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Environmental Health

November 20, 2009

Mr. Jerry Wickham, PG Alameda County Department of Environmental Health Services 1131 Harbor Bay Parkway, Suite 250 Alameda, California 94502-6577

Subject: Fuel Leak Case#R00002996 Site Address: 316 38th Street, Oakland, CA

Dear Mr. Wickham:

SOMA's "Revised Workplan for Additional Soil and Groundwater Investigation" for the subject property has been uploaded to the State's GeoTracker database and Alameda County's FTP site for your review.

Thank you for your time in reviewing our report. If you have any questions or comments, please call me at (925) 734-6400.

Sincerely,

Mansour Sepehr, Ph.D., PE Principal Hydrogeologist

Enclosure

cc: Mr. John Kortum, Esq.



Revised Workplan for Additional Soil and Groundwater Investigation

316 38th Street Oakland, California Case RO0002996

November 20, 2009

Project 2720

Prepared for Mr. Earl Thompson, Jr. Executor for the Estate of Earl Thompson, Sr.

CERTIFICATION

SOMA Environmental Engineering, Inc. has prepared this report on behalf of Mr. Earl Thompson, Jr., Executor for the Estate of Earl Thompson, Sr., property owner of 316 38th Street, Oakland, California, to comply with Alameda County Environmental Health Services requirements for further soil and groundwater investigation, as specified in April 30, 2009 and September 17, 2009 correspondence.

Mansour Sepehr, PhD, PE Principal Hydrogeologist



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1. INTRODUCTION

SOMA Environmental Engineering, Inc. (SOMA) has prepared this report on behalf of Mr. Earl Thompson, Jr., Executor for the Estate of Earl Thompson Sr., property owner of 316 38th Street in Oakland, California (Thompson Property). The site is located in an area of primarily commercial and residential property uses (Figure 1). The site map showing locations of underground storage tanks (USTs), boreholes and wells is shown in Figure 2. This report summarizes decommissioning results of three underground storage tanks located under the sidewalk adjacent to the Thompson Property and proposes further investigation to delineate the extent of apparent contamination observed during confirmation sampling, to comply with requirements of Alameda County Environmental Health Services (ACEHS) for further soil and groundwater investigation specified in April 30, 2009 and September 17, 2009 correspondence.

1.1 Site Vicinity

Properties in the vicinity of the Thompson Property are primarily commercial and residential. Reportedly, six USTs were previously located at or near the nearby Glovatorium site located upgradient from the subject site at 3820 Manila Avenue (Glovatorium Property). The location of the Glovatorium Property is shown on Figures 1 and 2A. Two USTs associated with the Glovatorium Property were located under the sidewalk near 316 38th Street and four USTs were located inside the Glovatorium building. Capacities of the six Glovatorium USTs have been reported as ranging from 800 gallons to 5,000 gallons. They reportedly contained Stoddard solvent (TPH-ss), fuel oil, and possibly waste oil. In June 1997, HK2 obtained City of Oakland Fire Prevention Bureau permit No. 52-97 to decommission the USTs. USTs inside the building were interconnected through a series of pipes and valves. It was reported that in about the late 1970s a significant release of TPH-ss occurred when a new piping system was installed. In August 1997, the six Glovatorium USTs were abandoned in-place by backfilling with either cement-sand slurry or pea gravel. Groundwater monitoring wells associated with the Glovatorium Property are currently monitored semiannually. Past groundwater monitoring events have indicated the presence of volatile organic compounds (VOCs) and PHCs in groundwater beneath the Glovatorium and adjacent properties.

Following are results of the latest groundwater monitoring event at the Glovatorium Property. Detectable levels ranged as follows: TPH-ss from 57 $\mu g/L$ in LFR-1 and SOMA-1 to 860,000 $\mu g/L$ in SOMA-2; total petroleum hydrocarbon as gasoline (TPH-g) from 67 $\mu g/L$ in LFR-1 to 1,300,000 $\mu g/L$ in SOMA-2; perchloroethylene (PCE) from 1.5 $\mu g/L$ in LFR-3 to 1,200 $\mu g/L$ in B-10; trichloroethylene (TCE) from 6.2 $\mu g/L$ in GW-2 to 1,200 $\mu g/L$ in B-10; and cis-1,2-dichloroethylene (cis-1,2-DCE) from 0.7 $\mu g/L$ in LFR-4 to 5,900 $\mu g/L$ in SOMA-2. Vinyl chloride (VC) was below the laboratory-reporting limit throughout

the site, except for samples from LFR-2 at 32 μ g/L. Figure 2 shows locations of aforementioned sampling points.

The source of contamination is believed to be either the former Glovatorium USTs, which were used to store TPH-ss and VOCs, or releases from the Glovatorium piping on the washer system and from washing floors within the Glovatorium building with TPH-ss. At this time, multi-phase extraction (MPE) pilot testing events are being conducted at the Glovatorium Property to remediate subsurface contamination and additional soil and groundwater investigation is taking place to delineate vertical and horizontal extent of contamination at the site.

Surrounding properties are primarily commercial and residential. TOSCO Marketing Company is located north and upgradient of the site, at 40th Street and Broadway, and contains a number of groundwater monitoring wells.

1.2 Site Hydrogeology

The site is located on the alluvial plain between the San Francisco Bay shoreline and the Oakland hills. Surface sediments in the site vicinity consist of Holocene alluvial deposits that are representative of an alluvial fan depositional environment. These deposits consist of brown, medium dense sand that fines upward to sandy or silty clay. The pattern of stream channel deposition results in a three-dimensional network of coarse-grained sediments interspersed with finer grained silts and clays. The individual units tend to be discontinuous lenses aligned parallel to the axis of the former stream flow direction.

Sediments encountered in soil borings in the vicinity of the site are typical of those encountered in an alluvial fan depositional environment. The sediments are predominantly fine-grained, consisting of clay, silty clay, sandy clay, gravelly clay and clayey silt. Discontinuous layers of coarse-grained sediments (clayey sand, silty sand, and clayey gravel) generally also contain relatively high percentages of silt and clay, which tend to reduce their permeability. A relatively coarse-grained layer of silty sand, clayey sand, and clayey gravel was encountered at approximately 4.5 to 14 feet below ground surface (bgs). A discontinuous layer of silty to clayey sand was encountered at depths of 17 to 21 bgs.

According to results of historical groundwater monitoring activities in the site vicinity, groundwater occurs at 13 to 20 feet bgs. Based on current and previous groundwater monitoring reports for wells in the vicinity of the site, groundwater flows from the northeast to the southwest with an approximate groundwater flow gradient of 0.019 ft/ft to 0.035 ft/ft. Slug test results indicated that hydraulic conductivity of saturated sediments ranges between 1.2 x 10⁻⁴ and 6.9 x 10⁻⁴ cm/sec, which is equivalent to 0.34 ft/day to 1.95 ft/day. Using the average groundwater flow gradient of 0.027 and aquifer porosity of 0.32, the groundwater flow velocity ranges between 10.5 and 60.1 ft/year.

Based on confirmation soil borings advanced during USTs closure, the site is underlaid with unconfined sediments as follows around three decommissioned USTs located on the sidewalk in front of the site (see decommissioning summary below): primarily sand up to approximately 8 to 12 feet bgs (possibly fill material) and inorganic clays with sand to approximately 17 feet bgs around Tank 1; inorganic clays with sand to the total depth of the borings around Tank 2; and interbedded sand, clay, silt layers with gravel up to the total depth of the borings around Tank 3. Depth to water around Tank 1 was noted at approximately 12 feet bgs, Tank 2 at approximately 7 feet bgs, and Tank 3 at 7 feet bgs.

1.3 UST Decommissioning Summary

Three USTs located under the sidewalk in front of the Thompson Property were properly closed in November 2008, thereby effectively removing the contaminant source; results were documented in SOMA's report dated January 27, 2009. All residual amounts of the hazardous substances which were stored in the UST system prior to closure have been removed, properly disposed of, and neutralized; USTs and associated piping were filled according to Oakland Fire Department (OFD) with appropriate slurry mixture.

Based on observed recharge of 0.04 gallons per minute (gpm) into Tank 1 upon purging, it was determined that a small leak possibly existed in this UST at the time of closure. No purging or leak testing was conducted at Tanks 2 and 3 due to their apparent placement above the presumed water table. To verify integrity of the decommissioned USTs, SOMA performed confirmation soil and groundwater sampling activities in accordance with OFD approval of the workplan. To verify integrity of the decommissioned USTs, on November 20-21, 2008, SOMA's field geologist oversaw advancement of confirmation soil borings around each decommissioned UST by Fisch Drilling. Locations of advanced borings are shown in Figure 2. SOMA advanced seven vertical borings to 10-27 feet bgs, depending on the depth of each UST (Tank 1 bottom at 25 feet bgs, Tank 2 bottom at 8 feet bgs, Tank 3 bottom at 15 feet bgs) utilizing direct push technology (DPT). Soil samples were collected at depths where photoionization detector (PID) readings or visual observations indicated significant soil contamination. In addition, one soil sample was collected from the vadose zone at the soil-groundwater interface. Groundwater samples were collected from each advanced boring.

TPH-g was detected in soil samples at concentrations up to 4,100 mg/kg, and TPH-g and benzene were detected in grab groundwater samples at concentrations up to 29,000 and 22 μ g/L, respectively. Following are laboratory analytical results derived during the confirmation soil and groundwater sampling.

1.3.1 Extent of Soil Contamination

Elevated PID levels and hydrocarbon staining were observed in confirmation borings TB1-1, TB1-3, TB2-1, TB3-1, and TB3-2 (Figure 2). As Table 1 indicates, TPH-g was detected above California Regional Water Quality Control Board (CRWQCB) Environmental Screening Level (ESL) in TB1-3 at 14 feet bgs (1,200 mg/kg), TB2-1 at 6 and 10 feet bgs (750 and 120 mg/kg), TB2-2 at 10 feet bgs (120 mg/kg), TB3-1 at 14 feet bgs (3,800 mg/kg) and TB3-2 at 14 and 17 feet bgs (3,200 and 210 mg/kg). TPH-d was detected above ESL at TB1-1 at 18 feet bgs (110 mg/kg). TPH-ss and kerosene were observed above ESL in TB1-1 at 18 feet bgs (170 and 150 mg/kg), TB1-3 at 14 feet bgs (120 and 110 mg/kg), TB2-2 at 10 feet bgs (150 and 130 mg/kg) and in TB3-1 at 14 feet bgs (130 and 120 mg/kg).

As Table 2 indicates, all BTEX (collective term for benzene, toluene, ethylbenzene and total xylenes) and other VOCs were below laboratory-detection limits and well below ESLs at all depths. Figures 3 through 8 show chemicals of concern (COCs) in soil at different depths. Residual soil contamination appears to be present between 6 and 8 feet bgs between Tanks 1 and 2; at approximately 12 feet bgs northeast of Tank 3, between Tanks 3 and 2; and at approximately 14 feet bgs north and northeast of Tank 3.

1.3.2 Extent of Groundwater Contamination

Groundwater samples from each confirmation soil boring showed contaminants above CRWQCB ESLs for groundwater that is a current or potential source of drinking water, as well as a non-drinking-water source. ESLs for TPH-g, TPH as diesel (TPH-d), TPH-ss, and kerosene are 100 µg/L for drinking and 210 µg/L for non-drinking water. As Table 3 shows, TPH-g ranged from 890 µg/L to 29,000 μ g/L, TPH-d from 230 μ g/L to 330,000 μ g/L, TPH-ss from 140 μ g/L to 560,000 µg/L, and kerosene from 170 µg/L to 560,000 µg/L. All samples except those from TB3-2 showed contaminants above non-drinking-water ESLs. Benzene was detected above drinking-water ESL in TB3-1 (22 µg/L, ESL 1 µg/L) and total xylenes were detected above drinking and non-drinking-water ESLs in TB1-3 (1,700 µg/L, drinking-water ESL 20 µg/L). BTEX was below ESLs or laboratory-detection limits in remaining samples. Table 4 shows that VOCs were detected above drinking-water ESLs in TB1-3 (TBA at 28 µg/L, ESL 12 µg/L), TB1-4 (1,2-DCA at 3.6 μg/L, ESL 0.5 μg/L), TB2-1 and TB3-1 (naphthalene at 98 and 19 µg/L, respectively, ESL 17 µg/L). All other VOCs were below ESLs or below laboratory-detection limits. Figures 9 through 12 show PHC contamination in groundwater. Groundwater samples indicate that PCH contamination in groundwater is located in the vicinity of the decommissioned USTs.

1.4 Data Gap Summary

Due to elevated contaminant concentrations detected in shallow soil and groundwater in close proximity to the decommissioned USTs (under the sidewalk

in front of Thompson Property) and, in some instances, undefined extent of elevated concentrations, further investigation to delineate vertical and horizontal extent of contamination is warranted.

1.5 Preferential Flow Pathway and Sensitive Receptor Evaluation

In order to determine whether any preferential flow paths impacting the contaminant flow migration exist at the site, SOMA conducted an evaluation of utility lines in the general vicinity of the contaminant plume located under the sidewalk in front of the Thompson Property. A 54-inch storm drain conduit, which runs through the Glovatorium Property, maybe acting as a preferential flow path for the contaminants associated with that property, is located immediately west of the decommissioned USTs. The 54-inch storm drain is a culvertized former creek channel, part of the Rockridge branch of Glen Echo Creek. Surface topography and groundwater flow direction suggest that natural groundwater flow pathways are likely associated with the creek's drainage system, and may direct groundwater flow toward the storm drain.

