

90 DEC -3 PM 1:33

Please Refer To: AEMC Job No. 82580

30 November 1990

Mr. Paul Smith Alameda County Department of Environmental Health Hazardous Waste Division 80 Swan Way, Room 200 Oakland, California 94621

RE: UST UNAUTHORIZED RELEASE REPORT SEARS, ROEBUCK & CO., OAKLAND, CALIFORNIA

Dear Mr. Smith:

Please find enclosed the Underground Storage Tank Unauthorized Release Report for the Sears Automotive Center at 2633 Telegraph Avenue, Oakland, California. SENSIERED GEOLOGIO

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JAMES F. FRUMM

No. 4207

Thank you for your patience.

Sincerely,

James F. Frumm, R.G., R.E.A.

Regional Manager **Engineering Division**

PKW/scg 12src-11(pw-3)

Enclosures

UNDERGROUND STORAGE TANK CLOSURE, ASSESSMENT, AND REMEDIATION REPORTS

SEARS, ROEBUCK AND CO.
OAKLAND, CALIFORNIA

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OCTOBER 12, 1990

SOIL CONTAMINATION REPORT

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MOTOR OIL TANK AREA JANUARY 23, 1991

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PRELIMINARY REPORT AND

CONTAMINATION ASSESSMENT WORKPLAN

USED OIL TANK AREA

JANUARY 4, 1991



Please Refer to: AEMC Job No. 50109

12 October 1990

Ms. Bernadine Palka Sears, Roebuck and Co. Sears Tower Department 731, BSC 39-34 Chicago, Illinois 60684

RE: REPORT OF UNDERGROUND STORAGE TANK REMOVAL SEARS, ROEBUCK AND CO., OAKLAND, CALIFORNIA

Dear Ms. Palka:

American Environmental Management Corporation (AEMC) was retained by Sears, Roebuck and Co. to excavate, remove and dispose of the existing underground storage tanks (UST). In addition, AEMC obtained and chemically analyzed soil samples as part of the UST closure activities at 2633 Telegraph Avenue, Oakland, California (Figure 1). All work onsite was completed under the regulatory guidance of Mr. Paul Smith, Alameda County Department of Environmental Management (ACDEM) and under the direct supervision of a State of California Registered Geologist.

From 17 through 20 September 1990, seven underground storage tanks were excavated and removed from the site. Six tanks containing motor oil were removed from the eastern side of the automotive repair bay (five 1,000-gallon tanks and one 2,000-gallon tank). One 1,000-gallon tank containing waste oil was removed from the western side of the repair bay (Figure 2). Dry ice was introduced into the tanks to reduce the explosive hazard under the supervision of the Oakland Fire Department. All of the USTs had been bound by metal strapping to concrete slabs. These bindings were cut and the USTs were then pulled from the excavation using a backhoe. The tanks were transported as hazardous waste to Erickson Inc. in Richmond, California under Hazardous Waste Manifest No. 88441125. None of the concrete slabs were removed at this time.

During the removal process several tanks were noted to have poor integrity and several holes. Tank 2 (2,000-gallon) had a hole in its northwestern side and soil to the north of the tank was stained. Tank 6 had a cracked seam in its southwestern weld and the soil to the north and south of the tank was noticeably stained. Tank 7 (1,000-gallon waste oil) had two holes in the bottom of its southern side and many corrosion pin holes. Soil was extensively stained throughout this excavation.

The excavated soil was stockpiled into three locations. Two stockpiles were created during the motor oil tank excavation. Stockpile 1, located on the southeastern end of the excavation, contains approximately 200 cubic yards of soil and backfill from around Tanks 3, 4, 5, and 6. Stockpile 2, located on the northeastern end of the excavation, contains approximately 50 cubic yards of soil from around Tanks 1 and 2. Stockpile 3

Ms. Bernadine Palka Sears, Roebuck and Co. 12 October 1990 Page 2

contains approximately 30 cubic yards of soil from the Tank 7 excavation (Figure 3). All of the stockpiles are covered with Visqueen at the present time.

On 19 September 1990, soil samples were obtained from the base of both tank excavations (Figure 3). The samples were taken from native soil approximately 2 feet below the backfill material along the sides of the concrete slabs. The native soil is a highly plastic, heavy gray-green clay. The soil samples were gathered with a brass tube, sealed with Teflon tape, plastic end caps, and electrical tape. All samples were then labeled, refrigerated, and delivered to AEMC's State of California Certified Laboratory under chain-of-custody for analysis.

AEMC's Analytical Laboratory (State Certification No. 210) tested all of the excavation and stockpile samples for Benzene, Toluene, Ethylbenzene, and Xylenes (BTEX) by the EPA Method 8020 Modified, Total Petroleum Hydrocarbons as diesel (TPH-D) by EPA Method 8015 Modified, and Oil and Grease by EPA Method 9071. In addition, the waste oil tank excavation and stockpile samples were analyzed for Total Petroleum Hydrocarbons as gasoline (TPH-G) by EPA Method 8015 Modified, Volatile Organic Compound by EPA Method 8240, and the ICAP Metal Series by EPA Method 6010 using Total Threshold Limit Concentration Extraction.

The analytical results from the soil samples within both excavations indicate the presence of hydrocarbon contamination in the native soil (see Table 1 for analytical results). The highest concentrations of contamination in the motor oil tank excavation occur in SB-5/6B at 390 ppm TPH-D and Oil and Grease at 600 ppm. Although, apparent contamination was visually noted during the tank removal in the Tank 2 location, analytical results showed only Oil and Grease concentrations at 80 ppm.

Contamination in the waste oil tank excavation occurred in both samples. SB-7A contained 2,800 ppm TPH-D and 3,200 ppm Oil and Grease. SB-7B showed concentrations of TPH-D at 1,500 ppm and Oil and Grease at 2,100 ppm (see Table 2 for analytical results). Appendix A contains the complete Laboratory Results and chains-of-custody).

During the removal operations, neither AEMC's geologist nor Mr. Smith, ACDEM, observed any groundwater or interflow present in the excavations. However, groundwater in the area is believed to be less than 25 feet below ground surface.

Using the Department of Health Services Leaking Underground Fuel Tank Manual and the Regional Water Quality Control Board's Tri-Regional Recommendations Document (updated August 1990) as guides, AEMC will submit a Soil Contamination Workplan and Remediation Proposal for the motor oil tanks site to ACDEM. In addition, a separate Preliminary Report will be submitted for the waste oil tank site. This report will propose a method to assess the vertical and lateral extent of soil and groundwater contamination.

Ms. Bernadine Palka Sears, Roebuck and Co. 12 October 1990 Page 3

If you have any questions or comments regarding the site, please do not hesitate to call Mr. Phil Walsack or Mr. Michael Holley at (916) 36 8872.

Sincerely,

JAMES F. FRUMM

No. 4207

OF CALIFORNIA

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James F. Frumm, R.G., R.E.A. Regional Manager

Engineering Division

PKW/scg r1src-10(pw-3)

cc: Mr. Paul Smith, Alameda County Department of Environmental Management

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TABLE 1

Analytical Results of Soil Samples Sears, Roebuck and Co. Oakland, California

Motor Oil Tank Area

Sample ID	ID Depth (feet below ground surface)		Oil & Grease (ppm)
Excavation			Martin 1
SB-1A SB-1B SB-1/2 A SB-2/3 A SB-3/4 A SB-4/5 A SB 5/6 A SB-5/6 B SB-6A	10 10 10 10 10 10 11 11 10 9	20 20 20 20 20 20 20 20 20 20 20 20 20 2	ND N
SP-1-1 SP-1-2 SP-1-3 SP-1-4 SP-1-5		140 120 170 52 77	ND 260 280 240 100
SP-2-1 SP-2-2		39 87	100 200

^{*} Sample also contained Ethylbenzene @ 13 ppb and Xylenes @ 14 ppb.

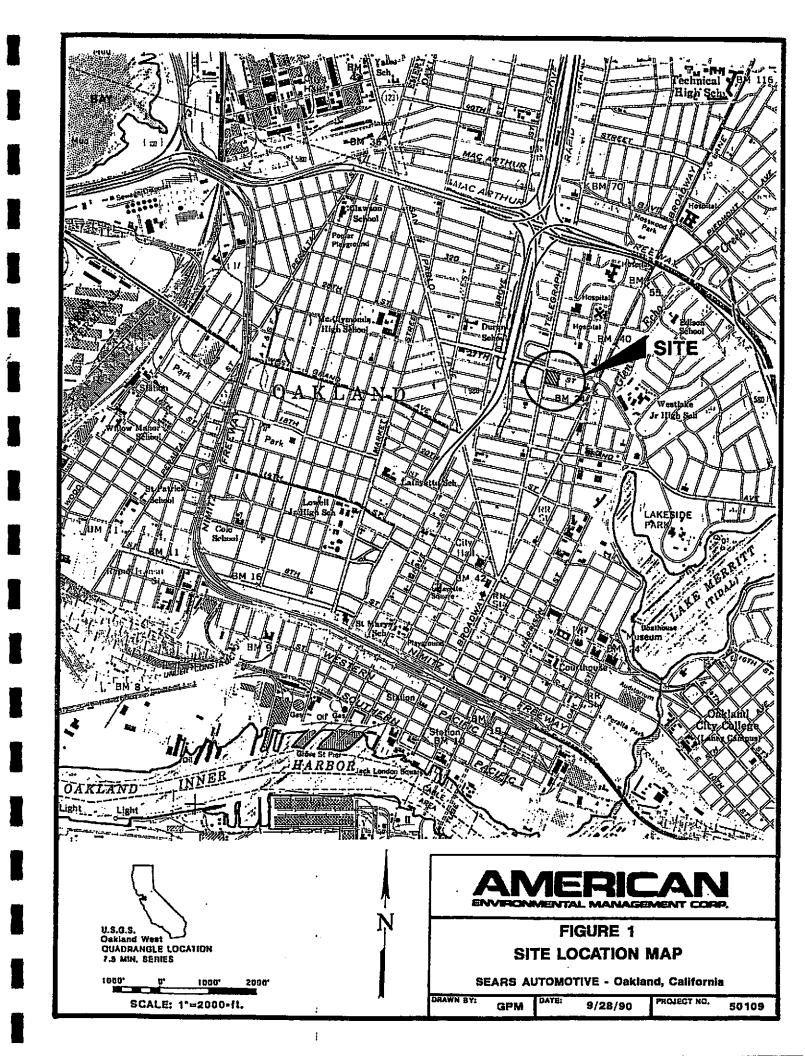
TABLE 2

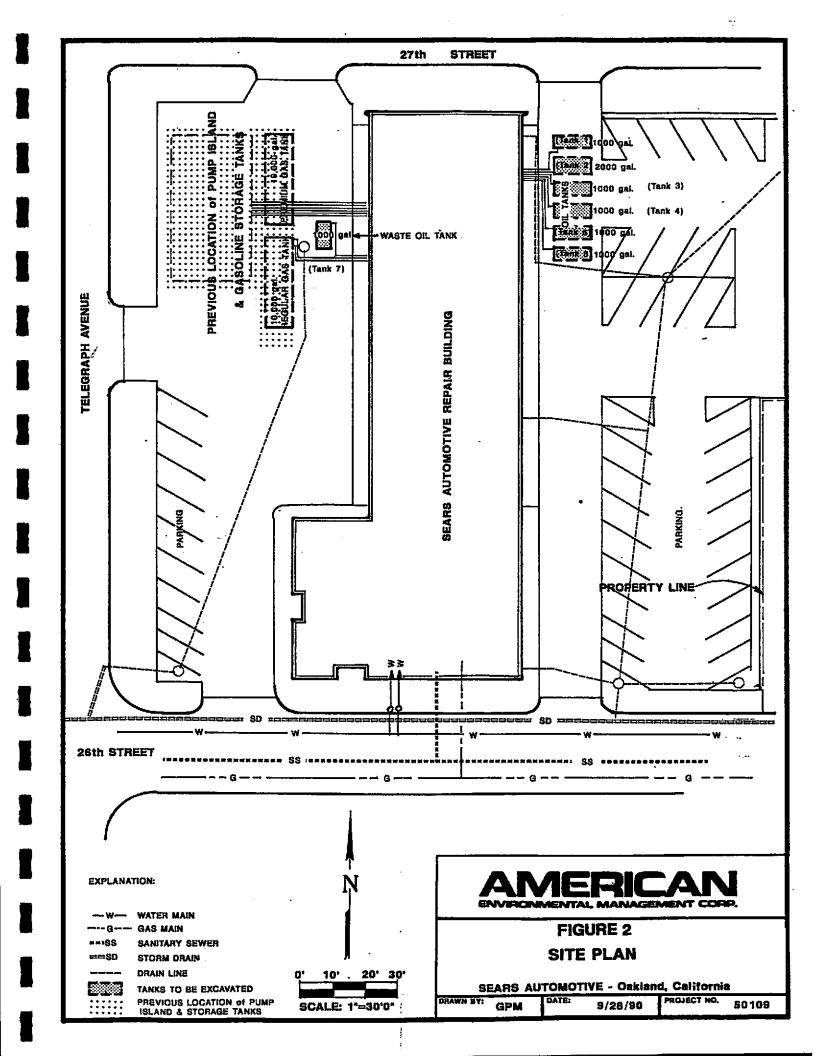
Analytical Results of Soil Samples Sears, Roebuck and Co. Oakland, California

