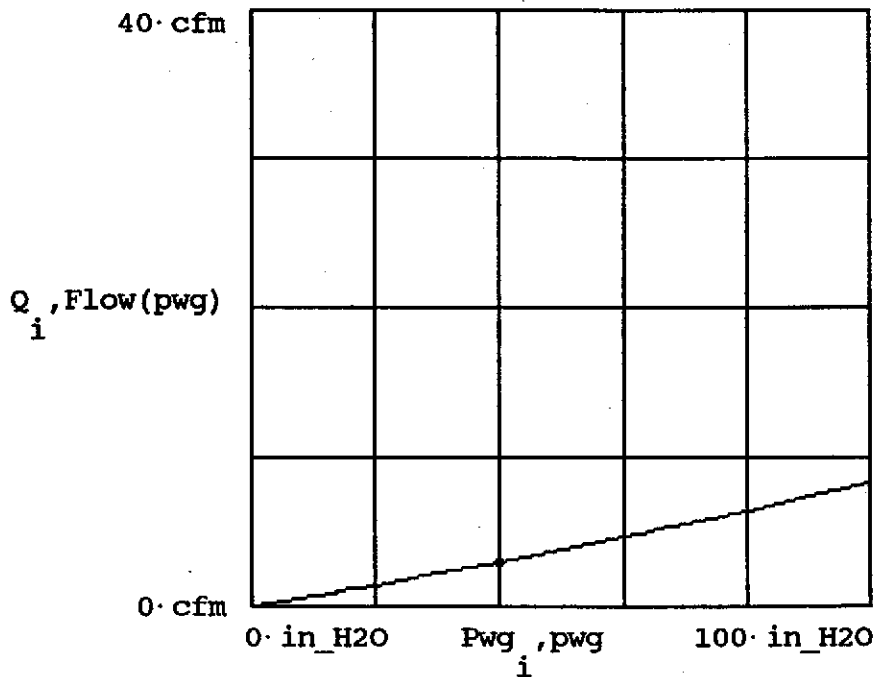
Radial Flow Equation Solved for Flow vs. Vacuum:

$$P(pwg) := Patm - pwg$$

$$Flow(pwg) := \frac{Kave \cdot H \cdot \pi \cdot P(pwg) \cdot \left[ 1.0 - \left[ \frac{Patm}{P(pwg)} \right]^2 \right]}{\mu \cdot \ln \left[ \frac{Rw}{Ri} \right]}$$

Plot field data and theoretical data for Flow (cfm) versus Vacuum (in H2O):

$$pwg := 0 \cdot in\_H2O, 10 \cdot in\_H2O \dots 100 \cdot in\_H2O$$



pwg	Flow(pwg)
in H2O	cfm
0	0
10	0.72
20	1.459
30	2.219
40	3
50	3.805
60	4.636
70	5.496
80	6.386
90	7.31
100	8.271

## UNIT DEFINITIONS

## MKS (SI) unit system

## I. Base units

$m \equiv 1L$

$kg \equiv 1M$

$sec \equiv 1T$

$coul \equiv 1Q$

## II. Angular measure

$rad \equiv 1$

$deg \equiv \frac{\pi}{180} \cdot rad$

## III. Derived units: Length

$cm \equiv .01 \cdot m$

$km \equiv 1000 \cdot m$

$mm \equiv .001 \cdot m$

$ft \equiv .3048 \cdot m$

$in \equiv 2.54 \cdot cm$

$yd \equiv 3 \cdot ft$

$mi \equiv 5280 \cdot ft$

## IV. Derived units: Mass

$gm \equiv 10^{-3} \cdot kg$

$tonne \equiv 1000 \cdot kg$   
(metric ton)

$lb \equiv 453.59247 \cdot gm$   
(use convention that lb represents pounds mass.)

$oz \equiv \frac{lb}{16}$

$ton \equiv 2000 \cdot lb$   
("short" ton)

$slug \equiv 32.174 \cdot lb$

## V. Derived units: Time

$min \equiv 60 \cdot sec$

$hr \equiv 3600 \cdot sec$

$day \equiv 24 \cdot hr$

$yr \equiv 365.2422 \cdot day$  (tropical year)

## VI. Derived units: Area, Volume

$hectare \equiv 10^4 \cdot m^2$

$acre \equiv 4840 \cdot yd^2$

$liter \equiv (.1 \cdot m)^3$   
(Sometimes defined with "L" symbol.)

$mL \equiv 10^{-3} \cdot liter$

$fl\_oz \equiv 29.57353 \cdot cm^3$

$gal \equiv 128 \cdot fl\_oz$

$darcy \equiv 10^{-8} \cdot cm^2$

## VII. Derived units: Velocity, Acceleration

$mph \equiv \frac{mi}{hr}$

$kph \equiv \frac{km}{hr}$

$cfm \equiv \frac{ft^3}{min}$

$g \equiv 9.80665 \cdot \frac{m}{sec^2}$  (acceleration of gravity)

## VIII. Derived units: Force, Energy, Power

$newton \equiv kg \cdot \frac{m}{sec^2}$

$dyne \equiv 10^{-5} \cdot newton$

$lbf \equiv g \cdot lb$   
(pound force)

sec

kgf = g · kg  
(kilogram force)

-----

$$\text{joule} = \text{newton} \cdot \text{m}$$

$$\text{erg} = 10^{-7} \cdot \text{joule}$$

$$\text{BTU} = 1.05505585262 \cdot 10^3 \cdot \text{joule}$$

$$\text{cal} = 4.1868 \cdot \text{joule}$$

$$\text{kcal} = 1000 \cdot \text{cal}$$

-----

$$\text{watt} = \frac{\text{joule}}{\text{sec}}$$

$$\text{kW} = 1000 \cdot \text{watt}$$

$$\text{hp} = 550 \cdot \frac{\text{ft} \cdot \text{lbf}}{\text{sec}}$$

(standard horsepower)