In May 1999, Levine-Fricke (LFR) conducted a utility survey, including a video survey of the 54-inch (54" x 69") storm drain between manhole 1 (MH-1) on Manila Avenue to manhole 2 (MH-2) in the center of 38th Street (Figure 2). Through a videotape survey of the storm drain, LFR observed joints in the concrete that extended up the walls and around the roof arch. Water was observed seeping into the storm drain from three joints, one located under the parking lot, west of the site. The storm drain was not followed beyond MH-2 in the center of 38th Street. The depth of the storm drain as it runs below 38th Street is between 8.5 and 14 feet bgs. An 18-inch lateral is located approximately 14-14.5 feet bgs, running from east to west along the 38th Street, a 10-inch lateral approximately 10 feet bgs and a 15-inch lateral approximately 7.5 feet bgs that run toward the site and the 54-inch storm drain. A shallow utility trench runs above the decommissioned tanks (Glovatorium USTs and USTs recently decommissioned by SOMA) toward the storm drain (Figure 2).

Depth to groundwater in confirmation soil borings and adjacent wells has been observed between 5 and 14.5 feet bgs; therefore, the storm drain and its laterals along the 38th Street may act as preferential flow pathways for contamination in the subsurface. As part of the additional investigation, SOMA proposes advancement of several soil borings along the length of the storm drain, and a survey of the lateral inlets within manhole MH-2, to determine exact depths of the lateral connections. Sampling locations and techniques are detailed in Section 3.

To evaluate the land use type in close proximity to the site, a copy of the zoning map for the area was obtained from the City of Oakland (Figure 2A). As shown, the site is zoned regional commercial with residential areas zoned mixed housing located downgradient from the site. Historical review undertaken to identify any wells in the vicinity of the site indicated that there are no domestic, industrial,

irrigation or any other water supply wells within a 2,000-foot radius; the only wells located adjacent to the site are groundwater monitoring wells. Several hospitals are within a 2,000-foot radius of the site; the closest, Kaiser Foundation Hospital, is located immediately south and downgradient from the site. Several schools are within a 2,000-foot radius of the site; the closest is Snow White Preschool, located southeast (crossgradient) approximately 1,100 feet from the site at 214 W. MacArthur Blvd.

1.6 Evaluation of Existing Groundwater Monitoring Well Network

In order to avoid duplication of effort and determine whether additional well installation is necessary, SOMA evaluated the existing groundwater monitoring well network associated with the adjacent Glovatorium Property. Three existing groundwater monitoring wells are associated with the Glovatorium (GW-5, GW-6A, and LFR-4) which were installed in close proximity to the USTs decommissioned by Glovatorium (Figure 2). Historical wells and borings GW-5A, GW-6, GW-7 and GW-8 were decommissioned between July 1999 and July 2000.

Well GW-5 is screened from 8 to 13 feet bgs, GW-6A from 5 to 15 feet bgs, and LFR-4 from 9 to 19 feet bgs. The wells are located 10 to 40 downgradient and crossgradient from the decommissioned USTs. GW-5 and GW-6A, located adjacent to the 54-inch storm drain conduit, have not been monitored since 2000, at that time all contaminants were below laboratory-detection limits. Contamination observed in LFR-4 (the closest well located downgradient of decommissioned tanks) is a few orders of magnitude lower than that observed during the UST decommissioning (most recent soil and groundwater investigation). During the Second Semi-Annual Monitoring Event conducted at the Glovatorium site (September 2009 Groundwater Monitoring Report), TPH-g was detected in LFR-4 at 480 μ g/L and TPH-ss at 270 μ g/L; highest TPH-g and TPH-ss concentrations, observed during the UST decommissioning, were detected at 29,000 μ g/L, and 560,000 μ g/L, respectively.

Groundwater has been encountered in confirmation soil borings and adjacent wells from 5 feet bgs to 14.5 feet bgs. Since the closest downgradient well LFR-4 is screened from 9 to 19 feet bgs, this well may not be ideal for contaminant plume monitoring around the decommissioned USTs. Though not ideal, wells GW-5 and GW-6A may be appropriate for preferential flow path monitoring, adjacent to the 54-inch storm drain. Historical boring logs are attached in Appendix A.

Therefore, as part of the proposed soil and groundwater investigation, SOMA proposes collecting groundwater samples from wells LFR-4, GW-5, and GW-6A and advancing a soil boring in close proximity to LFR-4 to determine whether this well is suitable for monitoring contaminant plume migration. Results will be

evaluated in conjunction with data derived from the proposed investigation prior to making a final determination regarding additional well installation.

2. SCOPE OF WORK

Based on results of the most recent site investigation, conducted in 2008, and ACEHS approval, SOMA proposes installation of DP borings to delineate horizontal extent of contaminant plume downgradient (south) of decommissioned USTs.

Task 1: Permit acquisition, Health and Safety Plan preparation, and subsurface utility clearance

Task 2: Advancement of DP borings

Task 3: Report preparation

Following are brief descriptions of these tasks.

3. FIELD ACTIVITIES

3.1 Pre-Investigation Activities

Upon approval of this workplan, and prior to initiating field activities, drilling permits will be obtained from ACEHS.

SOMA will prepare a site-specific Health and Safety Plan (HASP). The HASP will be prepared according to the Occupational Safety and Health Administration (OSHA), "Hazardous Waste Operation and Emergency Response" guidelines (29 CFR 1910.120) and the California Occupational Safety and Health Administration (Cal/OSHA) "Hazardous Waste Operation and Emergency Response" guidelines (CCR Title 8, section 5192). The HASP is designed to address safety provisions during field activities and protect the field crew from physical and chemical hazards resulting from drilling and sampling. The HASP establishes personnel responsibilities, general safe work practices, field procedures, personal protective equipment standards, decontamination procedures, and emergency action plans. The HASP will be reviewed and signed by field staff and contractors prior to beginning field operations.

SOMA will contact Underground Service Alert (USA) to ensure that drilling areas are clear of underground utilities. Following USA clearance, SOMA will retain a private utility locator to survey proposed drilling areas and locate any additional subsurface conduits.

SOMA will also survey the 54-inch storm drain, by opening manhole MH-2 and surveying the depth to the storm lateral inlets.

3.2 Evaluation of Existing Groundwater Monitoring Well Network

Three existing groundwater monitoring wells are associated with the adjacent Glovatorium Property in the vicinity of the subject site (GW-5, GW-6A, and LFR-4). Nearby wells and borings, GW-5A, GW-6, GW-7 and GW-8 were decommissioned between July 1999 and July 2000. Except for LFR-4 as noted earlier, the existing wells appear to be screened across the appropriate interval (5 to 15 feet bgs, discussed below). GW-5 is screened from 8 to 13 feet bgs, GW-6A is screened from 5 to 15 feet bgs, and LFR-4 is screened from 9 to 19 feet bgs. The wells are located 10 to 40 feet downgradient and crossgradient of the decommissioned USTs. Contamination has been observed in LFR-4 (the closest well) with TPH-g observed at 480 µg/L and TPH-ss observed at 270 µg/L during the Second Semi-Annual Monitoring Event at the Glovatorium site (September 2009 Groundwater Monitoring Report). GW-5 and GW-6A have not been monitored since 2000, at which time all contaminants were below laboratory-detection limits. As part of the proposed soil and groundwater investigation, SOMA proposes collecting groundwater samples from wells LFR-4, GW-5, and GW-6A.

3.3 Direct Push Borings, Collection of Soil and Groundwater Samples

3.3.1 Direct Push Borings

Proposed Geoprobe borings around former USTs will be advanced using DPT. As many as eight borings (DP-1 through DP-8) are proposed adjacent to and downgradient from decommissioned USTs. Final quantity of borings will be determined in the field based on access limitations or subsurface conditions. The purpose of this subsurface assessment is twofold: 1) determine the lateral extent of elevated contaminant levels reported in soil and groundwater samples; and 2) determine the necessity for permanent groundwater monitoring well installation in proximity to and downgradient from decommissioned USTs.

Subsurface soil and groundwater samples will be collected from each boring. Since confirmation borings were advanced only 1 to 2 feet beneath each tank (some borings were as shallow as 10 feet bgs) the vertical plume extent is not fully defined at this time. Therefore, each boring will be advanced to at least 30 feet bgs, since the deepest tank was installed to total depth of 25 feet bgs (Tank 1). SOMA will utilize a PID and field observations of odor and staining to determine ultimate depth of each boring. Borings advanced adjacent to decommissioned USTs will aid in determining the vertical extent of contamination; borings positioned downgradient from the source will aid in determining horizontal extent of contamination.

In order to determine whether contaminant migration via preferential pathways is occurring, proposed borings DP-2 and DP-3 are located adjacent to the storm drain, crossgradient from the site, and DP-7 is located adjacent to the storm

drain, downgradient from the site. SOMA proposes utilizing a hand auger to clear these borings beyond 9 feet bgs, to avoid damaging the storm drain. Boring DP-7 will be advanced in close proximity to LFR-4 to allow for evaluation of existing screening interval in relation to contaminant transport and geology of saturated sediments.

Existing groundwater monitoring wells GW-5, GW-6A, and LFR-4 will be also sampled prior to making a final determination regarding additional well installation, in order to evaluate possible contaminant migration via preferential pathways along the storm drain and monitor groundwater flow patterns and plume migration in the vicinity of decommissioned tanks. All collected samples will be analyzed for constituents outlined in Section 3.4.

3.3.2 Soil Sampling

DPT is an efficient method of collecting continuous soil cores while preventing cross-contamination. DPT involves hydraulically hammering a set of steel rods into the subsurface with the lead section consisting of a polyethylene-lined sampler. After pushing the drilling rods to the desired depth, the soil-filled liner will be retrieved. SOMA's field geologist will log continuous soil cores from each boring location, characterizing the content of each soil-filled tube using the Unified Soil Classification System.

Encountered subsurface lithologies from all advanced borings will be recorded on geologic borehole logs. At each interval of depth-discrete soil sampling, the DP drilling rig will obtain a 4-foot soil core sample. The contents of each sediment-filled tube will be screened using PID. Vapors from the soil core sample(s) will be screened for volatile compounds and will be documented on geologic borehole logs. SOMA proposes that soil samples be collected at depths where PID readings or visual observations indicate the presence of significant soil contamination. In addition, one soil sample will be collected from the vadose zone at the soil-groundwater interface. Absent detectable contaminants of concern in soil during field screening, a minimum of two soil samples will be collected from each soil boring. SOMA's field geologist will select and cut sections of the soil-filled tubes into 6-inch-long sections and cap ends of each sample with a Teflon liner and polyethylene end caps. Samples will be labeled and immediately placed into a chilled ice chest for transportation to a California state-certified environmental laboratory for analysis.

3.3.3 Groundwater Sampling

To collect groundwater samples at field-identified depth intervals, a hydropunch groundwater profiler will be used. It is designed for discrete groundwater sampling without cross-contaminating water-bearing zones (WBZs) at different depth intervals. The dual-walled sampler involves hydraulically driving or hammering a cased set of rods into the ground with the lead rod section

consisting of a hollow acetate-lined sampler. After pushing the cased rods to the desired depth, the drilling rods are withdrawn from within the 1.25-inch-diameter outer casing to insert the screened sampler. The field crew will use disposable bailers or a Watera sampler fitted into plastic tubing to collect grab groundwater samples.

Following soil and groundwater sampling, borings will be abandoned and sealed with a bentonite grout mixture and completed at the surface with materials to match the existing grade.

Soil and waste water generated during boring activities will be temporarily stored on-site in separate DOT-rated 55-gallon steel drums pending characterization, profiling and transport to an approved disposal/recycling facility.

3.3.4 Laboratory Analyses

Grab groundwater samples and soil samples will be submitted to a California state-certified environmental laboratory under appropriate chain-of-custody protocol for analysis of the following:

- TPH-g, TPH-d, and TPH-ss, EPA Method 8015
- Kerosene, EPA Method 8015
- BTEX, EPA Method 8260
- VOCs such as PCE, TCE, VC, and naphthalene, and gasoline oxygenates such as methyl tertiary-butyl ether (MtBE) and tertiary-butyl alcohol (TBA), EPA Method 8260
- Total lead, EPA Method 6010

3.3.5 Data Review

Soil and groundwater data collected during the DP investigation will be utilized to determine whether all data gaps have been closed. Furthermore, investigation results will be made available to the regulatory agency for final determination regarding possible future well installation.

4. REPORT PREPARATION

SOMA will prepare a full report of the additional site investigation, including the following:

 Description of field activities; tabulation of soil and groundwater sample analytical data; maps illustrating boring locations; and description of lateral and vertical extent of impacted soil and groundwater utilizing all available up-to-date data.

- Conclusions regarding lateral and vertical extent of impact at the assessment area of concern, based on data and information derived from field work and laboratory analysis.
- Proposed further actions. If it appears that additional monitoring wells are necessary, a workplan detailing proposed well locations and installation procedures will be prepared for review.
- Prescreening evaluation of possible remedial technologies for addressing soil and groundwater contamination at the site.