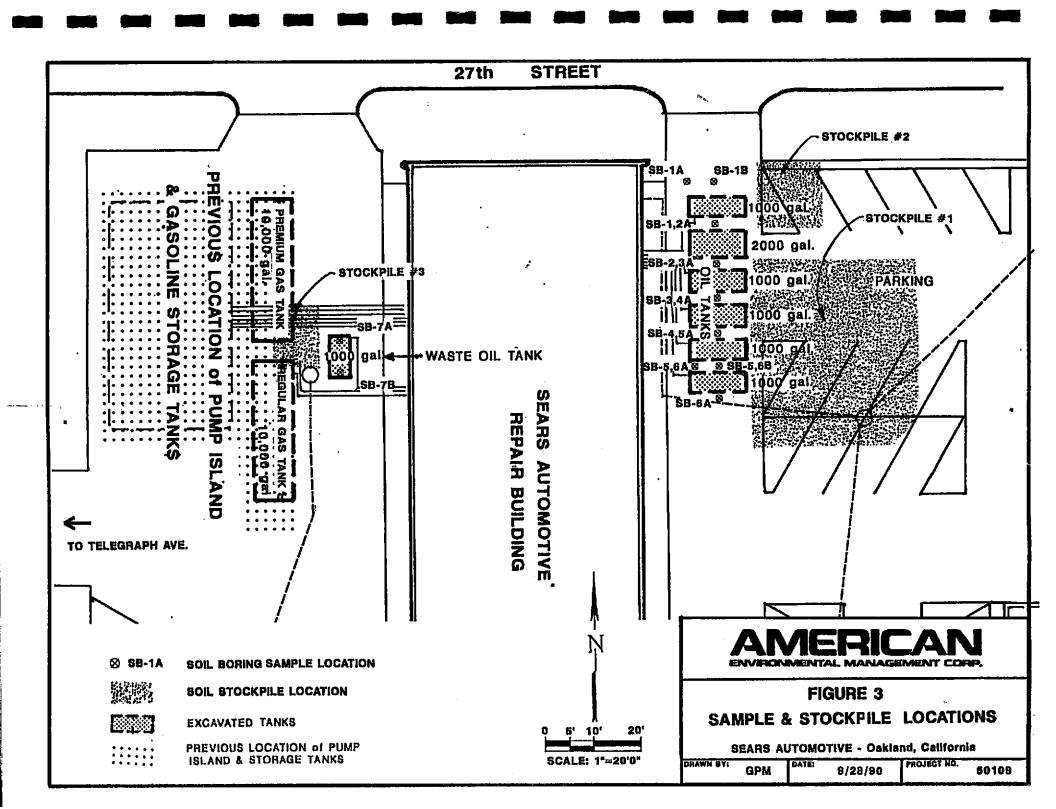
Waste Oil Tank Area

Sample ID	Depth (feet bes)	TPH-G	TPH-D (ppm)	Oil & Grease (ppm)	B (ppb)	T (ppb)	E (ppb)	X (ppb)	Cd (ppm)	Cr (ppm)	Ni (ppm)	Pb (ppm)	Zn (ppm
Excavation													
SB-7A ^a SB-7B ^b	9 9	31 31	2,800 1,500	3,200 2,100	ND 12	58 200	100 250	720 1,400	ND ND	33 28	28 24	360 190	54 64
Stockpile													
SP-3-1 ^C SP-3-2	Ξ	39 13	4,400 850	6,800 1,600	ND ND	310 9	410 23	3,000 220	ND 1	20 32	20 34	440 47	62 45

bgs		below ground	surface
	H-G H-D	Total Petroleu Total Petroleu Benzene Toluene Xylenes Ethylbenzene Cadmium Chromium Nickel Lead Zinc	am Hydrocarbons as gasoline am Hydrocarbons as diesel
a	Sampl	e also contained:	Tetrachloroethene @ 82 ppb Trichloroethene @ 17 ppb
b	Sampl	e also contained:	Acetone @ 140 ppb Tetrachloroethene @ 7 ppb Trichloroethane @ 19 ppb
C	Sampl	e also contained:	Tetrachloroethene @ 52 ppb









AEMC White Rock 11855 White Rock Road Rancho Cordova, CA 95742

09/28/90

Attn: T. Anckner

Project No.: 50109

Project: Sears Roebuck Co.
AEMC Lab Reference No.: L5351 P.
Date Samples Received: 09/20/90
No. Samples Received: 38 Soil samples

These samples were received by AEMC in a chilled state, intact, and accompanied by chain-of-custody documentation.

The above referenced samples were analyzed as follows:

No. of Samples	Analysis
4	Cadmium by ICAP
4	Chromium (Total) by ICAP
4	Lead by ICAP
4	Nickel by ICAP
4	Zinc by ICAP
4	TPH Gasoline only
20	TPH Diesel only
16	Purgeable Aromatics by EPA 8240
20	Oil and Grease - gravimetric

The "TPH as Diesel" data presented here represent measurement of only those hydrocarbons contained within the molecular weight range (C10-C25) which includes the major diesel components.

Analytical results are attached to this letter. Please call if we can provide additional assistance.

Sincerely,

CLIENT: AEMC 11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90
Date Received: 9/20/90
Date Extracted:9/20/90
Date Analyzed: 9/20/90
Date Reported: 9/24/90
Client Sample I.D.: SB-7A

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351-9 Batch No.: 51214 Matrix: Soil

Element	Results (mg/kg)	Rpt. Limit (mg/kg)	Method	
Cd (Cadmium)	ND	1.0	6010	
Cr (Chromium - total)	33	5.0	6010	
Ni (Nickel)	28	5.0	6010	
Pb (Lead)	360	5.0	6010	
Zn (Zinc)	54	5.0	6010	

Rpt. Limit - Reporting Limit
ND - Not Detected at or above indicated Reporting Limit.

ENVIRONMENTAL LABORATORIES CORP.

ANALYSIS REPORT: Metals, TTLC, EPA Method 6010

CLIENT: AEMC 11855 White Rock Road Cordova CA 9:

Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90 Date Received: 9/20/90 Date Extracted:9/20/90 Date Analyzed: 9/20/90 Date Reported: 9/24/90 Client Sample I.D.: SB-7B

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351-10-Batch No.: 51214
Matrix: Soil

Element	Results (mg/kg)	Rpt. Limit (mg/kg)	Method	
Cd (Cadmium)	ND	1.0	6010	_
Cr (Chromium - total)	28	5.0	6010	
Ni (Nickel)	24	5.0	6010	
Pb (Lead)	190	5.0	6010	
Zn (Zinc)	64	5.0	6010	

Rpt. Limit - Reporting Limit
ND - Not Detected at or above indicated Reporting Limit.

CLIENT: AEMC 11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90
Date Received: 9/20/90
Date Extracted: 9/20/90
Date Analyzed: 9/20/90
Date Reported: 9/24/90
Client Sample I.D.: SP-3-1 Composite

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351-34 Batch No.: 51214 Matrix: Soil

Element	Results (mg/kg)	Rpt. Limit (mg/kg)	Method	
Cd (Cadmium)	ND	1.0	6010	
Cr (Chromium - total)	20	5.0	6010	
Ni (Nickel)	20	5.0	6010	
Pb (Lead)	440	5.0	6010	
Zn (Zinc)	62	5.0	6010	

Rpt. Limit = Reporting Limit
ND = Not Detected at or above indicated Reporting Limit.

CLIENT: AEMC 11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90
Date Received: 9/20/90
Date Extracted: 9/20/90
Date Analyzed: 9/20/90
Date Reported: 9/24/90
Client Sample I.D.: SP-3-2 Composite

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351-37 Batch No.: 51214 Matrix: Soil

Element	Results (mg/kg)	Rpt. Limit (mg/kg)	Method	
Cd (Cadmium)	1.0	1.0	6010	
Cr (Chromium - total)	32	5.0	6010	
Ni (Nickel)	34	5.0	6010	
Pb (Lead)	47	5.0	6010	
Zn (Zinc)	45	5.0	6010	

Rpt. Limit = Reporting Limit
ND = Not Detected at or above indicated Reporting Limit.

CLIENT: AEMC 11855 White Rock Road

Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90
Date Received: 9/20/90
Date Extracted:9/20/90
Date Analyzed: 9/20/90
Date Reported: 9/24/90
Client Sample I.D.: Method Blank

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351-MB Batch No.: 51214 Matrix: Soil

Element	Results (mg/kg)	Rpt. Limit (mg/kg)	Method	
Cd (Cadmium)	ND	1.0	6010	
Cr (Chromium - total)	ND	5.0	6010	
Ni (Nickel)	ND	5.0	6010	
Pb (Lead)	ND	5.0	6010	
Zn (Zinc)	ND	5.0	6010	

Rpt. Limit - Reporting Limit ND = Not Detected at or above indicated Reporting Limit.

CLIENT: AEMC 11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90 Date Received: 9/20/90 Date Extracted:9/20/90 Date Analyzed: 9/20/90 Date Reported: 9/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351 Batch No.: 51214 Matrix: Soil

ELEMENT	Spike Conc. (mg/kg)	MS %Rec	MSD %Rec	Duplicate RPD	
Cd (Cadmium)	25	97%	90%	7%,	
Cr (Chromium - total)	25	86%	84%	2%	
Ni (Nickel)	25	80%	75%	6%	
Pb (Lead)	250	98%	104%	6%	•
Zn (Zinc)	25	78%	74%	5%	

MS

Matrix Spike
Matrix Spike Duplicate
Percent Recovery
Relative Percent Difference MSD

% Rec RPD

CLIENT: AEMC 11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90 Date Received: 9/20/90 Date Extracted:9/20/90 Date Analyzed: 9/20/90 Date Reported: 9/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351 Batch No.: 51214

Element	LCS Conc. LCS (mg/L) %Rec		
Cd (Cadmium)	0.5	112%	, .
Cr (Chromium - total)	0.5	99%	*
Ni (Nickel)	0.5	92%	
Pb (Lead)	0.5	88%	
Zn (Zinc)	0.5	92%	

LCS = Laboratory Control Standards % Rec = Percent Recovery LCS



CLIENT: AEMC

11855 White Rock Road

Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 09/19/90 Date Received: 09/20/90 Date Extracted:09/20/90 Date Analyzed: 09/20/90 Date Reported: 09/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351

Matrix: Soil

Sample Client	I.D. AEMC	Batch #		TPH as Gasoline (mg/kg)	•	
SB-7A	L5351-9	6265	. چۀ'	31		
SB-7B	L5351-10	6265		31		
SP-3-1 Composite	L5351-34	6265		39		
SP-3-2 Composite	L5351-37	6265		13		
Method Blank	L5351-MB	6265		ND		
REPORTING LIM	IT*			1.0		•

*Unless otherwise indicated in parentheses

ND - Not Detected at or above indicated Reporting Limit.



CLIENT: AEMC 11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 09/19/90 Date Received: 09/20/90 Date Extracted:09/20/90 Date Analyzed: 09/20/90 Date Reported: 09/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351 Batch No.: 6265 Matrix: Soil

Analyte	Spike Conc. (mg/kg)	MBS %Rec	MBSD %Rec	Duplicate RPD	
Gasoline	4.0	105%	108%	3%	

MBS

Method Blank Spike
Method Blank Spike Duplicate
Percent Recovery
Relative Percent Difference MBSD -% Rec RPD



CLIENT: AEMC 11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 09/19/90 Date Received: 09/20/90 Date Extracted:09/20/90 Date Analyzed: 09/20/90 Date Reported: 09/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351 Batch No.: 6265

Analyte	LCS Conc. (mg/L)	LCS %Rec	
Gasoline	0.8	112%	

- Laboratory Control Standards LCS % REC - Percent Recovery



CLIENT: AEMC

11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 09/19/90 Date Received: 09/20/90 Date Extracted:09/20/90 Date Analyzed: 09/20/90 Date Reported: 09/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351

Matrix: Soil

Sample Client	I.D. AEMC	Batch #	TPH as Diesel (mg/kg)		•		
SB-1A	L5351-1	6268	ND			~	
SB-1/2A	L5351-2	6268	ND		· ·		
SB-2/3A	L5351-3	6268	ND				
SB-3/4A	L5351-4	6268	ND				
SB-4/5A	L5351-5	6268	ND	r			
SB-5/6A	L5351-6	6268	ND				
SB-5/6	L5351-7	6268	390	•			
SB-6A	L5351-8	6268	ND				
SB-7A	L5351-9	6268	2,800				•
\$B-7B	L5351-10	6268	1,500				
SP-1-1 Comp.	L5351-13	6267	140				
SP-1-2 Comp.	L5341-16	6267	120				
SP-1-3 Comp.	L5341-19	6267	170				
SP-1-4 Comp.	L5341-22	6267	52				
SP-1-5 Comp.	L5351-25	6267	77				
SP-2-1 Comp.	L5351-28	6267	39				
SP-2-2 Comp.	L5351-31	6267	87				
SP-3-1 Comp.	L5351-34	6267	4,400	•			
SP-3-2 Comp.	L5351-37	6267	850				
SB-1B	L5351-38	6267	ND		<u>*</u>		
Method Blank	L5351-MB	6267	ND				
Method Blank	L5351-MB	6268	ND				
	i e						

REPORTING LIMIT*

10

*Unless otherwise indicated in parentheses

ND - Not Detected at or above indicated Reporting Limit.