IX. Derived units: Pressure, Viscosity ,

$$\text{Pa} = \frac{\text{newton}}{\text{m}^2}$$

$$\text{psi} = \frac{\text{lbf}}{\text{in}^2}$$

$$\text{atm} = 1.01325 \cdot 10^5 \cdot \text{Pa}$$

$$\text{torr} = 1.33322 \cdot 10^2 \cdot \text{Pa}$$

$$\text{in}_\text{Hg} = 3.38638 \cdot 10^3 \cdot \text{Pa}$$

-----

$$\text{poise} = .1 \cdot \text{Pa} \cdot \text{sec}$$

$$\text{in}_\text{H2O} = \frac{\text{in}_\text{Hg}}{13.596}$$

$$\text{stokes} = 10^{-4} \cdot \frac{\text{m}^2}{\text{sec}}$$

Pacific Environmental Group, Inc.  
 Project: 330-06.22  
 Date: August 27, 1993

CONCENTRATION  
 DECAY

This document shows the results of decay in influent concentrations to a soil vapor extraction system. And calculates the theoretical mass of TPH removed from soils at a site.

Initial conditions:

Initial concentration  $C_0 := 13 \frac{\text{ug}}{\text{l}}$

Half-concentration:  $t_{\text{halfA}} := 50 \cdot \text{day}$

Flow Rate:  $\text{flow} := 9 \frac{\text{cf}}{\text{m}}$

Asymtote: (Percent of initial concentration,  $C_f$ ):  $C_f := 5$

Compute decay constant,  $k_A$ :

$$k_A := \frac{\ln(2)}{t_{\text{halfA}}} \quad k_A = 0.014 \cdot \text{time}^{-1}$$

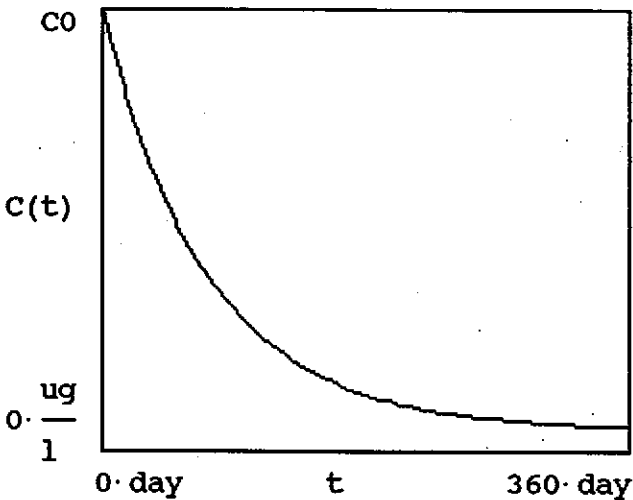
Decay function:

$$C(t) := C_0 \cdot \left[ \left[ 1 - \frac{C_f}{100} \right] \cdot e^{-(k_A \cdot t)} + \frac{C_f}{100} \right]$$

Graph the decay in concentration,  $C$  over time:

$t := 0 \cdot \text{day}, 5 \cdot \text{day} \dots 360 \cdot \text{day}$

$t_2 := 0 \cdot \text{day}, 90 \cdot \text{day} \dots 360 \cdot \text{day}$



$t_2$ day	$C(t_2) \cdot \frac{1}{\text{ug}}$
0	13
90	4.2
180	1.7
270	0.9
360	0.7

C

Pacific Environmental Group, Inc.  
 Project: 330-06.22  
 Date: August 27, 1993

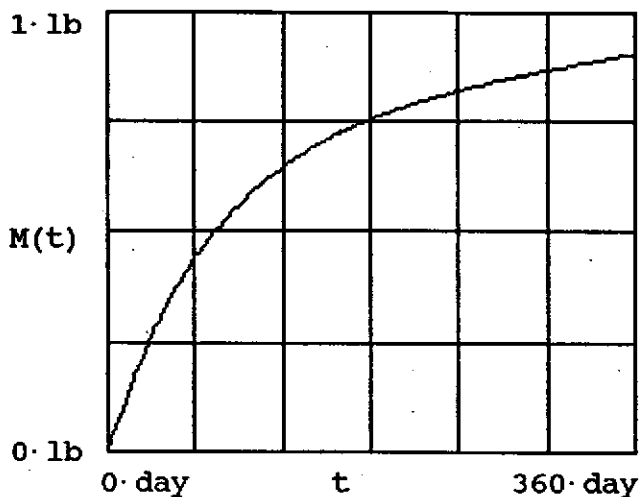
Calculate Mass Removal Over Time:

$$M(t) := (C0 \cdot \text{flow}) \cdot \left[ \left[ 1 - \frac{Cf}{100} \right] \cdot \left[ \frac{1}{kA} \right] \cdot (1 - \exp(-kA \cdot t)) + \left[ \frac{Cf}{100} \right] \cdot t \right]$$

Graph the mass removal, M over time:

t := 0·day, 2·day .. 360·day

t2 := 0·day, 90·day .. 360·day



t2	M(t2)	t2 := 1·day
0	0	M(t2) = 0.0
90	5.612 · 10 <sup>-4</sup>	lb
180	7.561 · 10 <sup>-4</sup>	
270	8.458 · 10 <sup>-4</sup>	
360	9.053 · 10 <sup>-4</sup>	