Tables

Table 1: Soil Analytical Results (TPH and BTEX) November 20 and 21, 2008 316 38th Street, Oakland

Borehole	Depth ¹	TPH-g	TPH-d	TPH-ss	Kerosene	Benzene	Toluene	Ethyl-benzene	Total Xylenes
Dorellole	(feet bgs)	(mg/kg)	(mg/kg)	(mg/kg)	(mg/kg)	(mg/kg)	(mg/kg)	(mg/kg)	(mg/kg)
TB1-1	10	3.1	3.3	4.5	6	< 0.023	<0.023	<0.023	<0.047
TB1-1	12	850	260	340	340	<2.4	<2.4	<2.4	<4.9
TB1-1	18	0.91	110	170	150	<0.005	<0.005	<0.005	<0.0099
TB1-1	27	<0.23	< 0.99	< 0.99	< 0.99	<0.005	<0.005	<0.005	<0.01
TB1-3	10	7.3	12	21	22	NA	NA	NA	NA
TB1-3	12	300	35	58	53	<0.97	<0.97	< 0.97	<1.9
TB1-3	14	1,600	66	120	110	<0.022	<0.022	<0.022	<0.043
TB1-3	27	<0.23	1.0	< 0.99	2.8	<0.005	<0.005	<0.005	<0.01
TB1-4	24	<0.24	<1.0	1.8	3.7	<0.0047	<0.0047	<0.0047	<0.0095
TB1-4	27	2.0	<1.0	2.2	4.0	<0.005	<0.005	<0.005	<0.01
TB2-1	6	750	18	39	35	<0.005	<0.005	<0.005	<0.0099
TB2-1	10	120	1.8	1.7	3.6	<0.0049	<0.0049	<0.0049	<0.0099
TB2-2	6	250	15	28	27	< 0.96	< 0.96	< 0.96	<1.9
TB2-2	8	3,900	630	950	950	<2.5	<2.5	<2.5	<4.9
TB2-2	10	140	79	150	130	<0.012	<0.012	<0.012	<0.024
TB3-1	6	< 0.25	2.5	1.1	1.4	< 0.005	<0.005	<0.005	<0.01
TB3-1	8	220	4.4	4	7.4	<1.9	<1.9	<1.9	<3.9
TB3-1	14	3,800	81	130	120	<0.024	<0.024	0.036	<0.048
TB3-1	17	<0.24	<1.0	1.4	3.3	<0.005	<0.005	<0.005	<0.01
TB3-2	6	<0.25	31	2.5	5	< 0.005	<0.005	<0.005	<0.0099
TB3-2	12	2,100	12	15	17	<4.9	<4.9	<4.9	<9.7
TB3-2	14	3,200	5.5	5.5	7.9	<0.0049	<0.0049	<0.0049	<0.0099
TB3-2	14	4,100	NA	NA	NA	<0.0048	<0.0048	<0.0048	<0.0096
TB3-2	17	210	3.7	5.6	7.0	<0.0049	<0.0049	0.024	0.022
ESL - Shallow Soil <3m bgs		83	83	83	83	0.044	2.9	2.3	2.3
ESL - Deep Soil >3m bgs		83	83	83	83	0.044	2.9	3.3	2.3

Notes:

TPH-g: Total Petroleum Hydrocarbons as Gasoline (C5-C12) TPH-d: Total Petroleum Hydrocarbons as Diesel (C9-C19)

TPH-ss: Total Petroleum Hydrocarbons as Stoddard Solvents (C9-C13) TPH by EPA Method 8015, BTEX by EPA Method 8260B/CA_LUFTMS

ESL: California Regional Water Control Board Environmental Screening levels, Interim Final November 2007, Revised May 2008, Tables A and C

Table 2: Soil Analytical Results (VOC compounds) November 20 and 21, 2008 316 38th Street, Oakland

Borehole	Depth ¹	PCE	TCE	Vinyl Chloride	TBA	MtBE	1,2-DCA	Cis-1,2 DCE	Napthalene
Borenoie	(feet bgs)	(mg/kg)	(mg/kg)	(mg/kg)	(mg/kg)	(mg/kg)	(mg/kg)	(mg/kg)	(mg/kg)
TB1-1	12	<0.005	<0.005	< 0.005	NA	NA	NA	NA	NA
TB1-1	18	<0.005	<0.005	< 0.005	<0.0095	<0.0049	<0.005	< 0.005	<0.0099
TB1-1	27	<0.005	<0.005	< 0.005	< 0.0094	< 0.0047	< 0.005	< 0.005	<0.01
TB1-3	12	<0.005	<0.005	< 0.005	NA	NA	NA	NA	NA
TB1-3	14	<0.022	<0.022	<0.022	<1.9	< 0.97	<0.97	<0.022	<0.043
TB1-3	27	<0.005	<0.005	< 0.005	<0.01	<0.005	<0.005	< 0.005	<0.01
TB1-4	24	<0.005	<0.005	< 0.005	NA	NA	NA	NA	NA
TB1-4	27	<0.005	<0.005	< 0.005	<0.0095	<0.0047	<0.0047	<0.005	<0.01
TB2-1	6	<0.005	<0.005	< 0.005	<1.9	< 0.96	<0.96	< 0.005	<0.0099
TB2-1	10	< 0.0049	<0.0049	<0.0049	<1.9	< 0.94	<0.94	<0.0049	<0.0099
TB2-2	6	<0.0049	<0.0049	<0.0049	NA	NA	NA	NA	NA
TB2-2	10	<0.012	<0.012	<0.012	<2.0	<0.98	<0.98	<0.012	<0.024
TB3-1	6	< 0.0049	<0.0049	<0.0049	NA	NA	NA	NA	NA
TB3-1	14	<0.024	<0.024	<0.024	<5.0	<2.5	<2.5	<0.024	0.480
TB3-1	17	<0.005	<0.005	< 0.005	<0.0096	<0.0048	<0.0048	<0.005	<0.01
TB3-2	6	<0.005	<0.005	< 0.005	NA	NA	NA	NA	NA
TB3-2	14	<0.0049	<0.0049	<0.0049	<1.9	< 0.96	<0.96	<0.0049	<0.0099
TB3-2	14	NA	NA	NA	<9.6	<4.8	<4.8	<0.0049	<0.0099
TB3-2	17	<0.0049	<0.0049	<0.0049	<1.9	< 0.93	< 0.93	<0.0049	0.018
ESL - Shallow Soil <3m bgs		0.37	0.46	0.022	0.075	0.023	0.0045	0.19	1.3
ESL - Deep Soil >3m bgs		0.70	0.46	0.085	0.075	0.023	0.0045	0.19	3.4

Notes:

NA: Not Analyzed

TBA, MtBE, 1,2-DCA by Method 8260B/CA_LUFTMS

PCE, TCE, Vinyl Chloride, Cis-DCE, and Napthalene by EPA Method 8260B

Environmental Screening levels or Laboratory Detection levels

ESL: California Regional Water Control Board Environmental Screening levels, Interim Final November 2007, Revised May 2008, Tables A and C

Table 3: Groundwater Analytical Results (TPH and BTEX)
November 20 and 21, 2008
316 38th Street, Oakland

Borehole	TPH-g	TPH-d	TPH-ss	Kerosene	Benzene	Toluene	Ethyl-benzene	Total Xylenes
Borenole	(ug/L)	(ug/L)	(ug/L)	(ug/L)	(ug/L)	(ug/L)	(ug/L)	(ug/L)
TB1-1	2,600	7,400	2,700	7,500	<0.5	<0.50	< 0.50	<1.0
TB1-3	29,000	8,700	7,900	12,000	0.54	<0.50	<0.50	1,700
TB1-4	1,400	290	520	600	0.75	10	6.50	59
TB2-1	28,000	52,000	110,000	97,000	<5.0	<5.0	9.10	<10
TB2-2	12,000	330,000	560,000	560,000	<5.0	<5.0	<5.0	<10
TB3-1	1,100	700	490	730	22	<0.50	2.10	5.8
TB3-2	890	230	140	170	<0.50	<0.50	0.55	<1.0
ESL - Groundwater is a Current/Potential Source of Drinking Water	100	100	100	100	1	40	30	20

Notes:

TPH-g: Total Petroleum Hydrocarbons as Gasoline (C5-C12)

TPH-d: Total Petroleum Hydrocarbons as Diesel (C9-C19)

TPH-ss: Total Petroleum Hydrocarbons as Stoddard Solvents (C9-C13)

TPH by EPA Method 8015, BTEX by EPA Method 8260B/CA_LUFTMS

ESL: California Regional Water Control Board Environmental Screening levels, Interim Final November 2007, Revised May 2008, Tables F-1a

Table 4: Groundwater Analytical Results (VOC compounds)
November 20 and 21, 2008
316 38th Street, Oakland

Borehole	PCE	TCE	Vinyl Chloride	TBA	MtBE	1,2-DCA	Cis-1,2 DCE	Napthalene
Borenole	(ug/L)	(ug/L)	(ug/L)	(ug/L)	(ug/L)	(ug/L)	(ug/L)	(ug/L)
TB1-1	<0.50	<0.50	<0.50	9.3	<0.50	<0.50	<0.50	<1.0
TB1-3	< 0.50	<0.50	<0.50	28	<0.50	<0.50	<0.50	<1.0
TB1-4	1.8	1.1	< 0.50	7.3	<0.50	3.6	0.69	<1.0
TB2-1	< 0.50	<0.50	<0.50	<50	<5.0	<5.0	0.81	98
TB2-2	<0.50	<0.50	<0.50	<50	<5.0	<5.0	<0.50	1.5
TB3-1	< 0.50	<0.50	<0.50	<5.0	1.30	<0.50	1.0	19
TB3-2	< 0.50	<0.50	<0.50	<5.0	<0.50	< 0.50	<0.50	<1.0
ESL - Groundwater is a Current/Potential Source of Drinking Water	5	5	0.5	12	5	0.5	6	17

Notes:

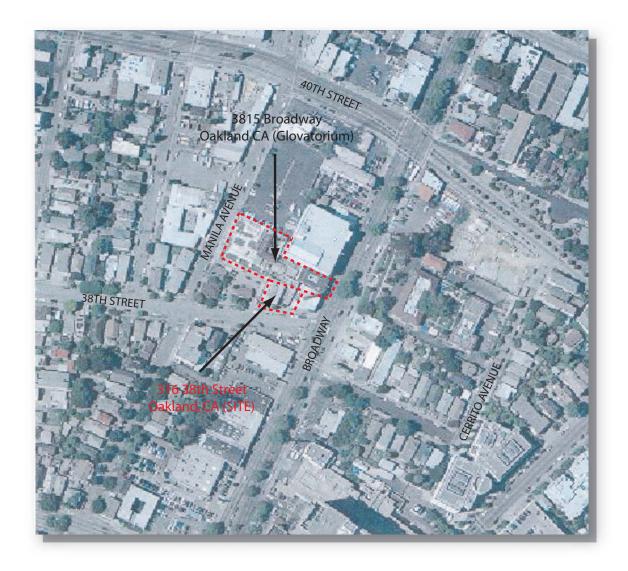
TBA, MtBE, 1,2-DCA by Method 8260B/CA_LUFTMS

PCE, TCE, Vinyl Chloride, Cis-DCE, and Napthalene by EPA Method 8260B

Semi-volatie organic compounds analyized by EPA Method 8270 were below Laboratory Detection levels

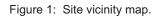
ESL: California Regional Water Control Board Environmental Screening levels, Interim Final November 2007, Revised May 2008, Tables F-1a