CLIENT: AEMC 11855 White Rock Road

Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 09/19/90 Date Received: 09/20/90 Date Extracted:09/20/90 Date Analyzed: 09/20/90 Date Reported: 09/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351 Batch No.: 6267 Matrix: Soil

Analyte	Spike Conc. (mg/kg)	MBS %Rec	MBSD %Rec	Duplicate RPD	
Diesel	100	111%	104%	7%	

MBS

Method Blank Spike
Method Blank Spike Duplicate
Percent Recovery MBSD

% Rec

- Relative Percent Difference RPD



CLIENT: AEMC 11855 White Rock Road

Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 09/19/90 Date Received: 09/20/90 Date Extracted:09/20/90 Date Analyzed: 09/20/90 Date Reported: 09/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351 Batch No.: 6268 Matrix: Soil

Analyte	Spike Conc. (mg/kg)	MBS %Rec	MBSD %Rec	Duplicate RPD	
Diesel	100	114%	111%	3%	

MBS

- Method Blank Spike - Method Blank Spike Duplicate - Percent Recovery MBSD % Rec RPD

- Relative Percent Difference



Diesel

CLIENT: AEMC 11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 09/19/90 Date Received: 09/20/90 Date Extracted:09/20/90 Date Analyzed: 09/20/90 Date Reported: 09/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351

LCS LCS Conc. &Rec Analyte (mg/L) 1,000 105%

LCS - Laboratory Control Standards % Rec - Percent Recovery

ANALYSIS REPORT: BTXE, EPA Method 8240

CLIENT: AEMC 11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90 Date Received: 9/20/90 Date Extracted:9/20/90 Date Analyzed: 9/20/90 Date Reported: 9/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351

Matrix: Soil

Sample I.	D. AEMC	Batch #	Benzene (ug/kg)	Toluene (ug/kg)	Ethyl- benzene (ug/kg)	Xylenes, Total (ug/kg)
SB-1A	L5351-1	6263	,ND	ND	ND	ND
SB-1/2A	L5351-2	6263	ND	ND	ND	ND
SB-2/3A	L5351-3	6263	ND	ND	ND	ND
SB-3/4A	L5351-4	6263	ND	ND	ND	ND
SB-4/5A	L5351-5	6263	ND	ND	ND	ND
SB-5/6A	L5351-6	6263	ND	ND	, ND.	ND
SB-5/6B	L5351-7	6263	, ND	ND	13	14
SB-6A	L5351-8	6263	ND	ND ·	ND	ND
Method Blank	L5351-MB	6263	ND	ND	ND	ND
SP-1-1 Composite	L5351-13	6264	ND	ND	ND	ND
SP-1-2 Composite	L5351-16	6264	ND	ND	ND	ND
SP-1-3 Composite	L5351-19	6264	ND	ND	ND	ND
SP-1-4 Composite	L5351-22	6264	ND	ND	ND	ND
SP-1-5 Composite	L5351-25	6264	ND	ND	ND	ND
SP-2-1 Composite	L5351-28	6264	ND	ND	ND	ND
SP-2-2 Composite	L5351-31	6264	ND	ND	ND	ND
SB-1B	* L5351-38	6264	ND	ND	ND .	ND
Method Blank	L5351-MB	6264	ND	ND	ND	ND
REPORTING LIMI	T*		5	5	5	10

*Unless otherwise indicated in parentheses

ND - Not Detected at or above indicated Reporting Limit.



ANALYSIS REPORT: BTXE, EPA Method 8240

CLIENT: AEMC

11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90 Date Received: 9/20/90 Date Extracted:9/20/90 Date Analyzed: 9/20/90 Date Reported: 9/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351

Matrix: Soil

Sample I.D. Client	AEMC	Toluene-d8 Conc. (ug/kg)	Surrogate Recovery % Recovery	
SB-1A	L5351-1	100	93%	
SB-1/2A	L5351-2	100	101%	
SB-2/3A	L5351-3	100	103%	
SB-3/4A	L5351-4	100	101%	
SB-4/5A	L5351-5	100	102%	
SB-5/6A	L5351-6	100	101%	
SB-5/6B	L5351-7	100	92%	
SB-6A	L5351-8	100.	101%	
Method Blank Batch 6263	L5351-MB	100	104%	
SP-1-1 Composite	L5351-13	100	85%	
SP-1-2 Composite	L5351-16	100	72%	
SP-1-3 Composite	L5351-19	100	71%	
SP-1-4 Composite	L5351-22	100	78%	
SP-1-5 Composite	L5351-25	100	102%	
SP-2-1 Composite	L5351-28	100	99%	
SP-2-2 Composite	L5351-31	100	92%	
SB-1B	L5351-38	100	103%	
Method Blank Batch 6264	L5351-MB	100	103%	

ENTAL LABORATORIES CORP.

ANALYSIS REPORT: BTXE, EPA Method 8240

CLIENT: AEMC 11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90 Date Received: 9/20/90 Date Extracted:9/20/90 Date Analyzed: 9/20/90 Date Reported: 9/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351 Batch No.: 6263 Matrix: Soil

Surrogate	Spike Conc. (ug/kg)	MS %Rec	MSD %Rec	
Toluene-d8	100	105%	102%	
Analyte	Spike Conc. (ug/kg)	MS %Rec	MSD %Rec	Duplicate RPD
Benzene	50	115%	114%	1%
Toluene	50	110%	110%	0%

MS

Matrix Spike
Matrix Spike Duplicate
Percent Recovery
Relative Percent Difference MSD

% REC

RPD



ANALYSIS REPORT: BTXE, EPA Method 8240

CLIENT: AEMC 11855 White Rock Road

Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90 Date Received: 9/20/90 Date Extracted:9/20/90 Date Analyzed: 9/20/90 Date Reported: 9/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351 Batch No.: 6264 Matrix: Soil

Surrogate	Spike Conc. (ug/kg)	MS %Rec	MSD %Rec		
Toluene-d8	100	102%	101%		
Analyte	Spike Conc. (ug/kg)	MS %Rec	MSD %Rec	Duplicate RPD	
Benzene	50	113%	113%	0%	
Toluene	50	106%	103%	3%	

MS - Matrix Spike
MSD - Matrix Spike Duplicate
% REC - Percent Recovery
RPD - Relative Percent Recovery

- Relative Percent Difference



ANALYSIS REPORT: BTXE, EPA Method 8240

CLIENT: AEMC 11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90 Date Received: 9/20/90 Date Extracted: 9/20/90 Date Analyzed: 9/20/90 Date Reported: 9/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351 Batch No.: 6263, 6264

Analyte	LCS Conc. (ug/L)	LCS %Rec		
Benzene	50	103%		
Toluene	50	98%	,	

LCS - Laboratory Control Standards % REC - Percent Recovery



CLIENT: AEMC 11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90
Date Received: 9/20/90
Date Extracted:9/20/90
Date Analyzed: 9/20/90
Date Reported: 9/24/90
Client Sample I.D.: SB-7A

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351-9 Batch No.: 6264 Matrix: Soil

Surrogate	CAS #	Spike Conc. (ug/kg)	Recovery (percent)
1,2-Dichloroethane-d4 Toluene-d8 p-Bromofluorobenzene	d107-06-2 d108-88-3 460-00-4	100 100 100	110% 91% 110%
Analyte	CAS #	Concentration (ug/kg)	Rpt. Limit (ug/kg)
Acetone Benzene Bromodichloromethane Bromomethane 2-Butanone Carbon disulfide Carbon tetrachloride Chlorobenzene Chloroethane 2-Chloroethyl vinyl ether Chloroform Chloromethane Dibromochloromethane Dibromomethane 1,2-Dichlorobenzene 1,3-Dichlorobenzene 1,4-Dichlorobenzene 1,1-Dichloroethane 1,2-Dichloroethane 1,2-Dichloroethane 1,2-Dichloroethene 1,2-Dichloropropane cis-1,3-Dichloropropane cis-1,3-Dichloropropene trans-1,3-Dichloropropene Ethylbenzene 2-Hexanone Methylene chloride 4-Methyl-2-pentanone Styrene 1,1,2,2-Tetrachloroethane Tetrachloroethene Toluene 1,1,1-Trichloroethane Trichloroethene Trichloroethene Trichloroethene Trichloroethene Trichloroethene Trichlorofluoromethane 1,1,2-Trichlorotrifluoroethane Vinyl acetate Vinyl chloride Xylenes, total	67-64-1 71-43-4 75-27-4 75-27-4 75-27-4 75-27-4 75-27-4 75-27-4 75-27-3 75-27-3 75-31 75-31 75-31 75-34-3 124-495-1 106-46-8 75-34-3 107-34-3 107-34-3 107-34-3 107-34-3 107-34-3 107-34-3 107-34-3 107-34-3 107-34-3 107-34-3 107-34-3 107-34-3 107-34-3 107-34-3 107-34-3 107-34-3 107-34-3 108-42-5 108-42-5 108-42-5 108-33-4 108-33-2 108-33-4 108-33-4 108-33-4 108-33-4 108-33-4 108-33-20-7	ND NDD NDD NDD NDD NDD NDD NDD NDD NDD	100 55 100 105 55 105 55 105 55 55 55 55 55 55 55 55 55 55 55 55 5

RPT. LIMIT - Reporting Limit
ND - Not Detected at or above indicated Reporting Limit



CLIENT: AEMC 11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90 Date Received: 9/20/90 Date Extracted:9/20/90 Date Analyzed: 9/20/90 Date Reported: 9/24/90 Client Sample I.D.: SB-7B

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMG I.D.: L5351-10 Batch No.: 6264 Matrix: Soil

Surrogate	CAS #	Spike Conc. (ug/kg)	Recovery (percent)
1,2-Dichloroethane-d4	d107-06-2	100	110%
Toluene-d8 p-Bromofluorobenzene	d108-88-3 460-00-4	2 100 100	92% 120%
Analyte	CAS #	Concentration (ug/kg)	Rpt. Limit (ug/kg)
Acetone	67-64-1	140	100
Benzene	71-43-2	12	5 5
Bromodichloromethane	75-27-4	ND	5
Bromoform	75-25-2	ND	5
Bromomethane	74-83-9	ND	10 100
2-Butanone	<u> 78-93-3</u>	ND	100
Carbon disulfide	75-15-0	ND	Ž -
Carbon tetrachloride	56-23-5	ND	2ٍ
Chlorobenzene	108-90-7	ND	15
Chloroethane	75-00-3	ND	10
2-Chloroethyl vinyl ether	110-75-8	ND	วก็
Chloroform	67-66-3	ND	20
Chloromethane	74-87-3	ND	īŽ
Dibromochloromethane	124-48-1	ND	ب
Dibromomethane	74-95-3	ND	٦
l,2-Dichlorobenzene	95-50-1 541-73-1	ND ND	٦
l,3-Dichlorobenzene l,4-Dichlorobenzene	106-46-1	ND	. ξ
Dichlorodifluoromethane	75-71-8	ND	ากั
l.l-Dichloroethane	75-34-3	ND	ŤŠ
1,2-Dichloroethane	107-06-2	ND	ξ
1,1-Dichloroethene	75-35-4	ND	š ·
1,2-Dichloroethene, total	540-59-0	ND	• ₹
1,2-Dichloropropane	78-87-5	ND	ξ .
cis-1,3-Dichloropropene	10061-01-5	ND	5
trans-1,3-Dichloropropene	10061-02-6	ND	5
Ethylbenzene	100-41-4	250	Š .
2-Hexanone	591-78-6	ND	5 5 10 10 10 5 5 5 5 5 5 5 5 5 5 5 5 5 5
Methylene chloride	75-09-2	ND	5
4-Methyl-2-pentanone	108-10-1	ND	50
Styrene	100-42-5	ND	5
1,1,2,2-Tetrachloroethane	79-34-5	ND	5
Tetrachloroethene	127-18-4	7	5
Toluene	108-88-3	200	5
1,1,1-Trichloroethane	71-55-6	19	5 50 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5
1,1,2-Trichloroethane	79-00-5	ИĎ	5
Trichloroethene	79-01-6	ND	5
Trichlorofluoromethane	75-69-4	ИD	5
1,1,2-Trichlorotrifluoroethane	79-13-1	ND	_5
Vinyl acetate	108-05-4	ND	50
Vinyl chloride	75-01-4	ND	- 10
Xylénes, total	1330-20-7	1,400	10