Units and conversions:

gm ≡ 1M

day ≡ 1T

cm ≡ 1L

m ≡  $\frac{\text{day}}{1440}$

l ≡ 1000 · cm<sup>3</sup>

cf ≡ 28.32 · l

ug ≡ 10<sup>-6</sup> · gm

lb ≡ 453.6 · gm

**ATTACHMENT D**

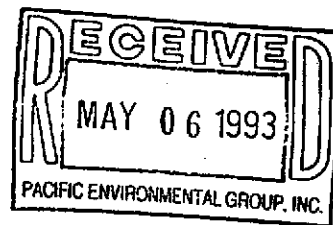
**SOIL VAPOR EXTRACTION TEST  
CERTIFIED ANALYTICAL REPORTS  
AND CHAIN-OF-CUSTODY DOCUMENTATION**





# SEQUOIA ANALYTICAL

680 Chesapeake Drive • Redwood City, CA 94063  
(415) 364-9600 • FAX (415) 364-9233



Pacific Environmental Group  
2025 Gateway Place, Suite 440  
San Jose, CA 95110  
Attention: Keith Winemiller

Project: 330-06.19/Arco 0608, San Lorenzo


Enclosed are the results from 2 air samples received at Sequoia Analytical on April 30, 1993. The requested analyses are listed below:

3DC5401	Air, V4-1	4/29/93	EPA 5030/8015/8020
3DC5402	Air, V4-2	4/29/93	EPA 5030/8015/8020

Please contact me if you have any questions. In the meantime, thank you for the opportunity to work with you on this project.

Very truly yours,

SEQUOIA ANALYTICAL

  
Eileen A. Manning  
Project Manager



# SEQUOIA ANALYTICAL

680 Chesapeake Drive • Redwood City, CA 94063  
(415) 364-9600 • FAX (415) 364-9233

Pacific Environmental Group 2025 Gateway Place, Suite 440 San Jose, CA 95110 Attention: Keith Winemiller	Client Project ID: 330-06.19/Arco 0608, San Lorenzo Sample Matrix: Air Analysis Method: EPA 5030/8015/8020 First Sample #: 3DC5401	Sampled: Apr 29, 1993 Received: Apr 30, 1993 Reported: May 5, 1993
---	---	--

## TOTAL PURGEABLE PETROLEUM HYDROCARBONS with BTEX DISTINCTION

Analyte	Reporting Limit µg/L	Sample I.D. 3DC5401 V4-1	Sample I.D. 3DC5402 V4-2
Purgeable Hydrocarbons	5.0	100	8,500
Benzene	0.050	0.72	100
Toluene	0.050	1.5	47
Ethyl Benzene	0.050	2.0	35
Total Xylenes	0.050	6.2	63

Chromatogram Pattern:	Gas + Non-gas < C8	Non-gas < C8
-----------------------	-----------------------	-----------------

### Quality Control Data

Report Limit Multiplication Factor:	1.0	100
Date Analyzed:	4/30/93	4/30/93
Instrument Identification:	GCHP-3	GCHP-3
Surrogate Recovery, %: (QC Limits = 70-130%)	98	112

Purgeable Hydrocarbons are quantitated against a fresh gasoline standard.  
Analytes reported as N.D. were not detected above the stated reporting limit.

SEQUOIA ANALYTICAL

  
Eileen A. Manning  
Project Manager

3DC5401.PPP <1>



# SEQUOIA ANALYTICAL

680 Chesapeake Drive • Redwood City, CA 94063  
(415) 364-9600 • FAX (415) 364-9233

Pacific Environmental Group  
2025 Gateway Place, Suite 440  
San Jose, CA 95110  
Attention: Keith Winemiller

Client Project ID: 330-06.19/Arco 0608, San Lorenzo

QC Sample Group 3DC5401-02

Reported: May 5, 1993

## QUALITY CONTROL DATA REPORT

ANALYTE	Benzene	Toluene	Ethyl-Benzene	Xylenes
Method:	EPA 8020	EPA 8020	EPA 8020	EPA 8020
Analyst:	M. Nipp	M. Nipp	M. Nipp	M. Nipp
Conc. Spiked:	10	10	10	30
Units:	µg/L	µg/L	µg/L	µg/L
LCS Batch#:	GBLK043093	GBLK043093	GBLK043093	GBLK043093
Date Prepared:	N.A.	N.A.	N.A.	N.A.
Date Analyzed:	4/30/93	4/30/93	4/30/93	4/30/93
Instrument I.D.#:	GCHP-3	GCHP-3	GCHP-3	GCHP-3
LCS % Recovery:	100	100	100	100
Control Limits:	80-120	80-120	80-120	80-120

MS/MSD Batch #:	G9304B4502	G9304B4502	G9304B4502	G9304B4502
Date Prepared:	N.A.	N.A.	N.A.	N.A.
Date Analyzed:	4/30/93	4/30/93	4/30/93	4/30/93
Instrument I.D.#:	GCHP-3	GCHP-3	GCHP-3	GCHP-3
Matrix Spike % Recovery:	110	110	110	107
Matrix Spike Duplicate % Recovery:	110	110	110	107
Relative % Difference:	0.0	0.0	0.0	0.0

Quality Assurance Statement: All standard operating procedures and quality control requirements have been met.

SEQUOIA ANALYTICAL

Please Note:

The LCS is a control sample of known, interferent free matrix that is analyzed using the same reagents, preparation and analytical methods employed for the samples. The LCS % recovery data is used for validation of sample batch results. Due to matrix effects, the QC limits for MS/MSD's are advisory only and are not used to accept or reject batch results.

Eileen A. Manning  
Project Manager

SEQUOIA ANALYTICAL SAMPLE RECEIPT LOG

CLIENT NAME: P.E.G.  
 REC. BY (PRINT): T.C.