Figures

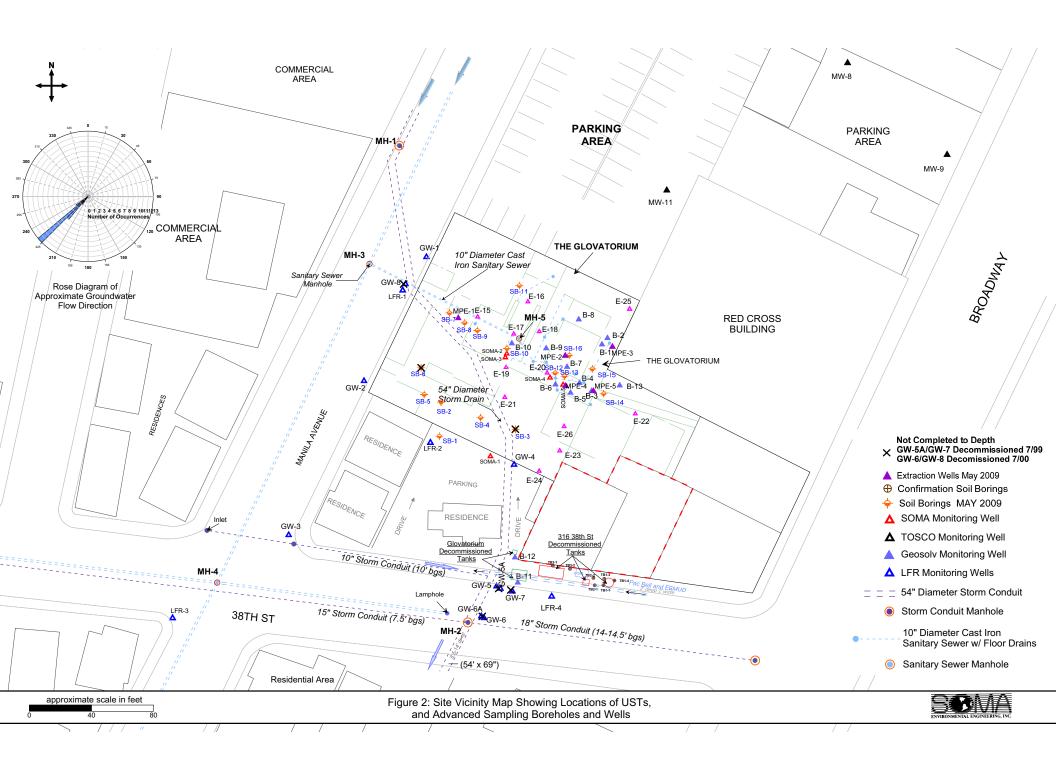


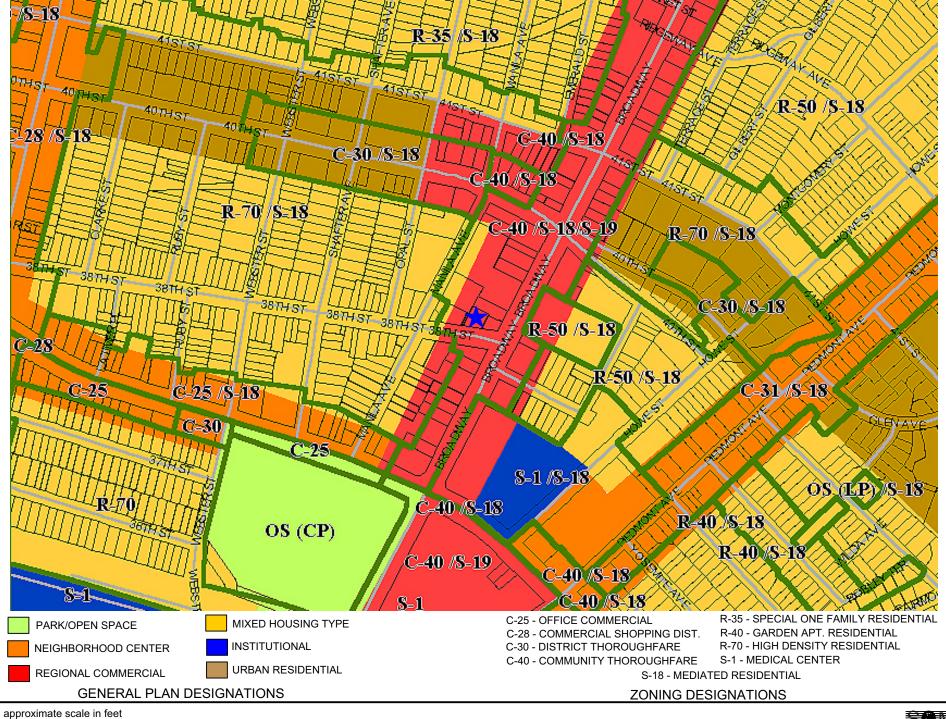


approximate scale in feet
250 500



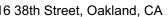




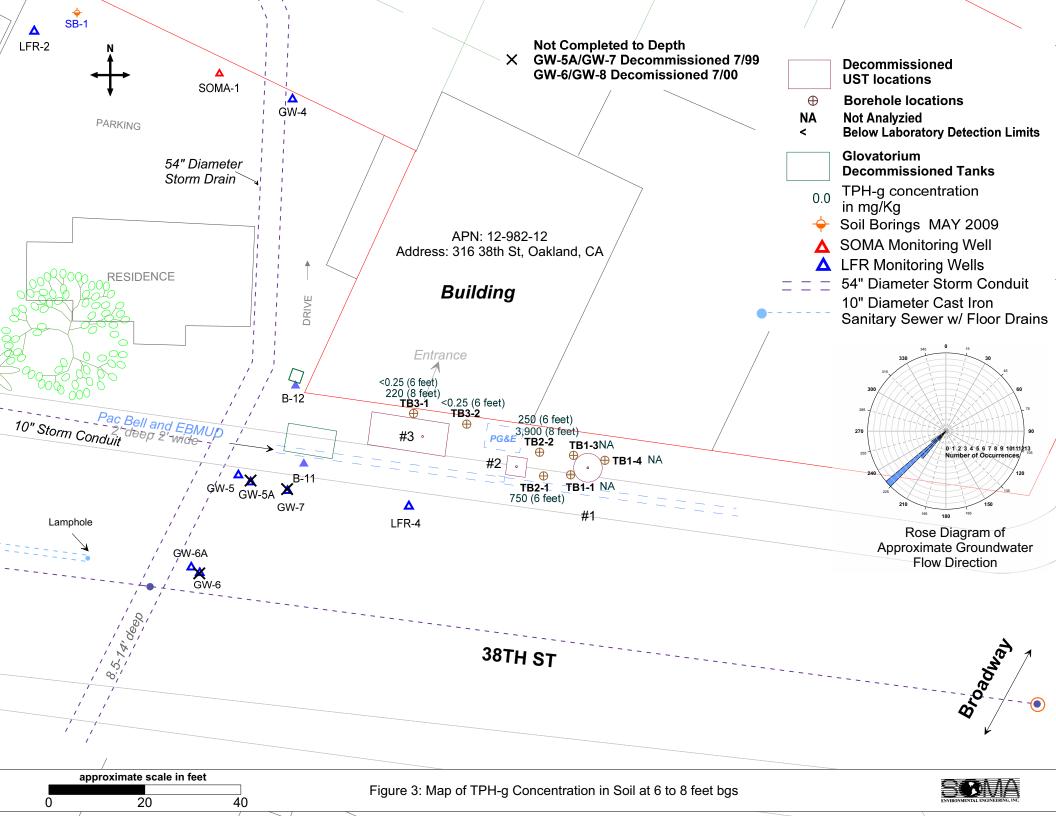


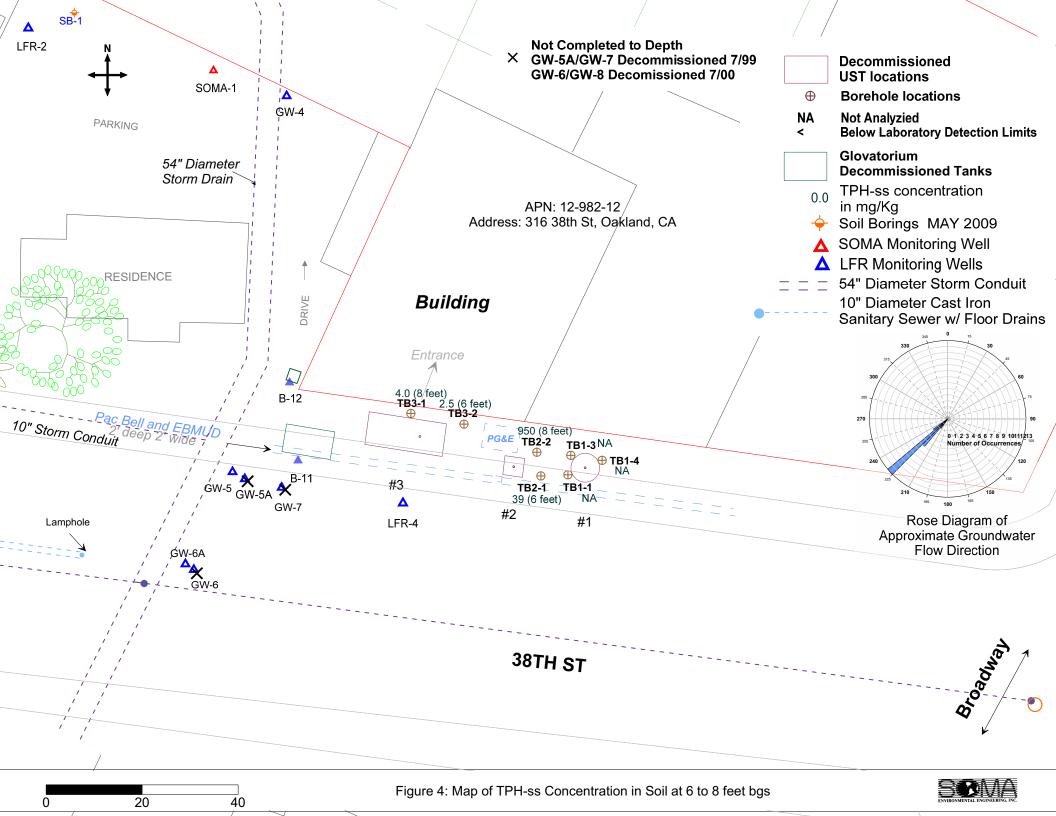


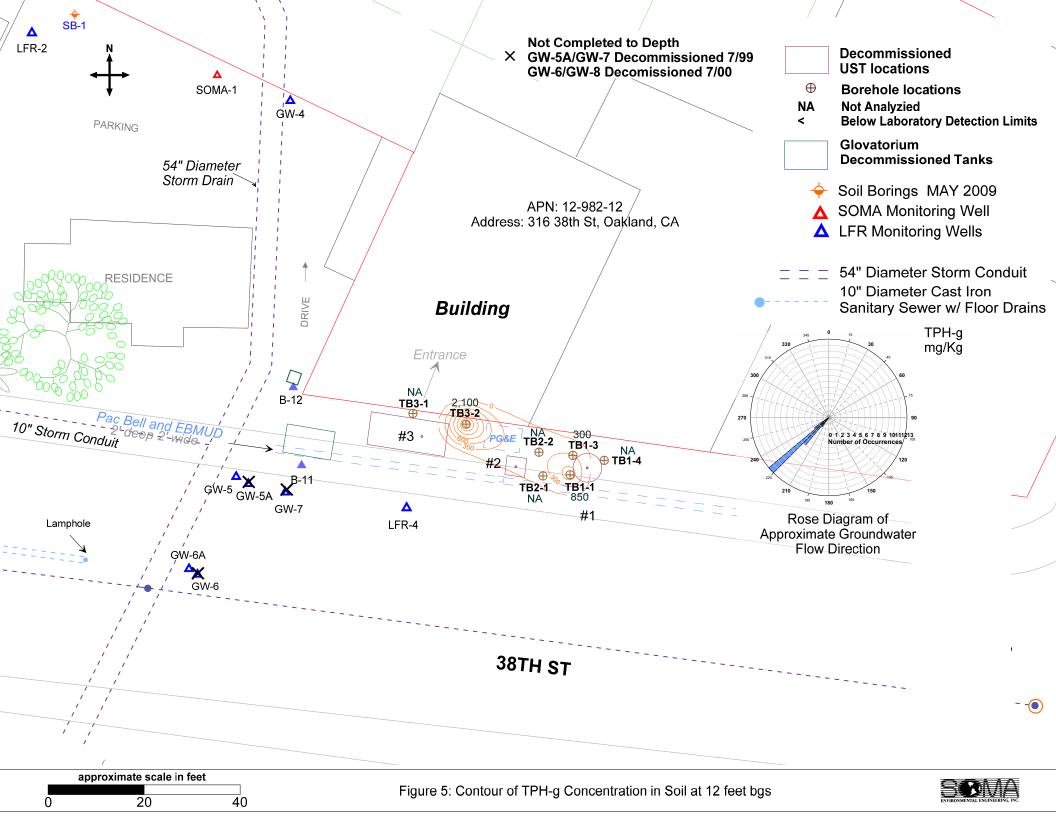
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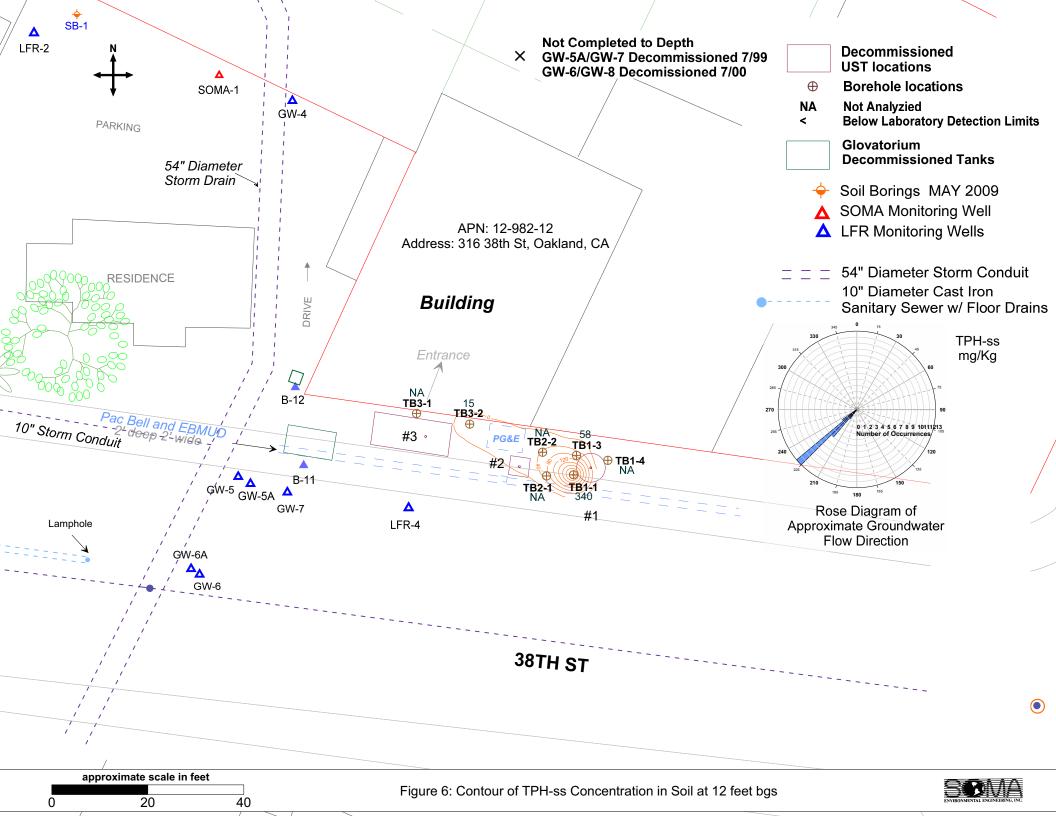