RPT. LIMIT - Reporting Limit
ND - Not Detected at or above indicated Reporting Limit



CLIENT: AEMC 11855 White Rock Road

Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90
Date Received: 9/20/90
Date Extracted: 9/20/90
Date Analyzed: 9/20/90
Date Reported: 9/24/90
Client Sample I.D.: SP-3-2 Composite

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351-37 Batch No.: 6264 Matrix: Soil

Surrogate	CAS #	Spike Conc. (ug/kg)	Recovery (percent)
1,2-Dichloroethane-d4 Toluene-d8 p-Bromofluorobenzene	d107-06-2 d108-88-3 460-00-4	100 100 100	110% 89% 120%
Analyte	CAS #	Concentration (ug/kg)	Rpt. Limit (ug/kg)
Acetone Benzene Bromodichloromethane Bromoform Bromomethane 2-Butanone Carbon disulfide Carbon tetrachloride Chlorobenzene Chloroethane 2-Chloroethyl vinyl ether Chloroform Chloromethane Dibromomethane Dibromomethane 1,2-Dichlorobenzene 1,3-Dichlorobenzene 1,4-Dichlorobenzene 1,1-Dichloroethane 1,1-Dichloroethane 1,2-Dichloroethane 1,2-Dichloroethene 1,2-Dichloroethene 1,2-Dichloropropane cis-1,3-Dichloropropene trans-1,3-Dichloropropene trans-1,3-Dichloropropene Ethylbenzene 2-Hexanone Methylene chloride 4-Methyl-2-pentanone Styrene 1,1,2,2-Tetrachloroethane Toluene 1,1,1-Trichloroethane Trichloroethene Trichloroethene Trichloroethene Trichloroethene Trichloroethene Trichloroethene Trichlorofluoromethane 1,1,2-Trichlorotrifluoroethane Vinyl acetate Vinyl chloride Xylenes, total	67-64-1 71-43-2 75-27-4 75-25-2 74-83-9 78-93-0 56-23-7 75-00-3 108-90-3 108-90-3 108-90-3 108-90-3 108-90-3 108-90-3 108-90-3 108-90-3 108-90-3 108-90-3 108-90-3 108-90-1 108-95-5 106-46-4 75-36-4 75-36-4 108-90-1 109-41-4 591-78-6 109-34-5 108-90-1		100 55 100 105 55 100 105 55 105 55 55 55 55 55 55 55 55 55 55 55 55 5

RPT. LIMIT - Reporting Limit
ND - Not Detected at or above indicated Reporting Limit



CLIENT: AEMC 11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90
Date Received: 9/20/90
Date Extracted:9/20/90
Date Analyzed: 9/20/90
Date Reported: 9/24/90
Client Sample I.D.: SP-3-1 Composite

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351-34 Batch No.: 6264 Matrix: Soil

Surrogate	CAS #	Spike Conc. (ug/kg)	Recovery (percent)	
1,2-Dichloroethane-d4 Toluene-d8 p-Bromofluorobenzene	d107-06-2 d108-88-3 460-00-4	500 500 500	110% 92% 120%	
Analyte	CAS #	Concentration (ug/kg)	Rpt. Limit (ug/kg)	·····-
Chloroethale 2-Chloroethyl vinyl ether Chloroform Chloromethane Dibromochloromethane Dibromomethane 1,2-Dichlorobenzene 1,3-Dichlorobenzene 1,4-Dichlorobenzene Dichlorodifluoromethane 1,1-Dichloroethane 1,2-Dichloroethane 1,2-Dichloroethene 1,2-Dichloroethene 1,2-Dichloropropane cis-1,3-Dichloropropene	74-48-1 74-48-1 74-95-3 95-50-1 541-73-1 106-46-1 75-71-8 75-34-3 107-06-2 740-59-0 78-87-5 10061-01-5 10061-02-6 100-41-4 591-78-6 75-09-2 108-10-1 109-42-5 127-18-4 108-88-3 71-55-6 79-00-56	ND NDD NDD NDD NDD NDD NDD NDD NDD NDD	5 0255500550555555555555555555555555555	•

RPT. LIMIT - Reporting Limit

ND - Not Detected at or above indicated Reporting Limit



CLIENT: AEMC

11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90
Date Received: 9/20/90
Date Extracted:9/20/90
Date Analyzed: 9/20/90
Date Reported: 9/24/90
Client Sample I.D.: Method Blank

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351-MB Batch No.: 6264 Matrix: Soil

Surrogate	CAS #	Spike Conc. (ug/kg)	Recovery (percent)
l,2-Dichloroethane-d4 Toluene-d8 p-Bromofluorobenzene	d107-06-2 d108-88-3 460-00-4	100 100 100	95% 103% 98%
Analyte	CAS #	Concentration (ug/kg)	Rpt. Limit (ug/kg)
Acetone Benzene Bromodichloromethane Bromoform Bromomethane 2-Butanone Carbon disulfide Carbon tetrachloride Chloroethane 2-Chloroethyl vinyl ether Chloroform Chloromethane Dibromochloromethane Dibromochloromethane 1,2-Dichlorobenzene 1,3-Dichlorobenzene 1,4-Dichlorobenzene 1,1-Dichloroethane 1,2-Dichloroethane 1,2-Dichloroethane 1,2-Dichloroethene 1,2-Dichloroethene 1,2-Dichloropropane cis-1,3-Dichloropropene trans-1,3-Dichloropropene Ethylbenzene 2-Hexanone Methylene chloride 4-Methyl-2-pentanone Styrene 1,1,2,2-Tetrachloroethane Tetrachloroethene Toluene 1,1,1-Trichloroethane Trichloroethene Trichloroethene Trichloroethene Trichlorofluoromethane Trichloroethene Trichlorofluoromethane 1,2-Trichlorotrifluoroethane Vinyl acetate Vinyl chloride Xylenes, total	67-64-1 77-43-4 77-43-4 775-27-27-27-27-27-27-27-27-27-27-27-27-27-		10055500505055555555555555555555555555

RPT. LIMIT - Reporting Limit ND - Not Detected at or above indicated Reporting Limit

ANALYSIS REPORT: Purgeable Organic Analytes, EPA Method 8240

CLIENT: AEMC 11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90 Date Received: 9/20/90 Date Extracted:9/20/90 Date Analyzed: 9/20/90 Date Reported: 9/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351 Batch No.: 6264 Matrix: Soil

Surrogate	CAS #	Spike Conc. (ug/kg)	MS %Rec	MSD %Rec
1,2-Dichloroethane-d4	d107-06-2	100	99%	99%
Toluene-d8	d108-88-3	100	102%	101%
p-Bromofluorobenzene	460-00-4	100	102%	102%

Analyte	Spike Conc. (ug/kg)	MS %Rec	MSD %Rec	Duplicate RPD	
Benzene	50	113%	113%	0%	
Chlorobenzene	50	99%	97%	2%	
1,1-Dichloroethene	50	117% .	111%	5%	
Toluene	50	106%	103%	3%	
Trichloroethene	50	107%	105%	2%	

MS

MSD

Matrix Spike
Matrix Spike Duplicate
Percent Recovery
Relative Percent Difference % REC RPD



ANALYSIS REPORT: Purgeable Organic Analytes, EPA Method 8240

CLIENT: AEMC 11855 White Rock Road

Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90 Date Received: 9/20/90 Date Extracted: 9/20/90 Date Analyzed: 9/20/90 Date Reported: 9/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351 Batch No.: 6264

Analyte	LCS Conc. (ug/L)	LCS %Rec	
Benzene	50	103%	,
Chlorobenzene	50	103%	
1,1-Dichloroethene	50	104%	
Toluene	50	98%	
Trichloroethene	50	98%	

LCS - Laboratory Control Standards % REC - Percent Recovery. LCS

ANALYSIS REPORT: Oil & Grease, EPA Method 9071

CLIENT: AEMC 11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90 Date Received: 9/20/90 Date Extracted: 9/20/90 Date Analyzed: 9/21/90 Date Reported: 9/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351 Batch No.: 6251, 6252 Matrix: Soil

Sample Client	I.D. AEMC	Concentration (mg/kg)		Rpt. Limit (mg/kg)	
SB-1A	L5351-1	ND		50	
SB-1/2A	L5351-2	80		50̈́	
SB-2/3A	L5351-3	ND		50	
SB-3/4A	L5351-4	ND		50	•
SB-4/5A	L5351-5	ND		50	
SB-5/6A	L5351-6	ND		50	
SB-5/6B	L5351-7	600		50	
SB-6A *	L5351-8	ND		50	
SB-7A	L5351-9	3,200		50	
SB-7B	L5351-10	2,100		50	
Method Blank Batch 6251	L5351-MB	ND		50	•
SP-1-1 Composite	L5351-13	ND		50	·
SP-1-2 Composite	L5351-16	260		50	
SP-1-3	L5351-19	280		50	
SP-1-4	L5351-22	240		50	
SP-1-5	L5351-25	100	·	50	
SP-2-1	L5351-28	100		50	
SP-2-2 Composite	L5351-31	200		50	
SP-3-1 Composite	L5351-34	6,800 '		50	
SP-3-2 Composite	L5351-37	1,600		50	
SB-1B	L5351-38	· ND		50	· .
Method Blank Batch 6252	L5351-MB	ND		50	

RPT. LIMIT - Reporting Limit
ND - Not Detected at or above indicated Reporting Limit.



ANALYSIS REPORT: Oil & Grease, EPA Method 9071

CLIENT: AEMC

11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90 Date Received: 9/20/90 Date Extracted:9/20/90 Date Analyzed: 9/21/90 Date Reported: 9/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351 Batch No.: 6251 Matrix: Soil

Analyte	Spike Conc. (mg/kg)	MS %Rec	MSD %Rec	Duplicate RPD	
Oil & Grease	4,940	988	96%	1%	

MS

% REC

Matrix Spike
Matrix Spike Duplicate
Percent Recovery
Relative Percent Difference RPD



ANALYSIS REPORT: Oil & Grease, EPA Method 9071

CLIENT: AEMC 11855 White Rock Road Rancho Cordova, CA 95742

Project: Sears Roebuck Co.

Date Sampled: 9/19/90 Date Received: 9/20/90 Date Extracted:9/20/90 Date Analyzed: 9/21/90 Date Reported: 9/24/90

Project No.: Contact: T. Anckner

Phone:

AEMC Contact: M. Jaeger

Job No.: 50109 COC Log No.: 20427

AEMC I.D.: L5351 Batch No.: 6252 Matrix: Soil

Analyte	Spike Conc. (mg/kg)	MS %Rec	MSD %Rec	Duplicate RPD	·.
Oil & Grease	4,840	97%	98%	1%	

MS

MSD

% REC RPD

Matrix Spike
Matrix Spike Duplicate
Percent Recovery
Relative Percent Difference

CHAIN OF CUSTODY

ADDRESS - Oaklard	ELIENT JOB NUMBER		ANALYSIS REQUESTED	FIELD CONDITIONS:
ADDRESS	50109	B 1	14101	
	DESTINATION LABORATORY	PRESERVATIVES	ANCIE	COMPOSITE
PROJECT NAME S ROEBUCK CO	AETC 3249 FITZGERALD RD. RANCHO CORDOVA, CA.			composite: See Belaw
PHONE /	95670 •	IVES L	7 11 12 2	
1 111.1/6 0 11	OTHER		TE DOV	SPECIAL INSTRUCTIONS:
Soll Sampling under 6			25 graph 12 12 12 12 12 12 12 12 12 12 12 12 12	8240 O.K. & per Alameda Co Paul Snuth
I of tank 11 waste 011			12 N	
SITE LOCATION Calland	-		208天	TUHN AROUND TIME NOTE / FIELD READINGS
	CONTAINER	W	一年記る	HOURS HOURS WEEK 24 WEEK 2 WEEK 2
DATE IDENTIFICATION DEPTH METHOD		1) <u>S</u>	
6-19 SP-3-1A (PUSH	Soil Bras	1 1 1	XXXXX	X / CmP into
(SP-3-1B \ (XX	XXXX	
SP-3-2A		XX	XXXX	X Contintal
SP-3-2B V	6 6 8	XX	XXXXX	×
		CAMPLE	RETENTION TIME	
SUSPECTED CONSTITUENTS	5	SAMPLE	HELENTON HME	•
ARELINGUISHED BY DATE / TIME		ATE / TIME	REMARKS	PRESERVATIVES: (1) HCL (3) = COLD
P. Walsach 6/20 0630 M	Wester ?/	0630	!	(2) HNO ₃ (4)
May Long 4/20 9:30 M	1St Wester 9/2	01900	135	LAB TO SEND RESULTS TO:
0 0				Walson Anches
				Offiginal COPY
教练题引护PED VIA saaaaa FED X	Turs Toth	ER	____________________	IRBILL#