MASTER LOG NO. / PAGE: \_\_\_\_\_  
 DATE OF LOG-IN: 4/30/93

CIRCLE THE APPROPRIATE RESPONSE		LAB SAMPLE #	DASH #	CLIENT IDENTIFICATION	CONTAINER DESCRIPTION	SAMPLE MATRIX	DATE SAMP.	REMARKS: CONDITION (ETC)
1. Custody Seal(s):	Present / <del>Absent</del> Intact / Broken*	9304C54-01	A	V4-1	tedlar bag	A	4/29	
		↓ 02	↓	V4-2	↓	↓	↓	
2. Custody Seal Nos.:	_____							
3. Chain-of-Custody Records:	<del>Present</del> / Absent*							
4. Traffic Reports or Packing List:	Present / <del>Absent</del>							
5. Airbill:	Airbill / Slicker Present / <del>Absent</del>							
6. Airbill No.:	_____							
7. Sample Tags:	<del>Present</del> / Absent*							
Sample Tag Nos.:	<del>Listed</del> / Not Listed on Chain-of-Custody							
8. Sample Condition:	<del>Intact</del> / Broken* / Leaking*							
9. Does information on custody reports, traffic reports and sample tags agree?	<del>Yes</del> / No*							
10. Proper Preservatives Used:	<del>Yes</del> / No*							
11. Date Rec. at Lab:	<u>4.30.93</u>							
12. Time Rec. at Lab:	<u>10:40 am</u>							

Circled, contact Project Manager and attach record of resolution

Project no. 0608 City (Facility) San Lorenzo Project manager (Consultant) Keith Winemiller  
 Person m. ke whelan Telephone no. (ARCO) Telephone no. (Consultant) (408) 441-7500 Fax no. (Consultant) (408) 441-7539  
 Name Pacific Environmental Group Address (Consultant) 2025 Gateway Pl. Ste. 440 San Jose CA

Laboratory name Sequoia  
 Contract number

Lab no.	Container no.	Matrix			Preservation		Sampling date	Sampling time	BTEX EPA 8020	EAS BTEX/TPH EPA 1602/8020/8015	TPH Modified 8015 Gas <input type="checkbox"/> Diesel <input type="checkbox"/>	Oil and Grease 413.1 <input type="checkbox"/> 413.2 <input type="checkbox"/>	TPH EPA 418.1/SM503E	EPA 6018010	EPA 6240240	EPA 6250270	TCLP Metals <input type="checkbox"/> VOA <input type="checkbox"/> VOC <input type="checkbox"/>	Semi Metals <input type="checkbox"/> VOA <input type="checkbox"/> VOC <input type="checkbox"/>	CAN Metals EPA 6010/7000 TLC <input type="checkbox"/> STLC <input type="checkbox"/>	Lead Org/DHS <input type="checkbox"/> Lead EPA 7420/7421 <input type="checkbox"/>		
		Soil	Water	Other	Ice	Acid																
	1			X			4/29/93	2:00		X												
	1			X			↓	2:47		↓												

Method of shipment

Special detection Limit/reporting

Special QA/QC

Remarks  
 Tedlars appear to be deflating. Analyse immediately

Lab number

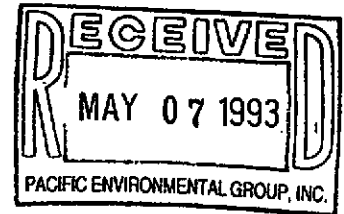
Turnaround time  
 Priority Rush 1 Business Day   
 Rush 2 Business Days   
 Expedited 5 Business Days   
 Standard 10 Business Days AIR

sample: Temperature received:  
 Date 4/30/93 Time 8:00 Received by M. Dooder  
 Date 4/30/93 Time 9:50 Received by M. Dooder  
 Date 4/30/93 Time 10:40 Received by laboratory Date 4-30-93 Time 10:40 am



# SEQUOIA ANALYTICAL

680 Chesapeake Drive • Redwood City, CA 94063  
(415) 364-9600 • FAX (415) 364-9233



Pacific Environmental Group  
2025 Gateway Place, Suite 440  
San Jose, CA 95110  
Attention: Keith Winemiller

Project: 330-06.19/Arco 0608, San Lorenzo

Enclosed are the results from 3 air samples received at Sequoia Analytical on April 30, 1993. The requested analyses are listed below:

3DD5101	Air, V5-0	4/30/93	EPA 5030/8015/8020
3DD5102	Air, V5-1	4/30/93	EPA 5030/8015/8020
3DD5103	Air, V5-2	4/30/93	EPA 5030/8015/8020

Please contact me if you have any questions. In the meantime, thank you for the opportunity to work with you on this project.

Very truly yours,

SEQUOIA ANALYTICAL

  
Eileen A. Manning  
Project Manager



# SEQUOIA ANALYTICAL

680 Chesapeake Drive • Redwood City, CA 94063  
(415) 364-9600 • FAX (415) 364-9233

Pacific Environmental Group 2025 Gateway Place, Suite 440 San Jose, CA 95110 Attention: Keith Winemiller	Client Project ID: 330-06.19/Arco 0608, San Lorenzo Sample Matrix: Air Analysis Method: EPA 5030/8015/8020 First Sample #: 3DD5101	Sampled: Apr 30, 1993 Received: Apr 30, 1993 Reported: May 5, 1993
---	---	--

## TOTAL PURGEABLE PETROLEUM HYDROCARBONS with BTEX DISTINCTION

Analyte	Reporting Limit µg/L	Sample I.D. 3DD5101 V5-0	Sample I.D. 3DD5102 V5-1	Sample I.D. 3DD5103 V5-2
Purgeable Hydrocarbons	5.0	13	N.D.	6.7
Benzene	0.050	N.D.	N.D.	N.D.
Toluene	0.050	0.093	N.D.	0.061
Ethyl Benzene	0.050	0.31	0.10	0.19
Total Xylenes	0.050	1.9	0.63	1.2
Chromatogram Pattern:		Gas	--	Gas

### Quality Control Data

Report Limit Multiplication Factor:	1.0	1.0	1.0
Date Analyzed:	4/30/93	4/30/93	4/30/93
Instrument Identification:	GCHP-3	GCHP-3	GCHP-3
Surrogate Recovery, %: (QC Limits = 70-130%)	104	96	102

Purgeable Hydrocarbons are quantitated against a fresh gasoline standard.  
Analytes reported as N.D. were not detected above the stated reporting limit.