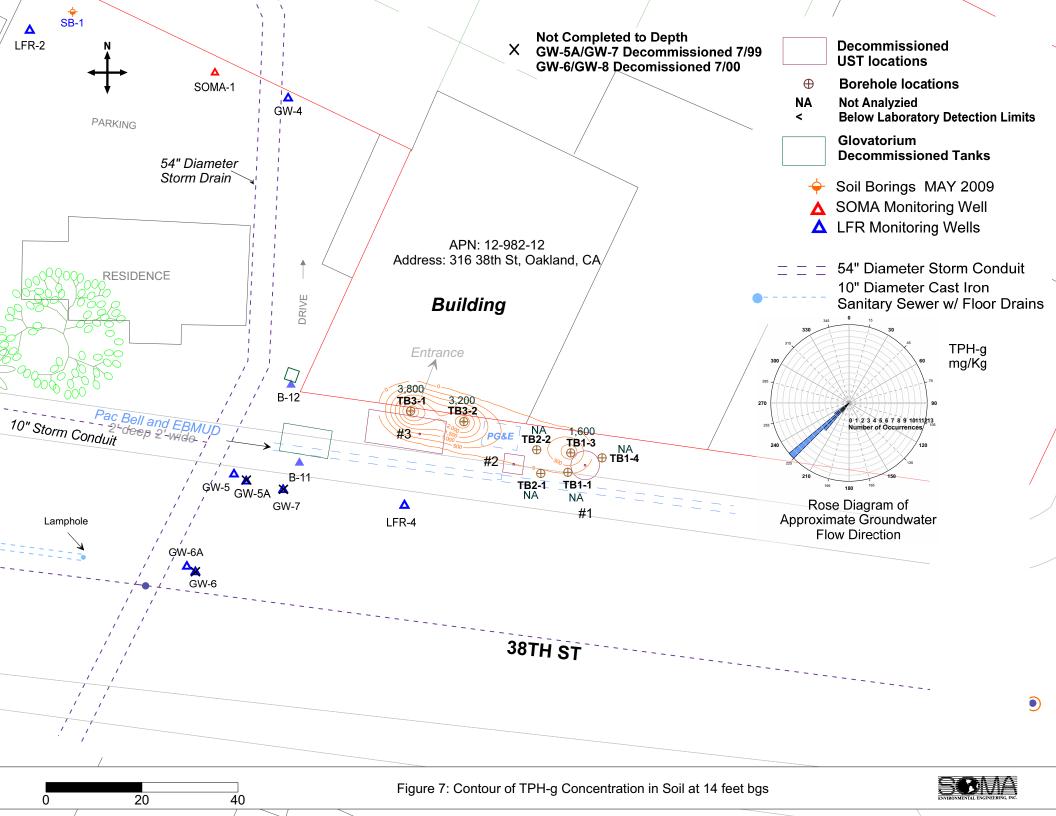


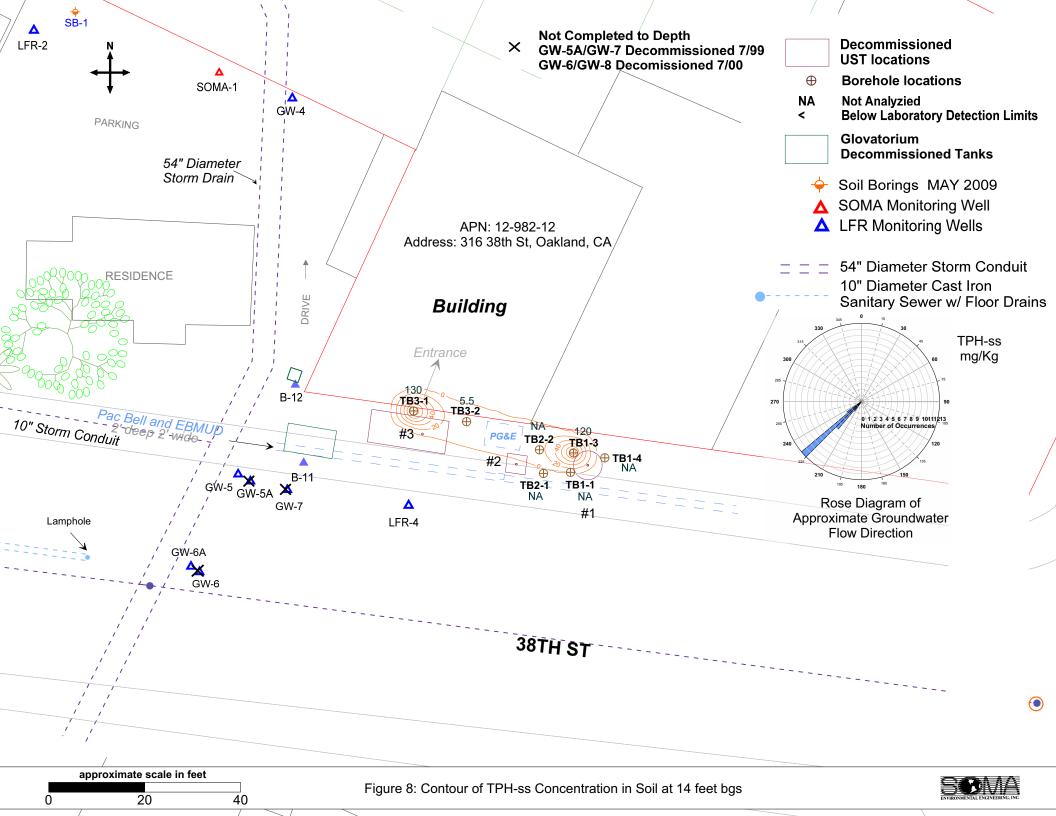


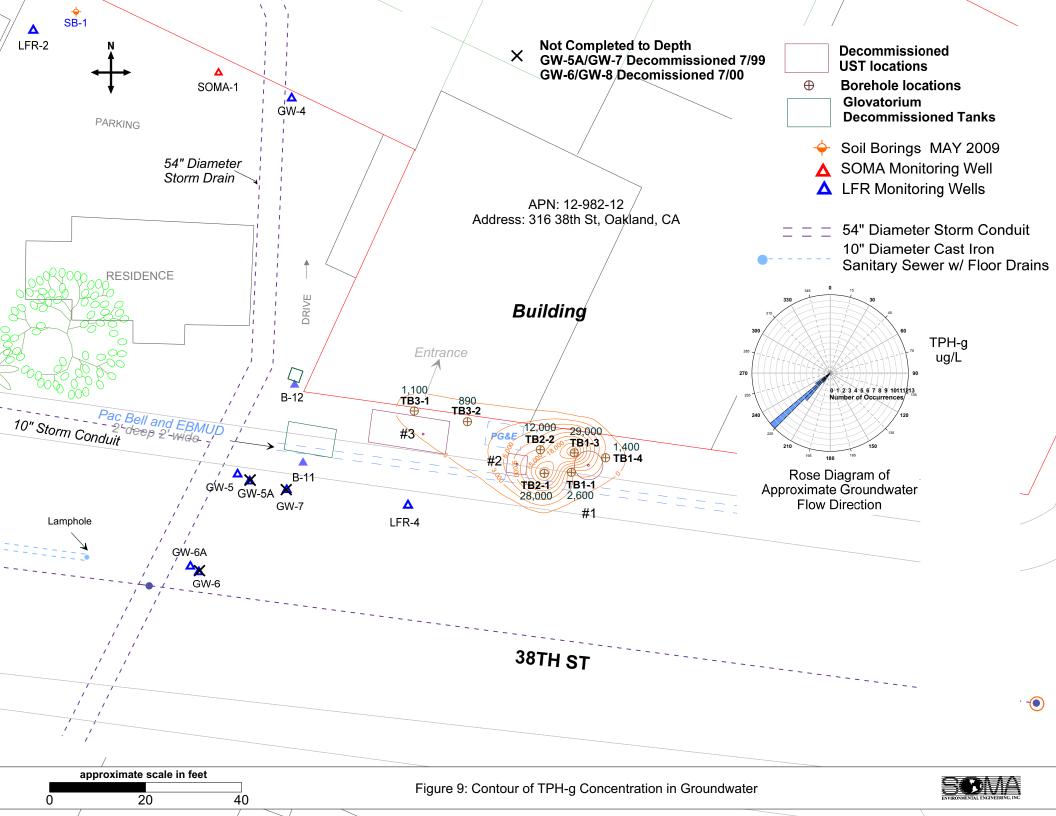


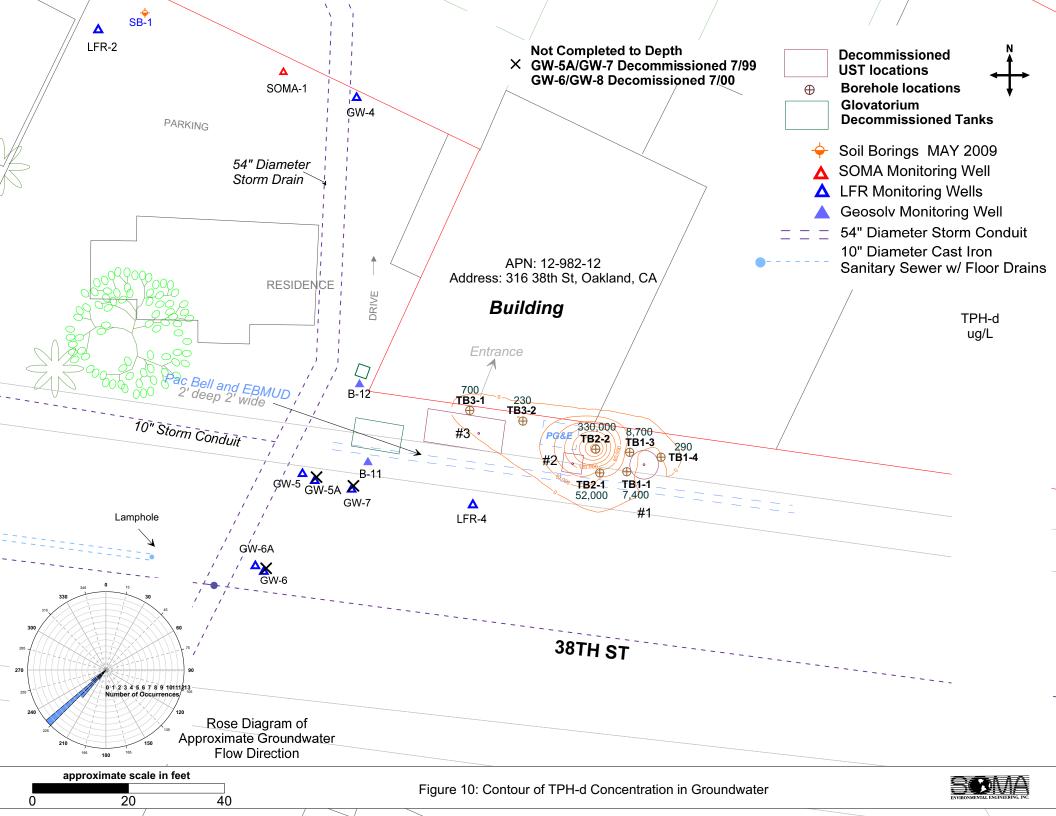


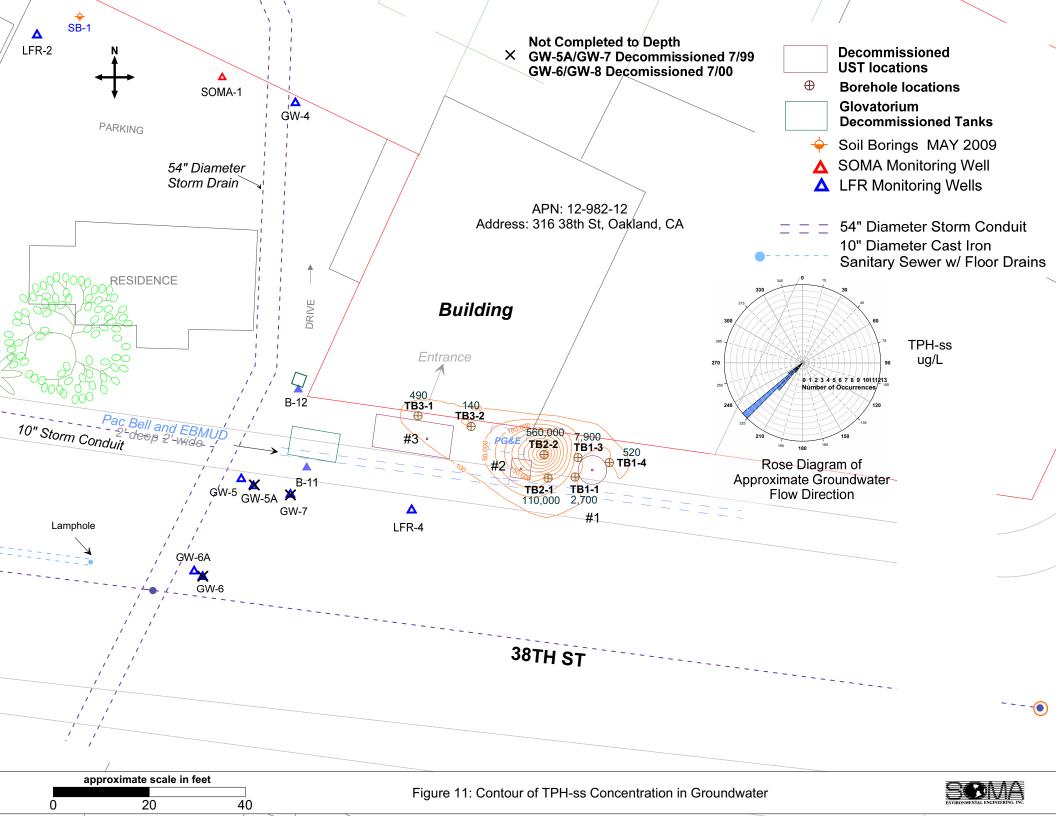


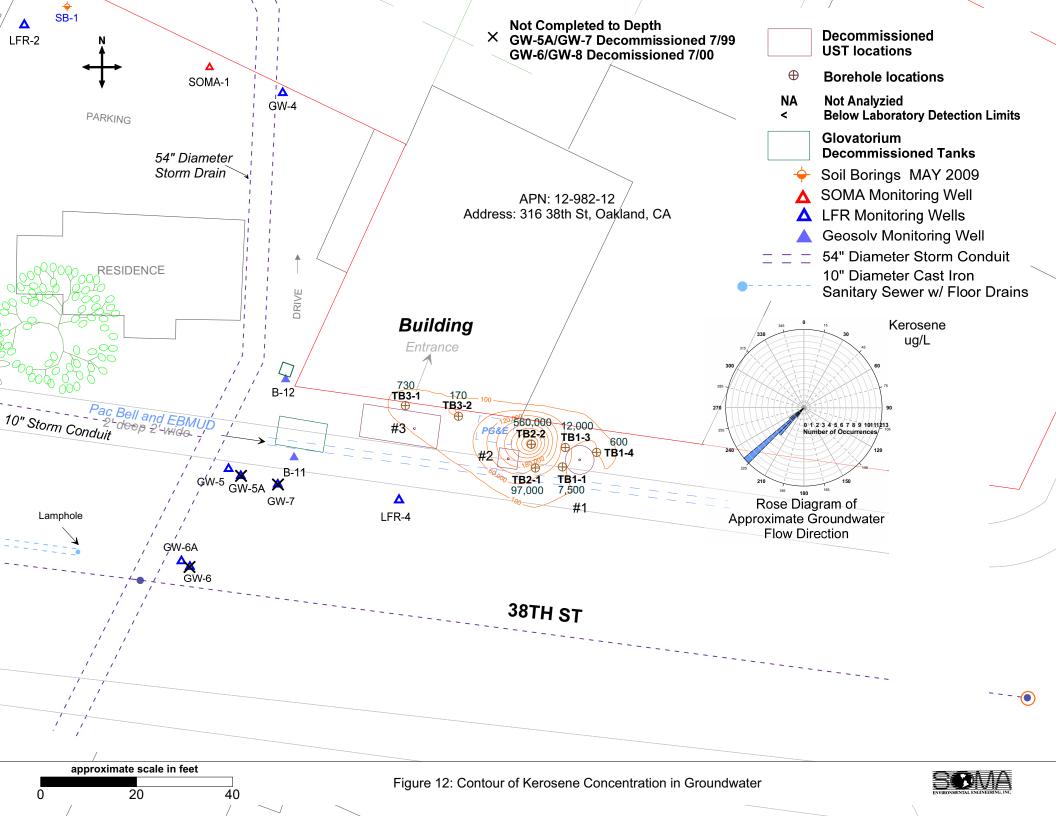


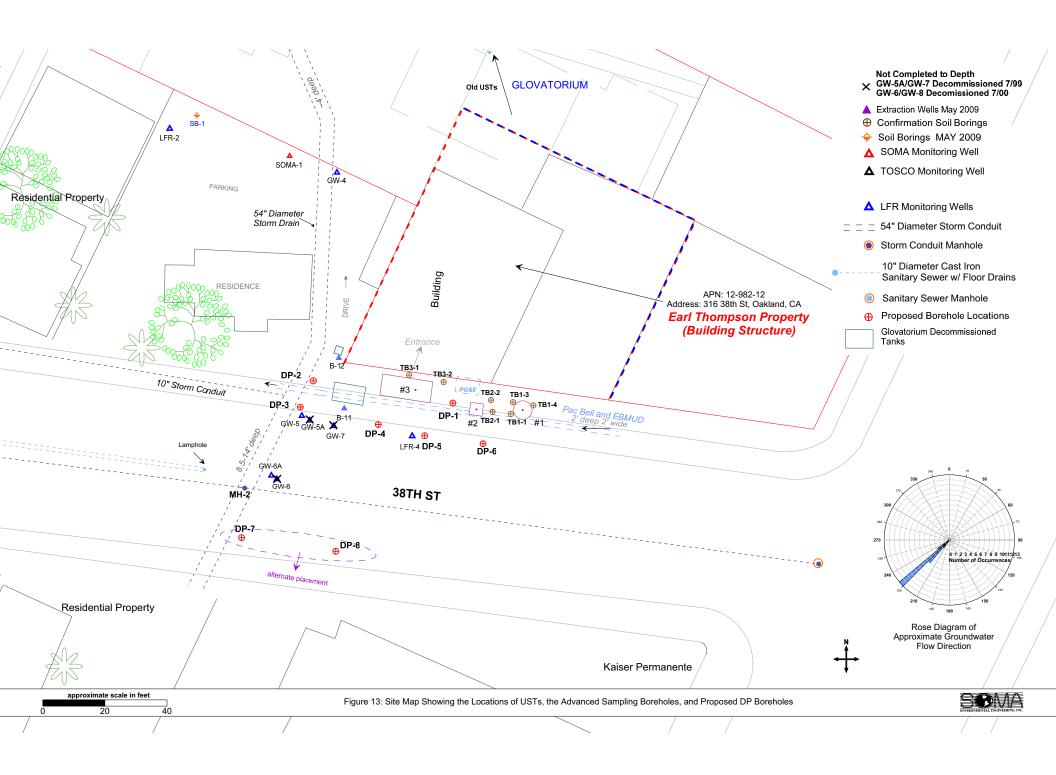












Appendix A Historical Boring Logs



PAGE 1 OF 2

PROJECT: 2722

DATE DRILLED: 11/21/2008

SITE LOCATION: 316 38th St., Oakland

CASING ELEVATION: N/A

DRILLER: Fisch Drilling

DEPTH TO GW: 12 ft. bgs.

DRILLING METHOD: Direct Push (DP)

T.O.C. TO SCREEN: N/A

BORING DIAMETER: 2.25"

SCREEN LENGTH: N/A

LOGGED BY: E. Hightower

APPROVED BY: M. Sepehr

PID ppm	DEРТН	GRAPHIC LOG	SOIL CLASS	GEOLOGIC DESCRIPTION	SPLIT SPOON	CORE	GW LEVEL	BLOWCOUNTS	WELL DIAGRAM
	- - 5— - -			HAND AUGER TO 10 ft. BGS.					
122.3 41.1	10		CL	LEAN CLAY w/sand: Dark greenish-brown; stiff; moist; LEK; fine to coarse-grained sand; slight Petroelum Hydrocarbon (PHC) odor. POORLY GRADED GRAVEL w/sand: Greenish-gray; medium stiff; wet; fine to			∇		
78.3 82.3 1	- 15—		CL	coarse-grained sand; fine- to medium-grained gravel; PHC odor. Sandy Lean Clay with Gravel: Greenish-gray; medium-stiff; moist; fine- to coarse-grained sand; fine- to medium-grained gravel; PHC odor.					
300	- -	-	SP ML	POORLY GRADED SAND: Grayish-green; medium dense; very moist to wet; fine- to medium-grained sand; PHC odor. SILT: Grayish-green; medium stiff; moist; PHC odor.		TB1-1 @ 18 ft			
135 35.7	20	-	SW	WELL GRADED SAND: Grayish-green; soft; moist to wet; fine-grained sand; PHC odor. SILT: Grayish-green; soft; moist to very moist; PHC odor.					
106.2	- 25—		CL	LEAN CLAY: Tan; stiff; moist; LEK; slight PHC odor.					

COMMENTS:



PAGE 2 OF 2

PROJECT: 2722

SITE LOCATION: 316 38th St., Oakland

DRILLER: Fisch Drilling

DRILLING METHOD: Direct Push (DP)

BORING DIAMETER: 2.25"

LOGGED BY: E. Hightower

DATE DRILLED: 11/21/2008

CASING ELEVATION: N/A

DEPTH TO GW: 12 ft. bgs.

T.O.C. TO SCREEN: N/A

SCREEN LENGTH: N/A

PID ppm	DEPTH	GRAPHIC LOG	SOIL CLASS	GEOLOGIC DESCRIPTION	SPLIT SPOON	CORE SAMPLE	GW LEVEL	BLOWCOUNTS	WELL DIAGRAM
15.2 31.4	-		CL	LEAN CLAY: Tan; stiff; moist; LEK; slight PHC odor.		TB1-1 @ 27 ft			
	-					TB1			
	30-								
	-								
	35—								
	- -								
	40— -								
	-								
	45— _								
	-								
	50—								



PAGE 1 OF 2

PROJECT: 2722

DATE DRILLED: 11/20/2008

SITE LOCATION: 316 38th St., Oakland

CASING ELEVATION: N/A

DRILLER: Fisch Drilling

DEPTH TO GW: 12 ft. bgs.

DRILLING METHOD: Direct Push (DP)

T.O.C. TO SCREEN: N/A

BORING DIAMETER: 2.25"

SCREEN LENGTH: N/A

LOGGED BY: E. Hightower

APPROVED BY: M. Sepehr

PID ppm	DЕРТН	GRAPHIC LOG	SOIL CLASS	GEOLOGIC DESCRIPTION	SPLIT SPOON	CORE SAMPLED	GW LEVEL	BLOWCOUNTS	WELL DIAGRAM
278 294 342 45.6 5	5—		CL GP SP ML	LEAN CLAY w/sand: Dark greenish-brown; stiff; moist; LEK; fine- to coarse-grained sand; slight Petroleum Hydrocarbon (PHC) odor. POORLY GRADED GRAVEL w/sand: Greenish-gray; medium stiff; wet; fine to coarse-grained sand; fine- to coarse-grained gravel; PHC odor. POORLY GRADED SAND: Grayish-green; medium dense; very moist to wet; fine- to medium-grained sand; slight PHC odor. SILT: Grayish-green; medium stiff; moist; slight PHC odor. Becomes wet from 20-22 ft.	8	TB₁3@14ff α	→	3	
5.5				LEAN CLAY: Tan; very stiff; moist; LEK; very slight PHC odor.					
4.7	_		CL						

COMMENTS:



PAGE 2 OF 2

PROJECT: 2722

SITE LOCATION: 316 38th St., Oakland

DRILLER: Fisch Drilling