AMEHGAN-

CHAIN OF CUSTODY

CLIENT NAME Sears - Oakland	SHE GALMINOR YOU	DER MARK		وَيِهِ أُوْفِكُ مِنْ	ANA	LYSIS	HEC	UEŠ	rED	FIELD	CONDITIONO	ONS:				
ADDRESS	50109 — adistination labo	HATOHY.	PRESERVATIVES	_\	1)	9	\cup								Ì
PROJECT NAME			ERVA	型	+/	-i-l	<i>x</i>	77	δ	СОМРО	SITE:					
PROJECT MANAGER PHONE #	AETC 3249 FITZGERAL RANCHO CORDO	D RD. DVA, CA. 95670	IVIT	十		UI X	-^-	7 7	-							
Angkaen SAMPLED BY.		55010	S	(')	,	",	3	, ≯	•							
JOB DESCRIPTION	LJ OTHER			D	3		Graces	<u>[</u>	_/	SPEC1/	L INSTR	IUCTION	S:		-	
							D O	70	8					•		
SITE LOCATION (2)						1	(9	୍ଦି (ଜୁନ	0458040	104	N AHb	UNB 1	ME	NOTE /	FIELD READ	INGS
Cokland	***************************************		(v)				07	607	, K		ferre forest	Children In	der i Kles			
DATE TIME I IDENTIFICATION DEPTH METH		AINER MATYPERA	\mathbb{Z}				(1-	000	P	24 HOURS	48 HOURS	WEEK	2 WEEKS			
6-19 SB-1A Buch		Bus	V.			X	×	<u> </u>		1						
SB-1/ZA	2	1000		Ź		X	X			文						
SB-2/3A	7	17	X	V		X	X			X						
SB - 3/4A				X		X	X			×						
SB-4/SA		17	×	X		X	X			X					-	,
SB-5/6A			X	X		X	X			X						
SB-5/6B		1-}	X	Х		X	1			×						
SB-6A		17	X	メ	A	*	X			X						
SB-7A			X	X	X	メ	×	X	X	X						
SB-7B	1)		入	X	乂	X	X	X	X	X			基			
SPLIA		17	X	X		X	X			X			X	Compo	sito	do
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ENVIRONMENTAL MANAGEMENT CORP.

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Please Refer To: AEMC Job No. 82580

23 January 1991

Ms. Bernadine Palka Sears, Roebuck and Co. Sears Tower Department 731, BSC 39-34 Chicago, Illinois 60684

RE: SOIL CONTAMINATION ASSESSMENT REPORT MOTOR OIL TANK AREA SEARS, ROEBUCK AND CO., OAKLAND, CALIFORNIA

Dear Ms. Palka:

American Environmental Management Corporation (AEMC) was retained by Sears, Roebuck and Co. to remediate hydrocarbon contamination at its Automotive Center, 2633 Telegraph Avenue in Oakland, California (Figure 1). All work onsite was conducted under the regulatory guidance of Mr. Paul Smith, Alameda County Department of Environmental Management (ACDEM) and under the direct supervision of a State of California Registered Geologist.

On 15 November 1990, AEMC overexcavated the contaminated soil from side walls and bottom of the existing excavation in the Tank 5 and Tank 6 area. AEMC removed the concrete slabs under both of these tanks. During this excavation process, AEMC removed approximately 20 cubic yards of material while extending the depth of the excavation an additional 2 feet below ground surface (bgs).

AEMC gathered two confirmatory soil samples from the undisturbed soil remaining in the excavation (Figure 2). The samples were collected in a brass tube, sealed with Teflon tape, plastic end caps, and electrical tape. Both samples were then labeled, refrigerated, and delivered to AEMC's State of California Certified Laboratory (No. 210) under chain-of-custody for analysis.

Table 1 lists both the soil samples taken from the initial UST removal and the confirmatory soil samples after overexcavation. In addition, Figure 2 illustrates all of the sample locations.

Both confirmatory samples were analyzed for Total Petroleum Hydrocarbons as Diesel (TPH-D) and Oil and Grease. Sample SB-5/6C showed no detectable levels of TPH-D and 80 parts per million (ppm) of Oil and Grease. Sample SB-6B showed no detectable levels of either TPH-D or Oil and Grease (Appendix A).

In addition, Mr. Paul Smith of ACDEM required AEMC to perform an additional excavation in the Tank 1 and Tank 2 area. This excavation was not included in the original scope of the Workplan submitted 13 November 1990. AEMC excavated approximately 5 cubic yards of soil from between the concrete slabs of Tank 1 and Tank 2. AEMC gathered one confirmatory soil sample (SB-1/2B) from this area. The sample was analyzed

Ms. Bernadine Palka Sears, Roebuck & Co. 23 January 1991 Page 2

for Total Petroleum Hydrocarbons as Diesel (TPH-D) and Oil and Grease. This sample showed less than 60 ppm TPH-D and 280 ppm of Oil and Grease (Appendix A).

The cleanup goal for TPH-D at the site was established at 100 ppm based on the Leaking Potential Analysis in the State Water Resources Control Board's Leaking Underground Fuel Tank Manual (October 1989). The Oil and Grease analysis did not have a cleanup goal. It did provide, however, a guide for establishing additional sampling requirements.

Based upon the results of the laboratory analyses of the soil samples, AEMC obtained regulatory approval on 18 November 1990 to backfill the excavation. However, ACDEM required that AEMC determine whether groundwater had been affected by hydrocarbon contamination due to Oil and Grease contamination still present in the soil. AEMC will submit a Groundwater Contamination Assessment Workplan for this site to ACDEM for approval. AEMC plans to implement this workplan in conjunction with the waste oil tank area contamination assessment.

AEMC lined the motor oil tank excavation with Supac geotextile and backfilled the excavation with pea gravel. This material was compacted to grade, covered with base rock, and asphalted.

AEMC delivered the contaminated soil from the motor oil tank excavation to Gibson Asphalt Recyclers in Bakersfield, California. The soil was transported under Bills of Lading from the site to Gibson (Appendix B).

If you have any questions, please feel free to contact either Mr. Phil Walsack or me at (916) 364-8872. STERED GEOLOGIS,

太

JAMES F. FRUMM

No. 4207

OF CALIFORN

Sincerely,

James F. Frumm, R.G., R.E.A.

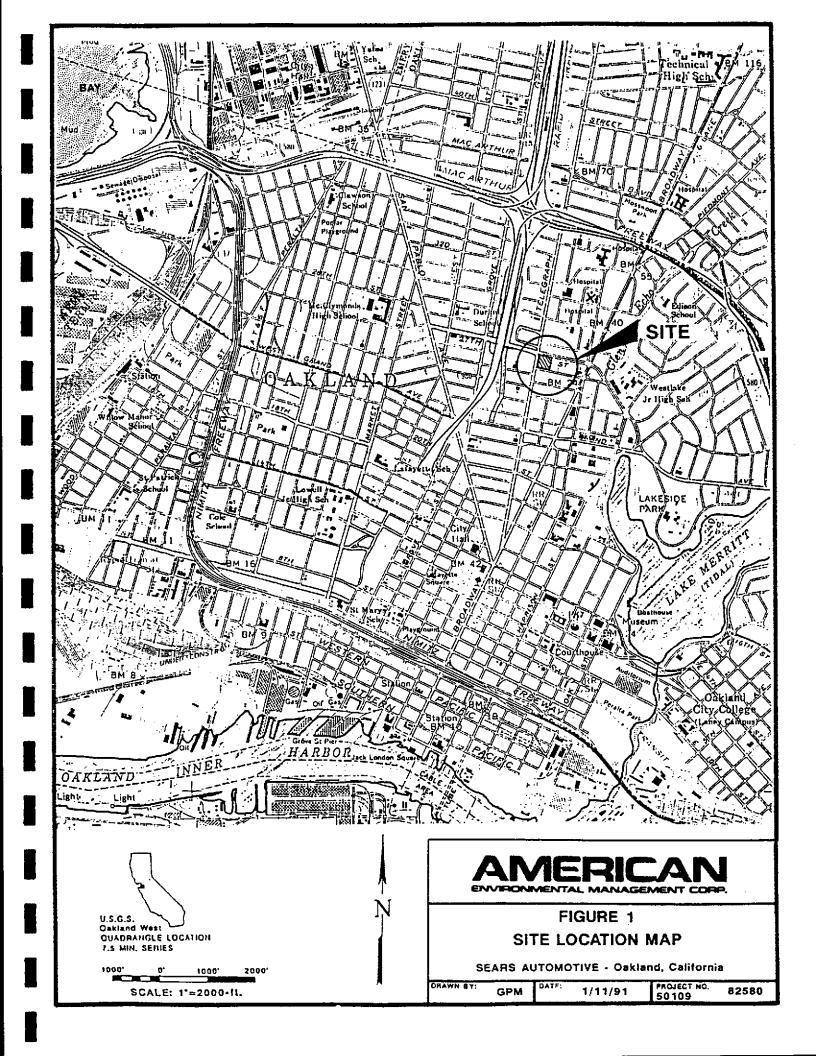
James H. Hun

Regional Manager Engineering Division

PW/scg 12src-01(pw-3)

Attachments

cc: Mr. Paul Smith, Alameda County Department of Environmental Health



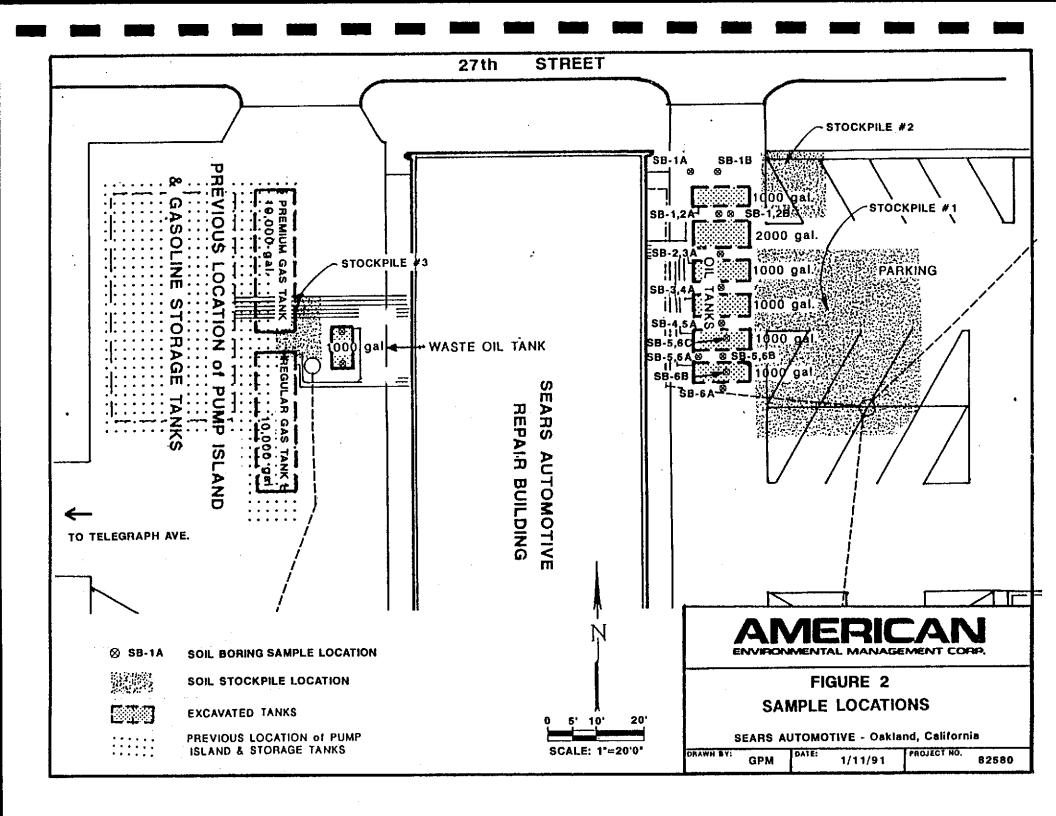


TABLE 1
Analytical Results of Soil Samples

Sears, Roebuck and Co. Oakland, California

Motor Oil Tank Area

Sample ID	Date	Depth (feet below ground surface)	TPH-D (ppm)	Oil & Grease (ppm)
Excavation				
SB-1A SB-1B SB-1/2 A SB-1/2B SB-2/3 A SB-3/4 A SB-4/5 A SB 5/6 A	9/19/90 9/19/90 9/19/90 11/15/90 9/19/90 9/19/90	10 10 10 12 10 10	ND ND ND <60 ND ND ND ND	ND ND 80 280 ND ND ND ND
SB-5/6 B SB-5/6C SB-6A SB-6B	9/19/90 9/19/90 11/15/90 9/19/90 11/15/90	11 10 12 9 11	390 ND ND ND	80 ND ND

^{*} Sample also contained Ethylbenzene @ 13 ppb and Xylenes @ 14 ppb.