SEQUOIA ANALYTICAL

  
Eileen A. Manning  
Project Manager



# SEQUOIA ANALYTICAL

680 Chesapeake Drive • Redwood City, CA 94063  
(415) 364-9600 • FAX (415) 364-9233

Pacific Environmental Group  
2025 Gateway Place, Suite 440  
San Jose, CA 95110

Client Project ID: 330-06.19/Arco 0608, San Lorenzo

Attention: Keith Winemiller

QC Sample Group 3DD5101-03

Reported: May 5, 1993

## QUALITY CONTROL DATA REPORT

ANALYTE	Benzene	Toluene	Ethyl-Benzene	Xylenes
Method:	EPA 8020	EPA 8020	EPA 8020	EPA 8020
Analyst:	M. Nipp	M. Nipp	M. Nipp	M. Nipp
Conc. Spiked:	10	10	10	30
Units:	µg/L	µg/L	µg/L	µg/L
LCS Batch#:	GBLK043093	GBLK043093	GBLK043093	GBLK043093
Date Prepared:	N.A.	N.A.	N.A.	N.A.
Date Analyzed:	4/30/93	4/30/93	4/30/93	4/30/93
Instrument I.D.#:	GCHP-3	GCHP-3	GCHP-3	GCHP-3
LCS % Recovery:	100	100	100	100
Control Limits:	80-120	80-120	80-120	80-120

MS/MSD Batch #:	G9304B4502	G9304B4502	G9304B4502	G9304B4502
Date Prepared:	N.A.	N.A.	N.A.	N.A.
Date Analyzed:	4/30/93	4/30/93	4/30/93	4/30/93
Instrument I.D.#:	GCHP-3	GCHP-3	GCHP-3	GCHP-3
Matrix Spike % Recovery:	110	110	110	107
Matrix Spike Duplicate % Recovery:	110	110	110	107
Relative % Difference:	0.0	0.0	0.0	0.0

Quality Assurance Statement: All standard operating procedures and quality control requirements have been met.  
SEQUOIA ANALYTICAL

**Please Note:**

The LCS is a control sample of known, interferent free matrix that is analyzed using the same reagents, preparation and analytical methods employed for the samples. The LCS % recovery data is used for validation of sample batch results. Due to matrix effects, the QC limits for MS/MSD's are advisory only and are not used to accept or reject batch results.

  
Eileen A. Manning  
Project Manager



IT NAME:  
BY (PRINT):

PEG  
PH

MASTER LOG NO. / PAGE:  
DATE OF LOG-IN:

9304D51  
4-30-93

THE APPROPRIATE RESPONSE	LAB SAMPLE #	DASH #	CLIENT IDENTIFICATION	CONTAINER DESCRIPTION	SAMPLE MATRIX	DATE SAMP.	REMARKS: CONDITION (ETC)
Study Seal(s): Present / <u>Absent</u> Intact / Broken*		Δ	V4-0	TEOLAR	AIR	4-20	
		↓	V4-1	↓	↓	↓	
		↓	V4-2				
Chain-of-Custody Records: <u>Present</u> / Absent*			Samples labelled				
			V5-0				
			V5-1				
Traffic Reports or Tracking List: Present / <u>Absent</u>			V5-2				
Airbill: Airbill / Silcker Present / <u>Absent</u>							
Sample Tags: <u>Present</u> / Absent* Sample Tag Nos.: <u>Listed</u> / Not Listed on Chain-of-Custody							
Sample Condition: <u>Intact</u> / Broken* / Leaking*							
Does information on study reports, traffic reports and sample tags agree? <u>Yes</u> / No*							
Proper Preservatives Used: <u>Yes</u> / No*							
Date Rec. at Lab: 4-30-93							
Time Rec. at Lab: 1615							

circled, contact Project Manager and attach record of resolution

Project no. 06008	Facility (7601 HESPERIAN BLVD)			Project manager (Consultant) KEITH WINEMILUX		Laboratory name SEQUOIA			
engineer MIKE WYKLAN			Telephone no. (ARCO)		Telephone no. (Consultant) 408-441-7500		Fax no. (Consultant) 408-441-7539		
Contract name PACIFIC ENV. GROUP			Address (Consultant) 2025 GATEWAY, STE 440 SAN JOSE 95110					Contract number 07-073	

Lab no.	Container no.	Matrix			Preservation		Sampling date	Sampling time	BTEX EPA 802/EPA 8020	BTEX/TPH EPA 8020/8020/8015	TPH Modified 8015 Gas Diesel	Oil and Grease 413.1 413.2	TPH EPA 418.1/MS503E	EPA 601/8010	EPA 624/8240	EPA 625/8270	TCLP Metals VOA VOC	Send Metals VOA VOC	CAN Metals EPA 501/7000 ITLC STLC	Lead Org./DHS Lead EPA 7420/7421	Method of shipment	Special detection Limit/reporting	
		Soil	Water	Other AIR	Ice	Acid																	
0	1			+			4/30		X														
1	1			+			4/30		X														
2	1			+			4/30		X														