DRILLING METHOD: Direct Push (DP)

BORING DIAMETER: 2.25"

LOGGED BY: E. Hightower

DATE DRILLED: 11/20/2008

CASING ELEVATION: N/A

DEPTH TO GW: 12 ft. bgs.

T.O.C. TO SCREEN: N/A

SCREEN LENGTH: N/A

PID ppm	DEPTH	GRAPHIC LOG	SOIL CLASS	GEOLOGIC DESCRIPTION	SPLIT SPOON	CORE SAMPLE	GW LEVEL	BLOWCOUNTS	WELL DIAGRAM
1.5 1.5	-		CL	LEAN CLAY: Tan; very stiff; moist; LEK; very slight PHC odor.		TB1-3 @ 27 ft			
	-					TB1-			
	30-								
	- -								
	35— -								
	-								
	40—								
	-								
	45 								
	-								
	- 50—								



PAGE 1 OF 2

PROJECT: 2722

DATE DRILLED: 11/21/2008

SITE LOCATION: 316 38th St., Oakland

CASING ELEVATION: N/A

DRILLER: Fisch Drilling

DEPTH TO GW: 12 ft. bgs.

DRILLING METHOD: Direct Push (DP)

T.O.C. TO SCREEN: N/A

BORING DIAMETER: 2.25"

SCREEN LENGTH: N/A

LOGGED BY: E. Hightower

APPROVED BY: M. Sepehr

	LC	OOLD L) i . L. i i	ilgittower At 1 NOVED D1. W. Ocpor	"				
PID ppm	DEРТН	GRAPHIC LOG	SOIL CLASS	GEOLOGIC DESCRIPTION	SPLIT SPOON	CORE SAMPLED	GW LEVEL	BLOWCOUNTS	WELL DIAGRAM
1 48 10.4	5			LEAN CLAY w/sand: Dark brown; soft; very moist; no Petroleum Hydrocarbon (PHC) odor. Saturated at 12 ft. No Recovery LEAN CLAY: Tan; stiff; moist; LEK; no PHC odor.			∇		
		OMMENT	S:						



PAGE 2 OF 2

PROJECT: 2722

SITE LOCATION: 316 38th St., Oakland

DRILLER: Fisch Drilling

DRILLING METHOD: Direct Push (DP)

BORING DIAMETER: 2.25"

LOGGED BY: E. Hightower

DATE DRILLED: 11/21/2008

CASING ELEVATION: N/A

DEPTH TO GW: 12 ft. bgs.

T.O.C. TO SCREEN: N/A

SCREEN LENGTH: N/A

PID ppm	ОЕРТН	GRAPHIC LOG	SOIL CLASS	GEOLOGIC DESCRIPTION	SPLIT SPOON	CORE SAMPLE	GW LEVEL	BLOWCOUNTS	WELL DIAGRAM
	_		CL	LEAN CLAY: Tan; stiff; moist; LEK; no PHC odor.		TB1-4 @ 27 ft			
	_					TB1-			
	30-								
	-								
	- 35—								
	<u>-</u>								
	-								
	40— _								
	_								
	45—								
	-								
	- 50—								



PAGE 1 OF 1

PROJECT: 2722

DATE DRILLED: 11/20/2008

SITE LOCATION: 316 38th St., Oakland

CASING ELEVATION: N/A

DRILLER: Fisch Drilling

DEPTH TO GW: 7 ft. bgs.

DRILLING METHOD: Direct Push (DP)

T.O.C. TO SCREEN: N/A

BORING DIAMETER: 2.25"

SCREEN LENGTH: N/A

LOGGED BY: E. Hightower

APPROVED BY: M. Sepehr

PID ppm	DEPTH	GRAPHIC LOG	SOIL CLASS	GEOLOGIC DESCRIPTION	SPLIT SPOON	CORE SAMPLEL	GW LEVEL	BLOWCOUNTS	WELL DIAGRAM
27.8 22 124.4	5		CL	LEAN CLAY w/sand: Black with green mottling; soft; moist; LEK-MEK; fine- to coarse-grained sand; slight Petroleum Hydrocarbon (PHC) odor. Wet from 7-7.5 ft. Becomes very stiff at 8'		TB2-1@10ft TB2-1@6ft			

COMMENTS: TD @ 10 ft. bgs.



PAGE 1 OF 1

PROJECT: 2722

DATE DRILLED: 11/20/2008

SITE LOCATION: 316 38th St., Oakland

CASING ELEVATION: N/A

DRILLER: Fisch Drilling

DEPTH TO GW: 7 ft. bgs.

DRILLING METHOD: Direct Push (DP)

T.O.C. TO SCREEN: N/A

BORING DIAMETER: 2.25"

SCREEN LENGTH: N/A

LOGGED BY: E. Hightower

APPROVED BY: M. Sepehr

PID ppm DEPTH	GRAPHIC LOG	SOIL CLASS	GEOLOGIC DESCRIPTION	SPLIT SPOON	CORE SAMPLED	GW LEVEL	BLOWCOUNTS	WELL DIAGRAM
5- 10- 15- 20-		CL	LEAN CLAY w/sand: Greenish-brown with black mottling; medium stiff; moist; LEK-MEK; fine- to coarse-grained sand; slight Petroleum Hydrocarbon (PHC) odor. LEAN CLAY w/sand: Black; soft; moist to wet; LEK-MEK; fine- to coarse-grained sand; slight PHC odor.		TB2-2@10ft			

COMMENTS: TD @ 10 ft. bgs.



PAGE 1 OF 1

PROJECT: 2722

DATE DRILLED: 11/21/2008

SITE LOCATION: 316 38th St., Oakland

CASING ELEVATION: N/A

DRILLER: Fisch Drilling

DEPTH TO GW: 8 ft. bgs.