AEMC

9719 Lincoln Village Dr. Sacramento, CA 95727

Attn: Phil Walsack

Re:

Project: Sears Oakland Project No.: Chain of Custody number: 22552 Date Samples Received: 11/15/90 No. Samples Received: 3

Job No.: 82580 AELC Lab No. : L5674

11/27/90

These samples were received by American Environmental Laboratories in a chilled, intact state, and accompanied by valid chain of custody documentation.

The following analysis were performed on the above referenced project:

No. of Samples	Analysis
	
3	TPH Diesel by LUFT Method
3	EPA 9071 Oil and Grease

Analytical results are attached to this letter. Please call if we can provide additional assistance.

Sincerely.

George Hampton

Laboratory Director

AMERICAN

CA DOBS ELAP Accreditation/Registration Number 1233

Total Petroleum Hydrocarbons, EPA Method 8015 Shaker, DOHS Luft Method $\,$ ANALYSIS REPORT:

CLIENT: AEMC

9719 Lincoln Village Dr. Ste. 501 Sacramento, CA 95827

Project No.: Contact: P. Walsack

Phone:

Project: Sears Oakland

AEMC Contact: M. Jaeger

Job No.: 82580 COC Log No.: 22552

Date Sampled: 11/15/90 Date Received: 11/15/90 Date Extracted:11/15/90 Date Analyzed: 11/16/90 Date Reported: 11/21/90

AEMC I.D.: L5674

Matrix: Soil

Sample :			. TPH as Diesel
Client	AEMC	Batch #	(mg/kg)
SB-5/6C	L5674-1	6586	ND
SB-6B	L5674-2	6586	ND
SB-1/2B	L5674-3	6586	ND(60)
Method Blank	L5674-MB	6586	ND

REPORTING LIMIT*

10

*Unless otherwise indicated in parentheses

ND - Not Detected at or above indicated Reporting Limit.



CA DOES ELAP Accreditation/Registration Number 1233

Total Petroleum Hydrocarbons, EPA Method 8015 Shaker, DOHS Luft Method ANALYSIS REPORT:

CLIENT: AEMC 9719 Lincoln Village Dr. Ste. 501 Sacramento, CA 95827 Project No.: Contact: P. Walsack

Phone:

Project: Sears Oakland AEMC Contact: M. Jaeger

Date Sampled: 11/15/90 Date Received: 11/15/90 Date Extracted:11/15/90 Date Analyzed: 11/16/90 Date Reported: 11/21/90 Job No.: 82580 COC Log No.: 22552

AEMC I.D.: L5674

Matrix: Soil

Analyte	Spike Conc. (mg/kg)	MBS %Rec	MBSD %Rec	Duplicate RPD	
Diesel	100	104%	102%	2%	

MBS = Method Blank Spike

MBSD = Method Blank Spike Duplicate

Rec = Percent Recovery

Relative Percent Difference



CA DOHS ELAP Accreditation/Registration Number 1233

Total Petroleum Hydrocarbons, EPA Method 8015 Shaker, DOHS Luft Method $\,$ ANALYSIS REPORT:

CLIENT: AEMC 9719 Lincoln Village Dr. Ste. 501 Sacramento, CA 95827

Project No.: Contact: P. Walsack

Phone:

Project: Sears Oakland

AEMC Contact: M. Jaeger

Date Sampled: 11/15/90 Date Received: 11/15/90 Date Extracted:11/15/90 Date Analyzed: 11/16/90 Date Reported: 11/21/90

Job No.: 82580 COC Log No.: 22552

AEMC I.D.: L5674

Analyte

LCS Conc. (mg/L)

LCS &Rec

Diesel

1,000

106%

LCS - Laboratory Control Standards

- Percent Recovery % Rec



CA DOHS ELAP Accreditation/Registration Number 1233

ANALYSIS REPORT: Total Oil & Grease, EPA Method 9071 Shaker, DOHS Luft Method

CLIENT: AEMC 9719 Lincoln Village Dr. Ste. 501 Sacramento, CA 95827 Project No.: Contact: P. Walsack

Phone: AEMC Contact: M. Jaeger

Project: Sears Oakland

Job No.: 82580 COC Log No.: 22552

Date Sampled: 11/15/90 Date Received: 11/15/90 Date Extracted:11/15/90 Date Analyzed: 11/16/90 Date Reported: 11/21/90

AEMC I.D.: L5674 Batch No.: 6590 Matrix: Soil

Sample I.D. Client	AEMC	Total Oil & Grease (mg/kg)	
SB-5/6C	L5674-1	80	
SB-6B	L5674-2	ND	
SB-1/2B	L5674-3	280	
Method Blank	L5674-MB	ND	

Reporting Limit*

50

* Unless otherwise indicated within parentheses.
ND = Not Detected at or above indicated Reporting Limit.

THIS PERMIT MUST BE LEFT ON THE WORK AS AUTHORITY THEREFOR

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CA DOHS ELAP Accreditation/Registration Number 1233

ANALYSIS REPORT: Total Oil & Grease, EPA Method 9071 Shaker, DOHS Luft Method

CLIENT: AEMC 9719 Lincoln Village Dr. Ste. 501 Sacramento, CA 95827

Project No.: Contact: P. Walsack

Phone:

Project: Sears Oakland

AEMC Contact: M. Jaeger

Job No.: 82580 COC Log No.: 22552

Date Sampled: 11/15/90 Date Received: 11/15/90 Date Extracted:11/15/90 Date Analyzed: 11/16/90 Date Reported: 11/21/90

AEMC I.D.: L5674 Batch No.: 6590 Matrix: Soil

Analyte	Spike Conc (mg/kg)	MS %Rec	MSD %Rec	Duplicate RPD	
Oil & Grease	1040	93%	82%	13%	

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- Matrix Spike - Matrix Spike Duplicate - Percent Recovery MSD

% REC

- Relative Percent Difference

ENVIRONMENTAL MANAGEMENT CORP.

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Job No. 82580

PRELIMINARY REPORT and CONTAMINATION ASSESSMENT WORKPLAN

for

SEARS, ROEBUCK and CO. Oakland, California

4 January 1991

prepared by the

AMERICAN ENVIRONMENTAL MANAGEMENT CORPORATION
Engineering Division
9719 Lincoln Village Drive, Suite 501
Sacramento, California 95827
(916) 364-8872

I hereby certify that this
Preliminary Report and Contamination Assessment Workplan
for
Sears, Roebuck and Co. of Oakland, California
was prepared under my direct supervision.

James F. Frumm, R.G., R.E.A.
Regional Manager
Engineering Division
American Environmental Management Corporation

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JAMES F. FRUMM

No. 4207

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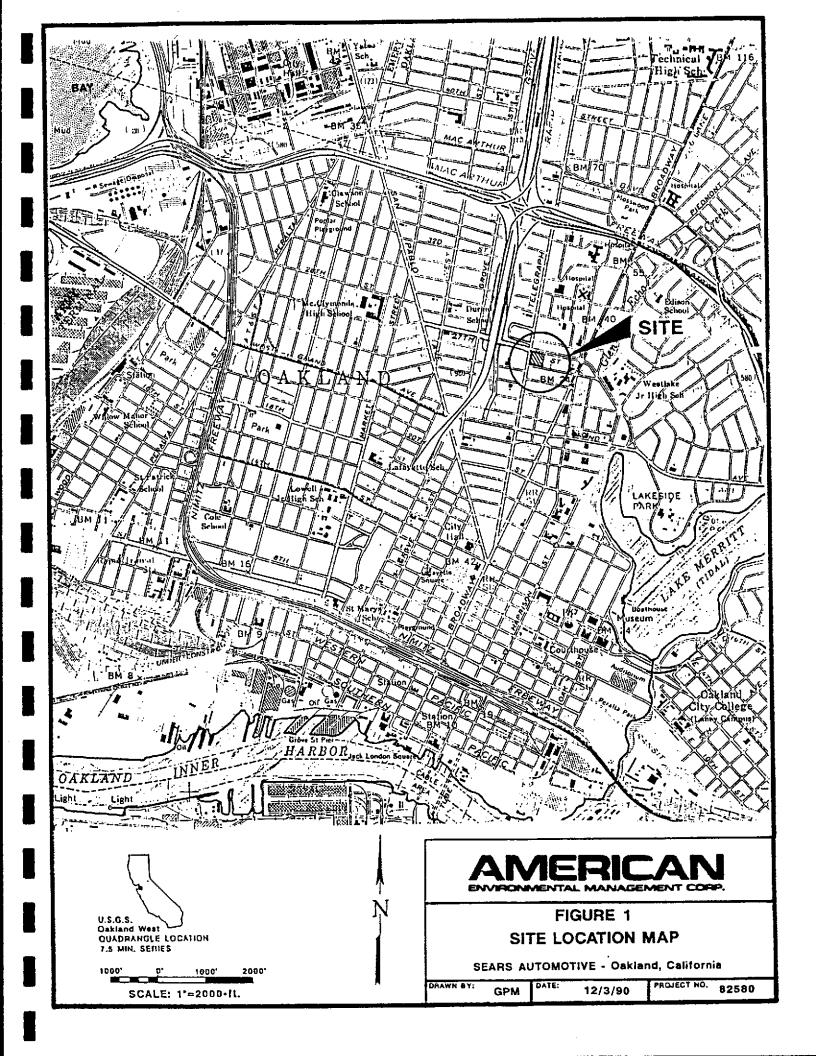
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INTRODUCTION

American Environmental Management Corporation (AEMC) has been retained by Ms. Bernadine Palka of Sears, Roebuck and Co. (Sears) to submit a Preliminary Report (PR) and Contamination Assessment Workplan regarding the hydrocarbon contamination discovered during the removal of seven (7) underground storage tanks (USTs) at the company's automotive repair facility located at 2633 Telegraph Avenue, Oakland, California (Figure 1).

The purpose of this report is to summarize initial investigative results and to develop a workplan which will assess the vertical and lateral extent of petroleum hydrocarbon contamination at the site.



INITIAL SITE INVESTIGATION

Sears has maintained seven (7) underground storage tanks to store oil products for automotive servicing. All of the USTs were installed in the early 1960s and were removed by AEMC during the week of 17 September 1990.

Two separate excavations were opened during the UST removals. Six motor oil tanks were removed from an excavation to the east of the service bays, and one waste oil tank was removed from an excavation to the west of the service bays (Figure 2). Due to the presence of hydrocarbon contamination in both excavations, the site characterization and remediation has been divided into two separate parts, the motor oil tank area and the waste oil tank area. This Preliminary Report and Contamination Assessment Workplan addresses the waste oil tank area.

On 19 September 1990, the 1,000-gallon waste oil tank (Tank 7) was excavated and removed by AEMC. It was noted that the tank had two holes in the bottom of its southern side and many corrosion pinholes. Soil in the excavation was stained. Two soil samples, SB-7A and SB-7B were gathered 9 feet below ground surface (Table 1). AEMC's letter report dated 12 October 1990 summarizes the tank excavation and removal activities.

Approximately 30 cubic yards of excavated soil is presently stockpiled northwest of the site. Two samples, SP-3-1 and SP-3-2, were obtained from the stockpile to characterize the degree of contamination (Table 1). The stockpile is placed on and covered with Visqueen sheeting.