Date of sample: 4/30/93				Temperature received: cool			
Signed by sampler: [Signature]		Date: 4/30/93		Time: 15:25		Received by: [Signature]	
Signed by: [Signature]		Date: 4/30/93		Time: 4:15		Received by: [Signature]	
Signed by: [Signature]				Date: 4-20-90		Time: 1615	

Priority Rush 1 Business Day

Rush 2 Business Days

Expedited 5 Business Days

Standard 10 Business Days AIR

White copy — Laboratory; Canary copy — ARCO Environmental Engineering; Pink copy — Consultant

**ATTACHMENT E**

**IN-SITU BIOREMEDIATION FEASIBILITY TESTING  
CERTIFIED ANALYTICAL REPORTS AND  
CHAIN-OF-CUSTODY DOCUMENTATION**



# BioScreen Testing Services, Inc.

Microbiology • Chemistry • Environmental • Asbestos

## ANALYTICAL REPORT

PACIFIC ENVIRONMENTAL GROUP  
2025 Gateway Place  
#440  
San Jose, CA 95110

REPORT DATE: 04/05/93  
ACCESSION #: 7338 - 7341  
PROJECT #: 0693-4

ATTN: Kelly Brown

SAMPLE:

<u>ACC #</u>	<u>SAMPLES:</u>	
7338	ARCO Products Soil B- 9	10-12'
7339	ARCO Products Soil B-10	11-13'
7340	ARCO Products Soil B-11	11-13'
7341	ARCO Products Soil B-12	11-13'

TEST(S)  
PERFORMED:

METHOD REFERENCE

Heterotrophic Plate Count	BTS 227
Fluorescent <i>Pseudomonas</i>	BTS 228
Hydrocarbon Degraders	BTS 229
Ammonia	EPA 350.3
Nitrate	EPA 300.0
Phosphate	EPA 300.0
Potassium	EPA 601.0
pH	BTS 544
Moisture	BTS 554
Calcium	EPA 601.0
Magnesium	EPA 601.0
Iron	EPA 601.0

RESULTS:

ACCESSION #: 7338

ARCO PRODUCTS SOIL

B- 9

10-12'

RESULTS:

DETECTION LIMIT

Heterotrophic Plate Count	1.0 x 10 <sup>3</sup> CFU/gm	1000 CFU/gm
Fluorescent <i>Pseudomonas</i>	<1000 CFU/gm	1000 CFU/gm
Hydrocarbon Degradars	<1000 CFU/gm	1000 CFU/gm
Ammonia	ND	10 ppm
Nitrate	ND	2.1 ppm
Phosphate	ND	5 ppm
Potassium	531	60 ppm
pH	8.28	---
Moisture	19.97 %	---
Calcium	3250 ppm	10 ppm
Magnesium	4,850 ppm	300 ppm
Iron	14,700 ppm	400 ppm

ACCESSION #: 7339

ARCO PRODUCTS SOIL

B-10

11-13'

RESULTS:

DETECTION LIMIT

Heterotrophic Plate Count	1.1 x 10 <sup>4</sup> CFU/gm	1000 CFU/gm
Fluorescent <i>Pseudomonas</i>	<1000 CFU/gm	1000 CFU/gm
Hydrocarbon Degradars	<1000 CFU/gm	1000 CFU/gm
Ammonia	ND	10 ppm
Nitrate	ND	2.1 ppm
Phosphate	ND	5 ppm
Potassium	684	60 ppm
pH	8.19	---
Moisture	21.18 %	---
Calcium	4,340 ppm	10 ppm
Magnesium	5,670 ppm	300 ppm
Iron	15,100 ppm	400 ppm

RESULTS: (cont.)

ACCESSION #: 7340

ARCO PRODUCTS SOIL

B-11

11-13'

RESULTS:

DETECTION LIMIT

Heterotrophic Plate Count	6.2 x 10 <sup>4</sup> CFU/gm	1000 CFU/gm
Fluorescent <i>Pseudomonas</i>	<1000 CFU/gm	1000 CFU/gm
Hydrocarbon Degradars	4.0 x 10 <sup>3</sup> CFU/gm	1000 CFU/gm
Ammonia	ND	10 ppm
Nitrate	2.4	2.1 ppm
Phosphate	ND	5 ppm
Potassium	620	60 ppm
pH	7.24	---
Moisture	23.25 %	---
Calcium	3920 ppm	10 ppm
Magnesium	2,820 ppm	300 ppm
Iron	16,100 ppm	400 ppm

ACCESSION #: 7341

ARCO PRODUCTS SOIL

B-10

11-13'

RESULTS:

DETECTION LIMIT

Heterotrophic Plate Count	3.0 x 10 <sup>3</sup> CFU/gm	1000 CFU/gm
Fluorescent <i>Pseudomonas</i>	<1000 CFU/gm	1000 CFU/gm
Hydrocarbon Degradars	<1000 CFU/gm	1000 CFU/gm
Ammonia	ND	10 ppm
Nitrate	ND	2.1 ppm
Phosphate	ND	5 ppm
Potassium	333	60 ppm
pH	7.91	---
Moisture	11.76 %	---
Calcium	3,100 ppm	10 ppm
Magnesium	5,510 ppm	300 ppm
Iron	9,460 ppm	400 ppm

**Discussion:**


The microbiological results show levels of heterotrophic organisms in the soil are below normal values. Based on results of samples tested at BioScreen Testing normal levels of total organism counts should be in the  $1 \times 10^5$  to  $1 \times 10^6$  range. The fluorescent *Pseudomonas* and hydrocarbon degraders levels are below normal values. Based on results of samples tested at BioScreen testing normal levels of fluorescent *Pseudomonas* and hydrocarbon degraders should be in the  $1 \times 10^5$  and  $1 \times 10^3$  range respectively. The low levels of microorganisms could be due to the depth at which samples were taken.