DRILLING METHOD: Direct Push (DP)

T.O.C. TO SCREEN: N/A

BORING DIAMETER: 2.25"

SCREEN LENGTH: N/A

LOGGED BY: E. Hightower

PID ppm	DEPTH	GRAPHIC LOG	SOIL CLASS	GEOLOGIC DESCRIPTION	SPLIT SPOON	OORE SAMPLED	GW LEVEL	BLOWCOUNTS	WELL DIAGRAM
	5— - 10— - 20— - 25—		SP CL-ML	SAND (Fill): Brown; loose; dry; MEK; fine- to medium-grained sand; no Petroleum Hydrocarbon (PHC) odor. LEAN CLAY w/sand: Dark greenish-gray; soft; moist to wet; LEK-MEK; fine- to medium-grained sand; no PHC odor. PHC odor increases at 13 ft. bgs. SILTY CLAY: Greenish-gray; medium stiff; moist; LEK; PHC odor.		TB3-1@17ft TB3/1@14ft			
	C	OMMENT	S:						



PAGE 1 OF 1

PROJECT: 2722

DATE DRILLED: 11/21/2008

SITE LOCATION: 316 38th St., Oakland

CASING ELEVATION: N/A

DRILLER: Fisch Drilling

DEPTH TO GW: 5 ft. bgs.

DRILLING METHOD: Direct Push (DP)

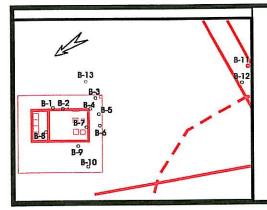
T.O.C. TO SCREEN: N/A

BORING DIAMETER: 2.25"

SCREEN LENGTH: N/A

LOGGED BY: E. Hightower

PID ppm	ОЕРТН	GRAPHIC LOG	SOIL CLASS	GEOLOGIC DESCRIPTION	SPLIT SPOON SAMPIED	CORE	GW LEVEL	BLOWCOUNTS	WELL DIAGRAM
23.3 4.3	- - - 5— - - - 10—		SP	HAND AUGER TO 5 FT BGS SAND (Fill): Brown; loose; saturated; fine- to medium-grained sand; no Petroleum Hydrocarbon (PHC) odor.			\bigvee		
34.2 7.7	- - 15 - - - 20 - - -			LEAN CLAY w/sand: Dark greenish-gray; soft; very moist; LEK-MEK; fine- to medium-grained sand; slight PHC odor. SILTY CLAY: Greenish-gray; medium stiff; moist; LEK; PHC odor.		TB3-2@17ft TB3-2@14ft			



DRILLING LOG

B11 BORING NO.

Depper PROJECT NO. 3815 Broadway, Oakland, California Frank Goldman DATE: 08/22/97 Precision SAMPLER:

221 TOTAL DEPTH:

2.5"

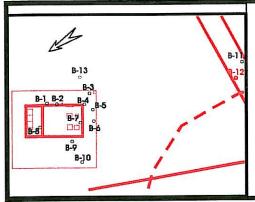
PROJECT NAME: ADDRESS: FIELD GEOLOGIST:

DRILLING COMPANY:

DRILLING METHOD:

BORING DIAMETER:

DEPTH	SAMPLE RECOVERY	BLOW	Old [mdd]	BORING CONSTR.	roe riihoroeic	USCS SYMBOLS	LITHOLOGIC DESCRIPTION Description, Grain Size, Sorting, Color, Moisture, Mechanical Properties
						SW	Sand, reddish brown, medium to coarse, very dense, dry to slightly moist; no odor.
						SP CL	Sand, medium grained, orange, dense, slightly moist; no odor. Gravelly clay, brown, stiff, moist; mottled; no odor.
-10- 						CL	Giavelly Clay, blown, sim, moist, momea, no odoi.
15 					 	CL ML	Silty clay, grayish green, firm to stiff, moist; no odor. Sandy silt, green, firm, moist; no odor.
20					111414 	SP	Silty sand, green, moderate dense, fine to medium, wet; no odor.
-20 						ML	Sandy silt, light brown, stiff, slightly moist to moist.
 -25 			上版	Grour	of boring at 2 ndwater enco g converted i	ounter	ed at 21'.
- 30-			Dish	V 515 472			
- - - -					,		
 -35 							
- 40 							



DRILLING LOG

B12 BORING NO.

Depper PROJECT NO. 3815 Broadway, Oakland, California Frank Goldman DATE: 08/22/97

Precision SAMPLER:

20'

2.5"

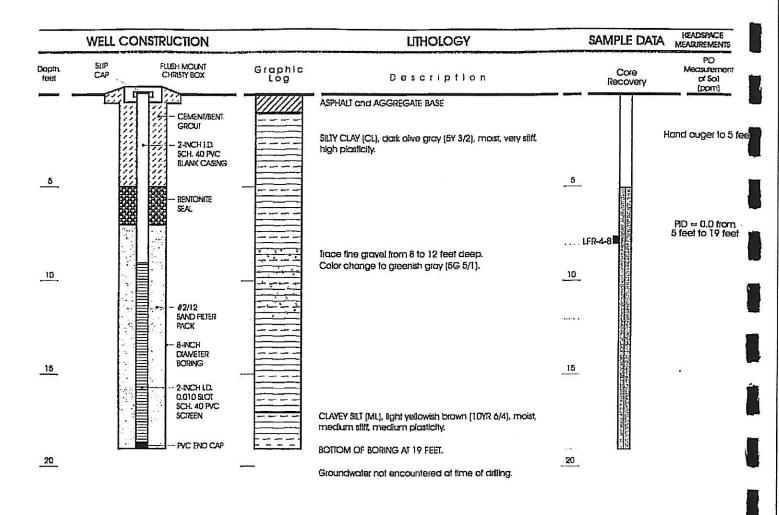
PROJECT NAME: ADDRESS: FIELD GEOLOGIST: DRILLING COMPANY:

DRILLING METHOD:

BORING DIAMETER:

	IOIAL	DFL	IH:
1			

DEPTH	SAMPLE RECOVERY	BLOW	[mdd]	BORING CONSTR.	roe riihoroeic	USCS	LITHOLOGIC DESCRIPTION Description, Grain Size, Sorting, Color, Moisture, Mechanical Properties				
						sw	Gravel, coarse, medium dense, gray, slightly moist; no odor. No core recovery to 4 feet. Used 1" macrocore sampler.				
- - - - -10-				VIIIVIIV		CL	Silty clay, reddish brown, firm to stiff, moist; no odor.				
						CL	No odor.				
-15- 	=				1 1 1 2 2 2 3 1 1 2 1 1 1 1 1 1 1 1 1 1	SW	Sand, medium to coarse grained, dark gray, dense, wet; strong diesel odor.				
 20- 			4				No odor.				
 - 25			157	Groun	f boring at 20 dwater enco converted i	ountere					
-30 							,				
 -35- 											

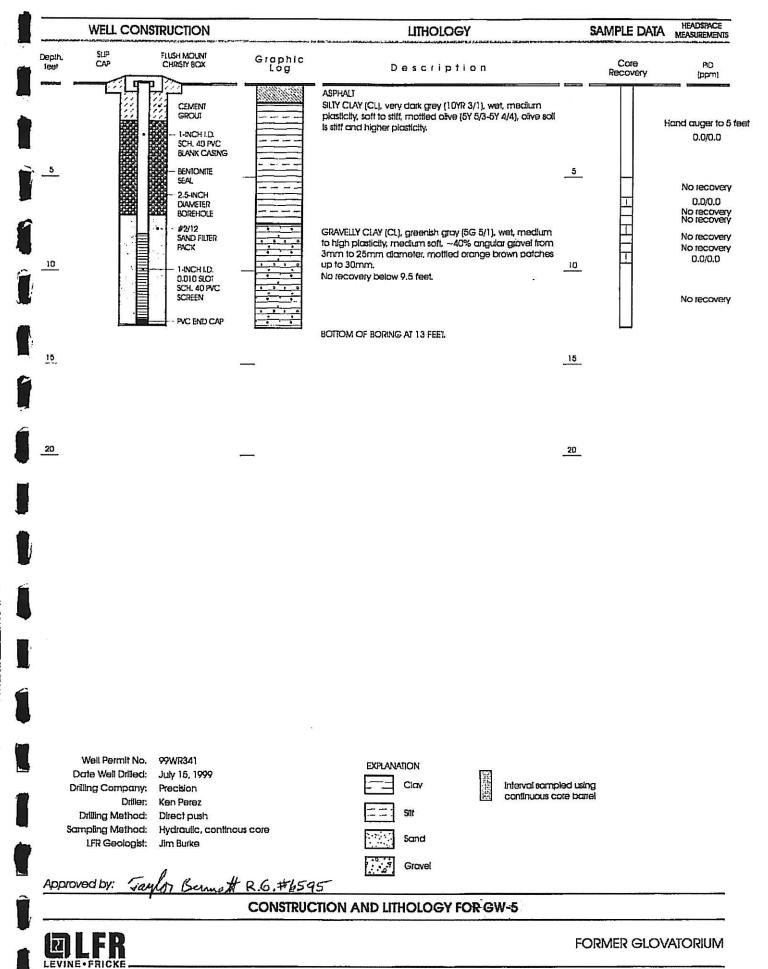


EXPLANATION Well Permit No. W00-446 Date Well Drilled: July 28, 2000 Clay interval sampled using Drilling Company: Gregg Drilling continuous core barrel Driller: Sät Soil sample collected for analysis Drill Rig: Marl M5-T (Rhino) Hollow Stem Auger Sampling Method: Hydraulic, confinous care Sand LFR Geologist: Dan Foster Grave!