Page 3

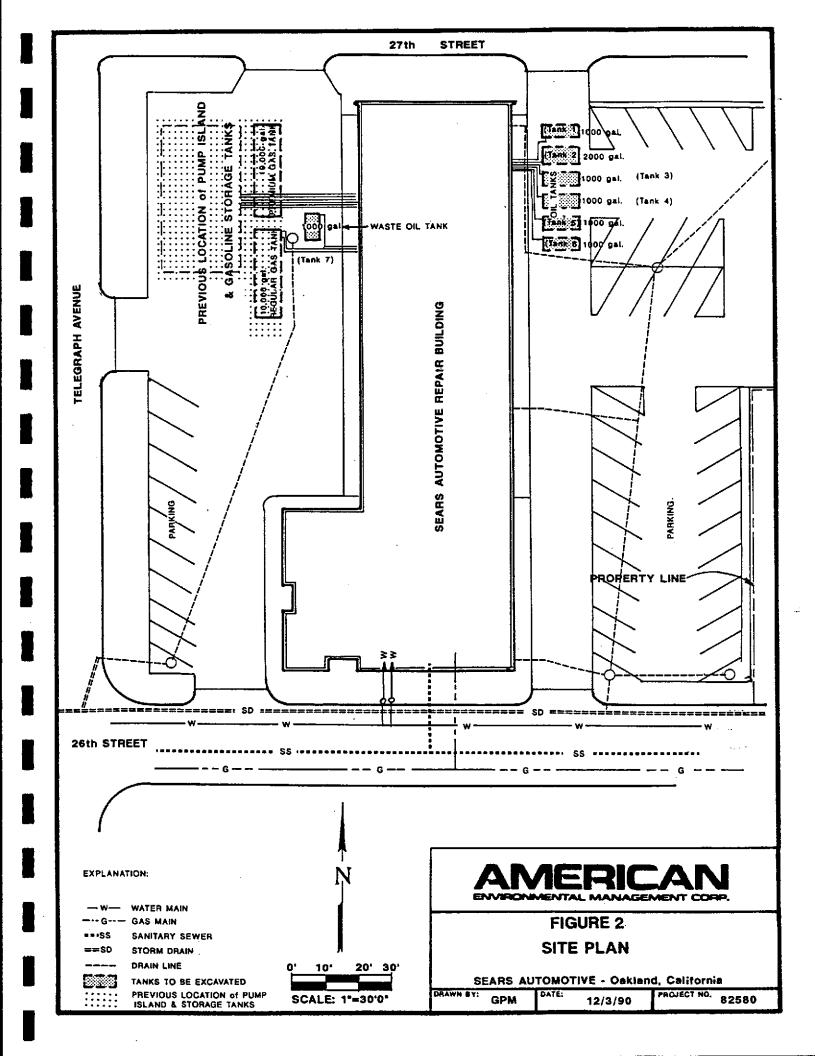


TABLE 1

Analytical Results of Soil Samples Sears, Roebuck and Co. Oakland, California

Waste Oil Tank Area

Sample ID	Depth (feet bgs)	TPH-G (ppm)	TPH-D (ppm)	Oil & Grease (ppm)	B (ppb)	T (dqq)	E (ppb)	X (ppb)	Cd (ppm)	Cr (ppm)	Ni (ppm)	Pb (ppm)	Zn (ppm)
Excavation													
SB-7A ^a SB-7B ^b	9 9	31 31	2,800 1,500	3,200 2,100	ND 12	58 200	100 250	720 1,400	ND ND	33 28	28 24	360 190	54 64
Stockpile			:										
SP-3-1 ^c SP-3-2	- .	39 13	4,400 850	6,800 1,600	ND ND	31 0	410 23	3,000 220	ND 1	20 32	20 34	440 47	62 45

bgs	below ground	surface
TPH-G TPH-D B T X E Cd Cr Ni Pb Zn		im Hydrocarbons as gasoline im Hydrocarbons as diesel
a Sampl	e also contained:	Tetrachloroethene @ 82 ppb Trichloroethene @ 17 ppb
b Sampl	e also contained:	Acetone @ 140 ppb Tetrachloroethene @ 7 ppb Trichloroethane @ 19 ppb
c Sampl	e also contained:	Tetrachloroethene @ 52 ppb

GEOLOGY AND GEOHYDROLOGY

The site, located in the City of Oakland, lies within the San Francisco Bay area of the Coast Range Geomorphic Province. This province is characterized by a series of nearly parallel mountain ranges that trend obliquely to the coast in a northwesterly direction. The alignment of the major fault zones throughout the San Francisco Bay area trend in the same northwesterly direction. The area surrounding the site is bounded by the seismically active Hayward Fault to the east and the San Francisco Bay to the west.

The bedrock underlying most of the East Bay area is composed of sandstone, siltstone, chert and greenstone of the Franciscan Formation and is Jurassic-Cretaceous in age. The Franciscan Formation is overlain by preconsolidated "old bay mud," sand deposits and "young bay mud" of the Cenozoic Age.

AEMC has observed a bay mud layer of unknown thickness beginning approximately 10 feet below grade. Groundwater in the area is believed to be approximately 25 feet below ground surface. At this time, the groundwater flow direction is not known, but tidal action may influence it.

PROPOSED PRELIMINARY INVESTIGATION

The proposed preliminary investigation is divided into four phases:

PHASE I—CONDUCT ELECTRONIC CONE PENETROMETRY SURVEY

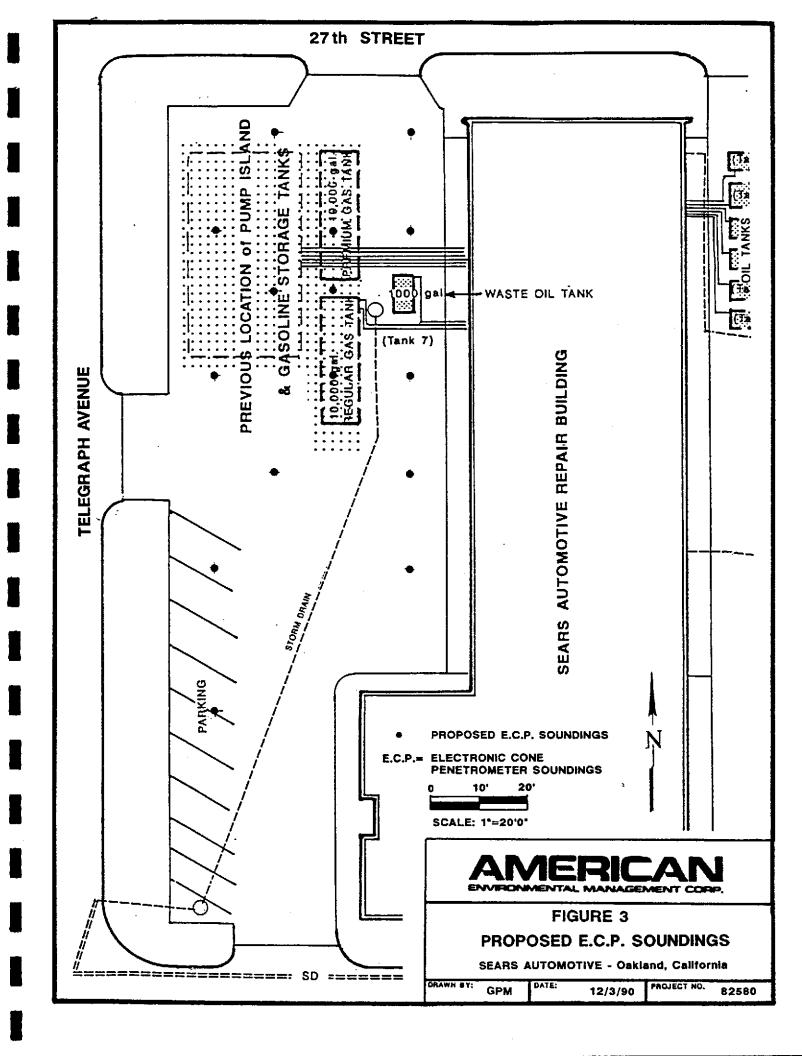
AEMC proposes to use electronic cone penetrometry (ECP) as a tool to determine the lateral and vertical extent of soil stratigraphy above the uppermost groundwater beneath the Sears site. ECP is an in-situ method which involves hydraulically advancing a small diameter cone-shaped electronic probe vertically into the soil. Resistance to probe penetration, and changes in pore water pressures with depth are measured electronically. AEMC will use this data to determine changes in soil types and permeabilities with depth. In addition, the ECP will enable AEMC to determine uppermost groundwater elevation, thus establishing the hydraulic gradient. Since this method does not require the use of a drill rig, the collection, containment and disposal of drill cuttings is avoided. The method is well suited to restricted work spaces, like Sears' heavily used parking lot. See Appendix A for additional information regarding the ECP process.

AEMC proposes to complete fifteen (15) ECP soundings, each to the depth of the uppermost water bearing unit. Upon completion of all soundings and sampling, each sounding location will be surveyed to provide a base of reference. Each ECP sounding borehole will be backfilled to grade with injected cement/bentonite grout to grade, in accordance with Alameda County requirements. Figure 3 illustrates the proposed locations for each ECP sounding.

PHASE II-SOIL SAMPLING AND ANALYSES

AEMC proposes to conduct soil sampling adjacent to the completed ECP soundings. The purpose for the sampling effort will be to determine the lateral and vertical extent of hydrocarbon and metals contamination in the soil profile above uppermost groundwater.

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AEMC proposes to advance a total of fifteen (15) soil sampling soundings at the locations illustrated on Figure 4.

AEMC will advance each borehole with the ECP hydraulic press and collect soil samples with the ECP retractable cone tipped sampler as described in Appendix B. Soil samples will be collected every 5 feet downhole beginning at 5 feet below grade to the capillary fringe of the uppermost groundwater.

The soil samples will be analyzed for Total Petroleum Hydrocarbons as Gas and Diesel by EPA Method 8015-m, Oil and Grease by EPA Method 9071, Purgeable Organic Compounds by EPA Method 8240, and Lead by EPA Method ICP/AA (Total Threshold Limit Concentration). All laboratory analyses will be conducted by American Environmental Laboratories Corporation (State Certification No. 210).

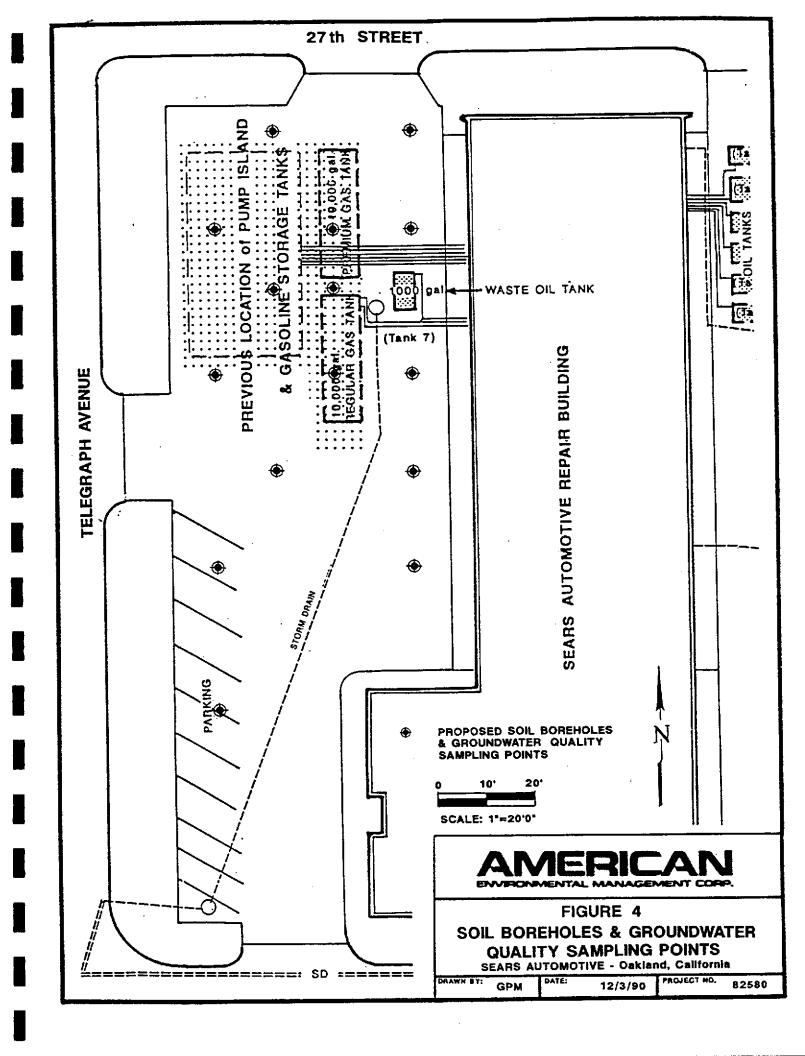
AEMC proposes to analyze the soil samples in several stages. The first stage of analysis will consist of soil taken from the five (5) sounding locations closest to the excavation. A second series of analyses will be initiated to further define a zero line of contamination. This process will continue until a zero line is fully defined.

PHASE III—SAMPLE UPPERMOST GROUNDWATER QUALITY

AEMC proposes to sample the uppermost groundwater quality using the ECP with the Hydropunch and BAT sampler as described in Appendix C. AEMC will have confirmed the depth (BGS) of the groundwater by the ECP sounding, thereby establishing the groundwater flow direction. A total of fifteen (15) groundwater quality samples will be obtained, one from each borehole.

The groundwater samples will be analyzed for Total Petroleum Hydrocarbons as Gas and Diesel by EPA Method 8015-m, Oil and Grease by EPA 9071 and Purgeable Organic Compounds by EPA Method 8240.

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AEMC proposes to analyze the groundwater samples in several stages. The first stage of analysis will consist of groundwater taken from the five (5) boreholes closest to the excavation. A second series of analyses will be initiated to further define a zero line of contamination. This process will continue until a zero line is fully defined.

Based on these analytical data, AEMC will determine whether groundwater quality has been affected by hydrocarbon contamination. If all of the groundwater samples are below action levels, AEMC will not install monitoring wells. If groundwater contamination exists, AEMC will be able to place recovery wells and monitoring wells in effective locations based on the ECP soundings and analytical results.

PHASE IV—REPORT PREPARATION

AEMC will prepare a Contamination Assessment Report which will describe the findings of the preliminary investigation. The report will also present recommendations for remedial activities to be conducted.