The nutrient data shows that ammonia, nitrate and phosphate are non detected except for sample number 7340. Potassium levels are not of concern at this time.


In addition the levels of calcium and magnesium are high. This is a cause for some concern and the nutrient amendment will have to be carefully buffered so that inorganic precipitation does not take place.

The moisture and pH seem to be within normal levels for supporting microbial growth.

The indication from the initial assessment data is that no bioremediation is taking place. With proper nutrient amendment of the soil it may be possible to stimulate bioremediation. The final ability to bioremediate a site is dependent on the type of contaminant present and the outcome of a treatability study.



Bradford L. Rope  
Laboratory Director



Ranil M. Fernando, B.S.  
Operations Supervisor

**ARCO Products Company**  
Division of AtlanticRichfieldCompany

330-06.20 Task Order No. 0608-93-2

**Chain of Custody**

ARCO Facility no. <b>0608</b>	City (Facility) <b>San Lorenzo</b>	Project manager (Consultant) <b>Kelly Brown</b>	Laboratory name <b>BioScreen Testing Services</b>
ARCO engineer <b>Michael Whelan</b>	Telephone no. (ARCO)	Telephone no. (Consultant) <b>(408) 441-7500</b>	Contract number
Consultant name <b>Pacific Environmental Group</b>	Address (Consultant) <b>2025 Gateway Pl. #440, San Jose CA 95110</b>		Contract number <b>000151 (Quotation #)</b>

Sample I.D.	Lab no.	Container no.	Matrix			Preservation		Sampling date	Sampling time	BTEX 602/EPA 8020	BTEX/TPH EPA 1602/9020/8015	TPH Modified 8015 Gas <input type="checkbox"/> Diesel <input type="checkbox"/>	Oil and Grease 413.1 <input type="checkbox"/> 413.2 <input type="checkbox"/>	Total Nitrogen EPA 8210/18010	Cadmium EPA 8210/18010	Moisture EPA 8210/18010	Sulfide EPA 8210/18010	Cyanide EPA 8210/18010	Lead EPA 8210/18010	Copper EPA 8210/18010	Zinc EPA 8210/18010	pH	Microbiology Testing	Microtoxicity	
			Soil	Water	Other	Ice	Acid																		
B-9 @ 10-12'	1		X			X		3/9/93	12:30 PM					X	X	X			X	X	X	X	X	X	X
B-10 @ 11-13'	1		X			X		↓	1:30 PM					↓	↓	↓			↓	↓	↓	↓	↓	↓	
B-11 @ 14-13'	1		X			X		↓	2:30 PM					↓	↓	↓			↓	↓	↓	↓	↓	↓	
B-12 @ 14-13'	1		X			X		↓	3:30 PM					↓	↓	↓			↓	↓	↓	↓	↓	↓	

Method of shipment  
**Federal Express**

Special detection Limit/reporting

Special QA/QC

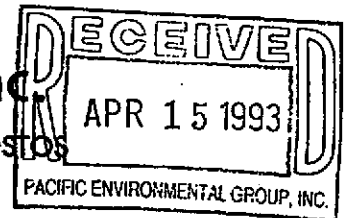
Remarks  
**Note: Very Short Hold times!**

Lab number

Turnaround time  
 Priority Rush 1 Business Day   
 Rush 2 Business Days   
 Expedited 5 Business Days   
 Standard 10 Business Days

Condition of sample:		Temperature received:	
Relinquished by sampler <b>Roger Hoffmann</b>	Date <b>3/9/93</b> Time <b>1710</b>	Received by <b>C. Huntsman</b>	
Relinquished by	Date	Time	Received by
Relinquished by	Date	Time	Received by laboratory
			Date
			Time





**ANALYTICAL REPORT**

PACIFIC ENVIRONMENTAL GROUP  
2025 Gateway Place  
#440  
San Jose, CA 95110

REPORT DATE: 04/05/93  
ACCESSION #: 7344 - 7345  
PROJECT #: 0713-1

ATTN: Kelly Brown

**SAMPLE:**

<u>ACC #</u>	<u>SAMPLES:</u>	
7344	ARCO Products Soil B-25	12-14'
7345	ARCO Products Soil B-26	12-14'

**TEST(S)  
PERFORMED:**

**METHOD REFERENCE**

Heterotrophic Plate Count	BTS 227
Fluorescent <i>Pseudomonas</i>	BTS 228
Hydrocarbon Degraders	BTS 229
Ammonia	EPA 350.3
Nitrate	EPA 300.0
Phosphate	EPA 300.0
Potassium	EPA 601.0
pH	BTS 544
Moisture	BTS 554
Calcium	EPA 601.0
Magnesium	EPA 601.0
Iron	EPA 601.0
Microtoxicity	

**RESULTS:**

**ACCESSION #: 7344**

**ARCO PRODUCTS SOIL**

**B- 25**

**12-14'**

**RESULTS:**

**DETECTION LIMIT**

Heterotrophic Plate Count	3.0 x 10 <sup>3</sup> CFU/gm	1000 CFU/gm
Fluorescent <i>Pseudomonas</i>	<1000 CFU/gm	1000 CFU/gm
Hydrocarbon Degradars	<1000 CFU/gm	1000 CFU/gm
Ammonia	ND	10 ppm
Nitrate	ND	2.1 ppm
Phosphate	ND	5 ppm
Potassium	513	60 ppm
pH	7.66	---
Moisture	19.35 %	---
Calcium	4,240 ppm	10 ppm
Magnesium	5,280 ppm	300 ppm
Iron	18,000 ppm	400 ppm
Microtoxicity	Non Toxic to Microorganism	