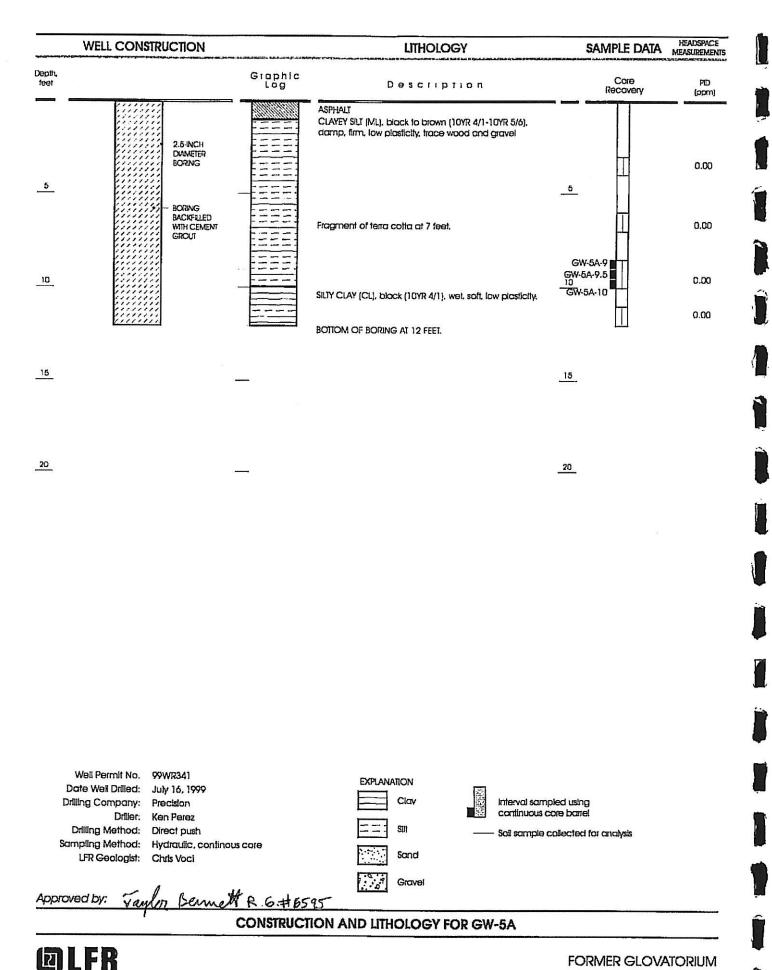
Approved by: july 12 = 52-63

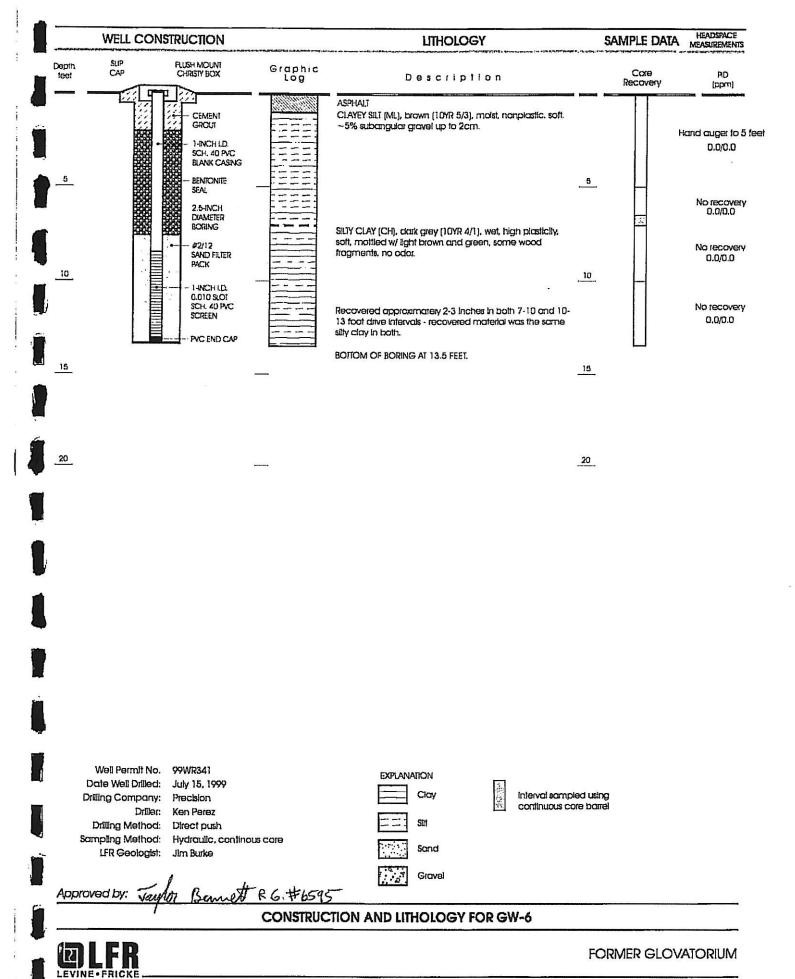
CONSTRUCTION AND LITHOLOGY FOR LFR-4



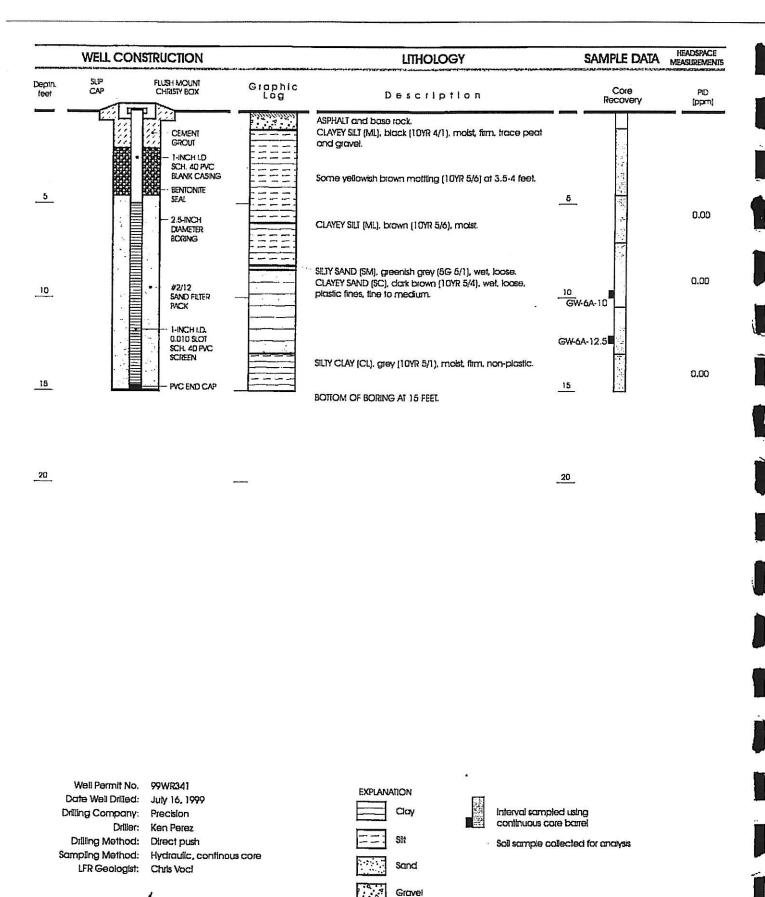


6895L104.CDR 030600





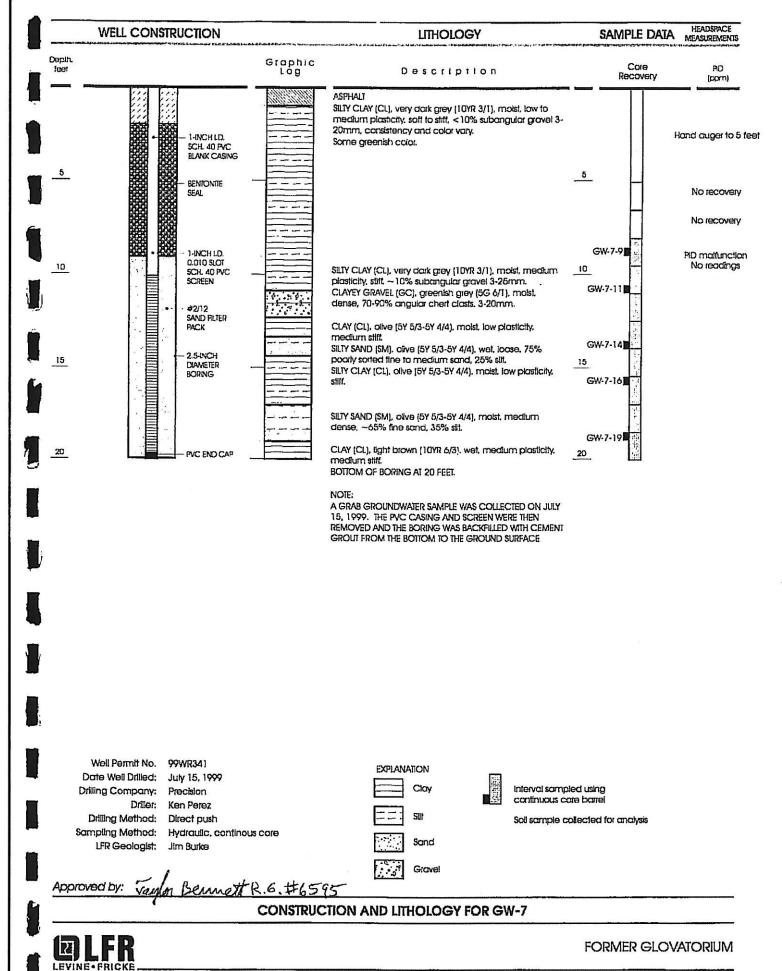
6895L106.CDR 030600



Approved by: Taylor Bernett R.G. #6595

CONSTRUCTION AND LITHOLOGY FOR GW-6A





6895L108.CDR 030600

Table 1

Construction Data for Temporary Sampling Points and Monitoring Wells

Former Glovatorium

3815 Broadway, Oakland, California

Location	Date Installed	Ground Surface Elevation (ft msl)	Top of Casing Elevation (ft msl)		Screened Interval	Screened Interval	Notes			
				(ft bgs)	Depth (ft bgs)	Elevation (ft msl)				
Temporary sampling points installed by GeoSolv, LLC:										
B-2	19-Aug-97	82.20	82.09	21	5 to 21	77.2 to 61.2				
B-3	19-Aug-97	82.60	82.57	18	5 to 18	77.6 to 64.6	(1)			
B-7	20-Aug-97	77.33	76.96	17.5	5 to 17.5 -	72.3 to 59.8	2 40			
B-8	20-Aug-97	82.06	81.82	24	9 to 24	73.1 to 58.1				
B-9	21-Aug-97	77.57	77.37	19.5	4.5 to 19.5	73.1 to 58.1				
B-10	21-Aug-97	81.65	81.50	19	4 to 19	77.7 to 62.7				
B-13	22-Aug-97	85.12	84.58	20	5 to 20	80.1 to 65.1				
Temporary sampling points installed by LFR:										
GW-1	16-Jul-99	80.24	79.94	8	3 to 8	77.2 to 72.2				
GW-2	16-Jul-99	79.44	79.14	20	10 to 20	69.4 to 59.4				
GW-3	15-Jul-99	78.48	77.92	20	10 to 20	68.5 to 58.5				
GW-4	16-Jul-99	82.55	82.37	12	7 to 12	75.6 to 70.6				
GW-5	15-Jul-99	81.31	81.01	13	8 to 13	73.3 to 68.3				
GW-6	15-Jul-99	81.91	81.65	13.5	7.5 to 13.5	74.4 to 68.4	(2)			
GW-6A	16-Jul-99	81.93	81.6 1	15	5 to 15	76.9 to 66.9				
GW-7	15-Jul-99	81.3	NS	20	10 to 20	71.3 to 61.3	(2)			
GW-8	16-Jul-99	80.28	80.10	20	10 to 20	70.3 to 60.3	(2)			
Groundwate	er Monitori	ng Wells Installed	by Tosco:							
MW-8	unknown	NS	87.44	unknown	unknown	unknown				
MW-9	unknown	NS	86.56	unknown	unknown	unknown				
MW-11	unknown	NS	84.13	unknown	unknown	unknown				
Groundwater Monitoring Wells Installed by LFR:										
LFR-1	28-Jul-00	NS	79.97	19	9 to 19					
LFR-2	27-Jul-00	NS	81.89	19	9 to 19					
LFR-3	27-Jul-00	NS	77.96	22	12 to 22					
LFR-4	28-Jul-00	NS	81.65	19	9 to 19					

Notes

(1) Top of casing surveyed on south side on January 21, 2000, because the casing was broken.

ft msl = feet above mean sea level

ft bgs = feet below ground surface

NS = Not surveyed.

⁽²⁾ GW-7 was abandoned on July 15, 1999, in accordance with LFR's work plan dated May 6, 1999, and GW-6 and GW-8 were abandoned on July 26, 2000, in accordance with LFR's work plan dated June 14, 2000.

Appendix B Field and Laboratory Procedures

Direct Push (GEOPROBE) Drilling

Utility Locating

Prior to drilling, boring locations are marked with white paint or other discernible marking and cleared for underground utilities through Underground Service Alert (USA). In addition, the first five feet of each borehole are air-knifed, or carefully advanced with a hand auger if shallow soil samples are necessary, to help evaluate the borehole location for underground structures or utilities.

Borehole Advancement

Pre-cleaned push rods (typically one to two inches in diameter) are advanced using a hydraulic push type rig for the purpose of collecting samples and evaluating subsurface conditions. The drill rod serves as a soil sampler, and an acetate liner is inserted into the annulus of the drill rod prior to advancement. Once the sample is collected, the rods and sampler are retracted and the sample tubes are removed from the sampler head. The sampler head is then cleaned, filled with clean sample tubes, inserted into the borehole and advanced to the next sampling point where the sample collection process is repeated.

Soil Sample Collection

The undisturbed soil samples intended for laboratory analysis are cut away from the acetate sample liner using a hacksaw, or equivalent tool, in sections approximately 6 inches in length. The 6 inch samples are lined at each end with Teflon® sheets and capped with plastic caps. Labels documenting job number, borehole identification, collection date, and depth are affixed to each sample. The samples are then placed into an ice-filled cooler for delivery under chain-of-custody to a laboratory certified by the State of California to perform the specified tests. The remaining collected soil that has not been selected for laboratory analysis is logged using the United Soil Classification System (USCS) under the direction of a State Registered Professional Geologist, and is field screened for organic vapors using a photo ionization detector (PI D), or an equivalent tool. Soil cuttings generated are stored in Department of Transportation (DOT) approved 55-gallon steel drums, or an equivalent storage container.

Grab Groundwater Sample Collection

Once the desired groundwater sampling depth has been reached, a Hydropunch tip is affixed to the head of the sampling rods. The Hydropunch tip is advanced between approximately 6 inches to one foot within the desired groundwater sampling zone (effort is made to emplace the Hydropunch screen across the center of the water table), and retracted to expose the Hydropunch screen. Grab groundwater samples are collected by lowering a pre-cleaned, single-sample polypropylene, disposable bailer down the annulus of the sampler rod. The groundwater sample is discharged from the bailer to the sample container through a bottom emptying flow control valve to minimize volatilization.

Alternatively, groundwater samples are collected by lowering a disposable bailer through the sampler rod or into the borehole.

Collected water samples are discharged directly into laboratory provided, pre-cleaned, vials or containers and sealed with Teflon-lined septum, screw-on lids. Labels documenting sample number, well identification, collection date, and type of preservative (if applicable. i.e. HCI for TPPH, BTEX, and fuel oxygenates) are affixed to each sample. The samples are then placed into an ice-filled cooler for delivery under chain-of-custody to a laboratory certified by the State of California to perform the specified tests.

Borehole Completion

Upon completion of drilling and sampling, the rods are retracted. Neat cement grout, mixed at a ratio of 6 gallons of water per 94 pounds of Portland cement, is introduced, *via* a tremmie pipe, and pumped to displace standing water in the borehole. Displaced groundwater is collected at the surface into DOT approved 55-gallon steel drums, or an equivalent storage container. In areas where the borehole penetrates asphalt or concrete, the borehole is capped with an equivalent thickness of asphalt or concrete patch to match finished grade.

Organic Vapor Procedures

Soil samples are collected for analysis in the field for ionizable organic compounds using a PID with a 10.2 eV lamp. The test procedure *involves* measuring approximately 30 grams from an undisturbed soil sample, placing this subsample in a Ziploc--type bag or in a clean glass jar, and sealing the jar with aluminum foil secured under a ring-type threaded lid. The container is warmed for approximately 20 minutes (in the sun); then the head-space within the container is tested for total organic *vapor*, measured in parts per million as benzene (ppm; volume/volume). The instrument is calibrated prior to drilling. The results of the field-testing are noted on the boring logs. PID readings are useful for indicating relative levels of contamination, but cannot be used to evaluate petroleum hydrocarbon levels with the confidence of laboratory analyses.

Equipment Decontamination

Equipment that could potentially contact subsurface media and compromise the integrity of the samples is carefully decontaminated prior to drilling and sampling. Drill augers and other large pieces of equipment are decontaminated using high pressure hot water spray. Samplers, groundwater pumps, liners and other equipment are decontaminated in an Alconox scrub solution and double rinsed in clean tap water rinse followed by a final distilled water rinse.

The rinsate and other wastewater are contained in 55-gallon DOT-approved drums, labeled (to identify the contents, generation date and project) and stored on-site pending waste profiling and disposal.

Soil Cuttings and Rinsate/Purge Water

Soil cuttings and rinsate/purge water generated during drilling and sampling are stored onsite in DOT-approved 55-gallon steel drums pending characterization. A label is affixed to the drums indicating the contents of the drum, suspected contaminants, date of generation,

Revised Workplan for Additional Soil and Groundwater Investigation

and the boring number by a licensed waste treatment/recycling.	from which disposal	the waste contractor	is gener under	ated. The manifest	drums a to an	re removed f appropriate	rom the facility	site for
Revised Workplan for A	dditional So	oil and Gro	undwate	r Investiga	tion			