**ACCESSION #: 7345**

**ARCO PRODUCTS SOIL**

**B-26**

**12-14'**

**RESULTS:**

**DETECTION LIMIT**

Heterotrophic Plate Count	<1000 CFU/gm	1000 CFU/gm
Fluorescent <i>Pseudomonas</i>	<1000 CFU/gm	1000 CFU/gm
Hydrocarbon Degradars	<1000 CFU/gm	1000 CFU/gm
Ammonia	ND	10 ppm
Nitrate	ND	2.1 ppm
Phosphate	ND	5 ppm
Potassium	756	60 ppm
pH	7.54	---
Moisture	23.82 %	---
Calcium	4,120 ppm	10 ppm
Magnesium	6,150 ppm	300 ppm
Iron	19,200 ppm	400 ppm
Microtoxicity	Non Toxic to Microorganism	

**Discussion:**

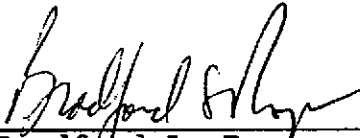
The microbiological results show levels of heterotrophic organisms in the soil are below normal values. Based on results of samples tested at BioScreen Testing normal levels of total organism counts should be in the  $1 \times 10^5$  to  $1 \times 10^6$  range. The fluorescent *Pseudomonas* and hydrocarbon degraders levels are below normal values. Based on results of samples tested at BioScreen testing normal levels of fluorescent *Pseudomonas* and hydrocarbon degraders should be in the  $1 \times 10^5$  and  $1 \times 10^3$  range respectively. The low levels of microorganisms could be due to the depth at which samples were taken.

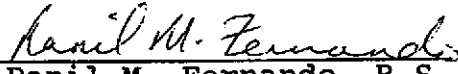
The nutrient data shows that ammonia, nitrate and phosphate are non detected in all samples. Potassium levels are not of concern at this time.

In addition the levels of calcium and magnesium are high. This is a cause for some concern and the nutrient amendment will have to be carefully buffered so that inorganic precipitation does not take place.

The moisture and pH seem to be within normal levels for supporting microbial growth.

The indication from the initial assessment data is that no bioremediation is taking place. With proper nutrient amendment of the soil it may be possible to stimulate bioremediation. The final ability to bioremediate a site is dependent on the type of contaminant present and the outcome of a treatability study.

  
\_\_\_\_\_  
Bradford L. Rope  
Laboratory Director

  
\_\_\_\_\_  
Ranil M. Fernando, B.S.  
Operations Supervisor

**ARCO Products Company**  
Division of AtlanticRichfieldCompany

330-06,20

Task Order No. 0608-93-2

**Chain of Custody**

ARCO Facility no. 0608 City (Facility) San Lorenzo Project manager (Consultant) Kelly Brown Laboratory name Big Screen Testing  
 ARCO engineer Michael Whelan Telephone no. (ARCO) \_\_\_\_\_ Telephone no. (Consultant) (408) 441-7500 Fax no. (Consultant) (408) 441-7539 Contract number (00151)  
 Consultant name Pacific Environmental Group Address (Consultant) 2025 Gateway Place #440 San Jose, CA 95110

Sample I.D.	Lab no.	Container no.	Matrix			Preservation		Sampling date	Sampling time	BTEX 602/EPA 8020	BTEX/TPH EPA 1631/8015	TPH Modified 8015 Gas <input type="checkbox"/> Diesel <input type="checkbox"/>	Oil and Grease 413.1 <input type="checkbox"/> 413.2 <input type="checkbox"/>	The Nutrient EPA 816/817/818/819	Sulfide EPA 816/817/818/819	Mercury EPA 816/817/818/819	pH	TCLP Metals <input type="checkbox"/> VOA <input type="checkbox"/>	SEM Metals <input type="checkbox"/> VOA <input type="checkbox"/>	CAM Metals EPA 8010/7000 ITLC <input type="checkbox"/> STLC <input type="checkbox"/>	Lead Org./DHS Lead EPA 7420/421 <input type="checkbox"/>	Microb Testing	Water Oxygen
			Soil	Water	Other	Ice	Acid																
<u>B-75 @ 12-14'</u>	<u>2</u>	<u>X</u>	<u>X</u>			<u>X</u>		<u>3/11/93</u>	<u>10:30AM</u>					<u>X</u>	<u>+</u>	<u>X</u>	<u>X</u>					<u>X</u>	<u>X</u>
<u>B-76 @ 12-14'</u>	<u>2</u>	<u>X</u>	<u>X</u>			<u>X</u>		<u>3/11/93</u>	<u>11:30AM</u>					<u>+</u>	<u>+</u>	<u>+</u>	<u>+</u>					<u>+</u>	<u>X</u>

Method of shipment  
Federal Express

Special detection Limit/reporting

Special QA/QC

Remarks  
Note:  
very short  
Hold time

Lab number

Turnaround time  
 Priority Rush  
 1 Business Day  
 Rush  
 2 Business Days  
 Expedited  
 5 Business Days  
 Standard  
 10 Business Days

Condition of sample: \_\_\_\_\_ Temperature received: \_\_\_\_\_  
 Relinquished by sampler Roger Hoffmann Date 3/11/93 Time \_\_\_\_\_ Received by Madrigal #745  
 Relinquished by Madrigal #745 Date \_\_\_\_\_ Time \_\_\_\_\_ Received by Tokuyuki/Riz  
 Relinquished by Y. G. H. (P) Date 3/11/93 Time 2:05 Received by laboratory \_\_\_\_\_ Date \_\_\_\_\_ Time \_\_\_\_\_