APPENDIX A

ELECTRIC CONE PENETROMETER

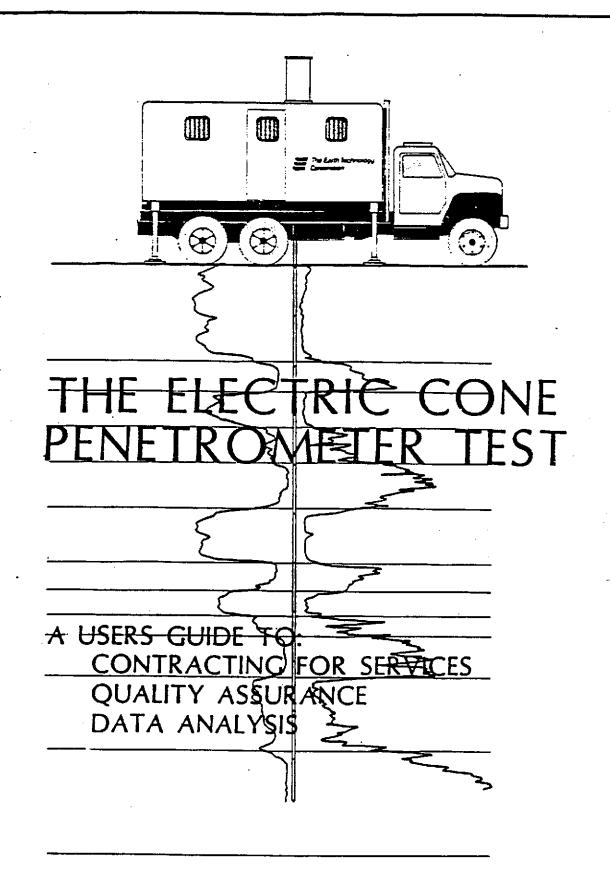




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1.0 INTRODUCTION

The purpose of this manual is to provide the practicing geotechnical, civil, or structural engineer with a simple set of summary guidelines related to the use of the electric Cone Penetrometer Test (CPT). The guidelines are intended only as advice to a responsible engineer, and are not intended to replace thorough study of the topic. However, the information contained herein will allow the engineer new to Cone Penetrometer Testing to become familiar with the basic use and pitfalls of the method.

The CPT has been used in European countries for many decades, and so most practicing engineers in those countries are familiar with its standard use and limitations. However, most engineers in the United States have not had such exposure to the method and so have little familiarity with what the CPT is used for, how to use it, when and where to use it and when to expect difficulties in its use.

This manual will present basic recommendations on those topics, including how to contract for services, how to assure data quality, how to interpret and apply the data, and when to require supporting information. The manual closes with a list of recommended references for those needing or desiring additional information.

2.0 WHEN TO USE THE CPT

The CPT is unequaled for delineation of subsurface strata. Whenever site conditions are appropriate, the use of the CPT will benefit a geotechnical exploration project. Essentially all CPT investigations are rapid, provide continuous data, provide repeatable data and utilize automated data logging. Estimates of virtually any soil property can be obtained from CPT data. These factors combine to make the CPT the premier tool for soil stratigraphic logging.

The CPT cannot reliably be used in cobbles, boulders and rock. Further, because there is no historical familiarity with the CPT, most projects require simultaneous conventional borings. Finally, the CPT method cannot provide the material characterization sometimes needed and which can only be obtained by use of other in-situ tests or careful borings, sampling and laboratory testing. The result of these factors is that for very small exploration projects and projects in either very unknown areas or areas known to have critical conditions, careful assessment of the suitability of the CPT should be performed well before project initiation. The potential benefits of the CPT method encourage such assessment on a routine basis.

The ideal use of the CPT is in areas of known geology with soils being gravelly sands or finer. Caliche-rich soils can effectively prevent penetration. A penetration depth of at least 50 feet can be expected when using a full 20 ton CPT system in sites not characterized by extensive coarse grained deposits. Penetration depths in excess of 250 feet have routinely been achieved in fine grained soils.

The questions that need to be answered from the exploration and testing, for example, definition of extent of strata and pile capacity, should be thoroughly defined before any testing is started. In addition, typical soil conditions or properties that separate critical from non-critical conditions should be defined in advance of the first test. In this manner all exploration and testing becomes pertinent to the project needs.

A typical CPT program is laid out and performed to provide the first delineation of overall site subsurface characteristics. Field locations are selected on the basis of planned structural layout, geology, and the size of a subsurface feature of importance to the site behavior. The CPT data are reviewed usually in the field, and the depths of correlation/verification samples are determined. The number of samples are reduced to the minimum required to characterize each critical stratum; exploration dollars are spent on obtaining high quality samples rather than on a high volume of samples.

Not all strata need to be routinely sampled. For example, a dense sand deposit is unmistakeble from the CPT, and if liquefaction is the concern of the investigation then no further tests will be required. However, if the strength of a clayey silt stratum is critical, then careful sampling and laboratory testing should supplement the CPT data. The CPT data can then be used to extrapolate the laboratory results across the site. Finally, if some characteristic of the soil is of importance which cannot be estimated from the CPT, for example, pH, then adequate samples should be obtained site wide.

3.0 CONTRACTING FOR CPT SERVICES

The best protection that a user of CPT services can have against a poor field program or inadequate data is to know the consultant/contractor who provides the service. It is virtually impossible to contractually prevent a substandard service. However, there are guidelines that can be applied to the contracting process to help ensure an appropriate service. The application of these guidelines requires some familiarity with either the supplier of the service or with the CPT method itself. Although there is an ASTM standard for the CPT (ASTM D3441-79), the standard alone cannot ensure the adequacy of service.

3.1 Equipment

The equipment to be used should be specified as to electrical or mechanical cone, the reaction mass available for pushing (not necessarily the same as the system weight or hydraulic system capacity), and the type of data logging. If specifying electrical cones, the specific sensors needed such as tip, sleeve, inclination, seismic and piezometer should also be specified. Electrical cone instruments are typically available in at least two sensitivities; high sensitivity for measurements in very soft soils and normal sensitivity for the broad spectrum of penetratable soils. A typical multi-channel instrument is shown in Figure 1 and brief descriptions of the use of the different sensors is given in Table 1.

3.2 Calibration

The contract should require documentation of general procedures used to calibrate the instruments and obtain the data. This includes requiring proof that the instruments are periodically subjected to measurement of the calibration errors, including repeatability, non-linearity, zero load, and hysteresis errors. These terms are defined in Figure 2.

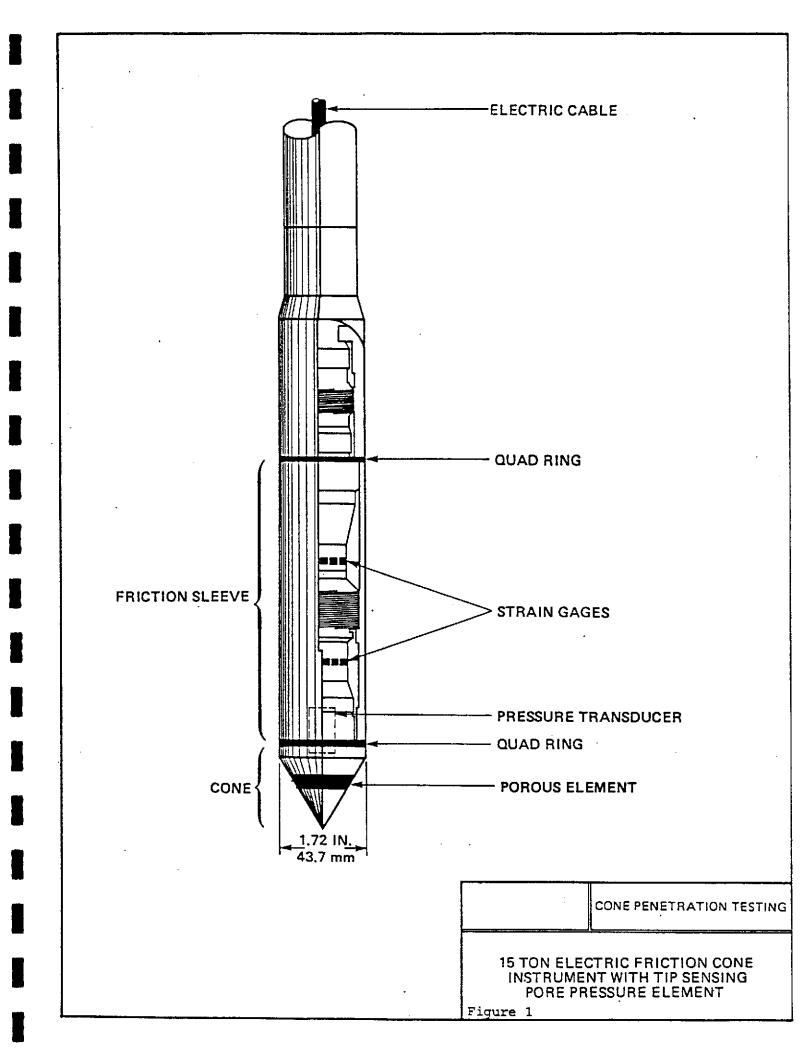


TABLE 1

TYPES OF CPT SENSORS

- 1. Standard CPT (cone end bearing and friction sleeve)

 o measures soil resistance to mechanical penetrations, in bearing and sliding shear failure
 modes.
- 2. Piezometric CPT (addition of pore pressure sensor)

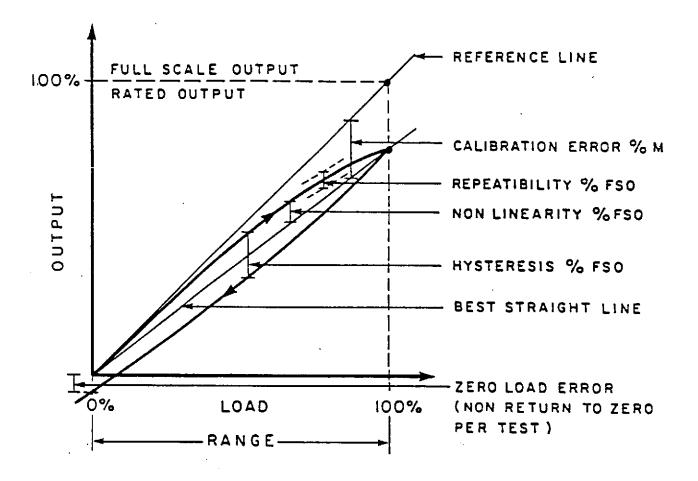
 o measures soil pore pressure response to mechanical penetration; can be used to obtain ambient pore pressures and an indication of permeability. Two designs are common: ported through cone tip or ported through front of sleeve. These designs give radically different measurements, especially in stiff soils.
- Resistivity CPT (addition of electrical field measurement)

 o measures electrical resistance of soil around CPT

 instrument; responds to degree of saturation and
 electrolyte type.
- 4. Thermal CPT (addition of thermistors)

 o measures soil thermal response to mechanical penetration; can be used to determine ambient temperatures.
- 5. Seismic CPT (addition of geophones)

 o measures soil response to surface seismic excitation, with superior sensor-soil coupling. Down hole and crosshole tests may be performed.
- 6. Nuclear CPT (addition of nuclear moisture-density-source)
 o measures soil response to low level radiation, indicating in situ densities and moisture content.
- 7. Pressuremeter CPT (addition of a pressuremeter cell) o measures radial response of soil to radial expansion and contraction of cell.
- 8. Fluid Sampler CPT (addition of lysimeter)
 o allows acquisition of select or continuous samples of in situ gases or liquids.



% FSO PERCENTAGE OF FULL SCALE OUTPUT % M PERCENTAGE OF MEASURED OUTPUT

DEFINITION OF TERMS TO CALIBRATE (6)
FIGURE 2



3.2.1 Operational Checking

The data report should reference these procedures and typical values and should include a statement of the difference between at least one field calibration. Further, the zero load error at the end of each test should be noted. If tests are performed under water, then the response of the sensor to hydrostatic pressures must also be determined and reported.

3.2.2 Piezocone Saturation

Special attention must be given in the contract to use of documented procedures for saturation of the piezometer element that is used. Lack of saturation will result in highly misleading data. Further, there is at this time little guarantee, and no certain way to check, that the element will remain saturated during the penetration process. Loss of saturation can result from the interaction of location and type of porous element, saturating fluid, soil density and soil degree of saturation. The most reliable way to obtain good piezometer data is through pre-project discussion with a reputable service during which procedures appropriate to the project needs can be defined. For example, Earth Technology has, on many projects, installed casing with the CPT truck to below the water table in order to reduce the possibility of piezometer saturation loss in unsaturated soils above the water table.

3.2.3 CPT Operator

The actual performance of a CPT test requires a highly trained, competent individual who is responsive to the test characteristics and the geologic environment that he is testing. Although the CPT is relatively operator independent, compared to the SPT for instance, a knowledgable, observant operator can maximize the quality of the results. In particular the operator should be capable of fully calibrating the instrument during a job if needed, performing field inspections and repairs of instruments and electronics, and fully documenting the test.

3.3 Data Presentation

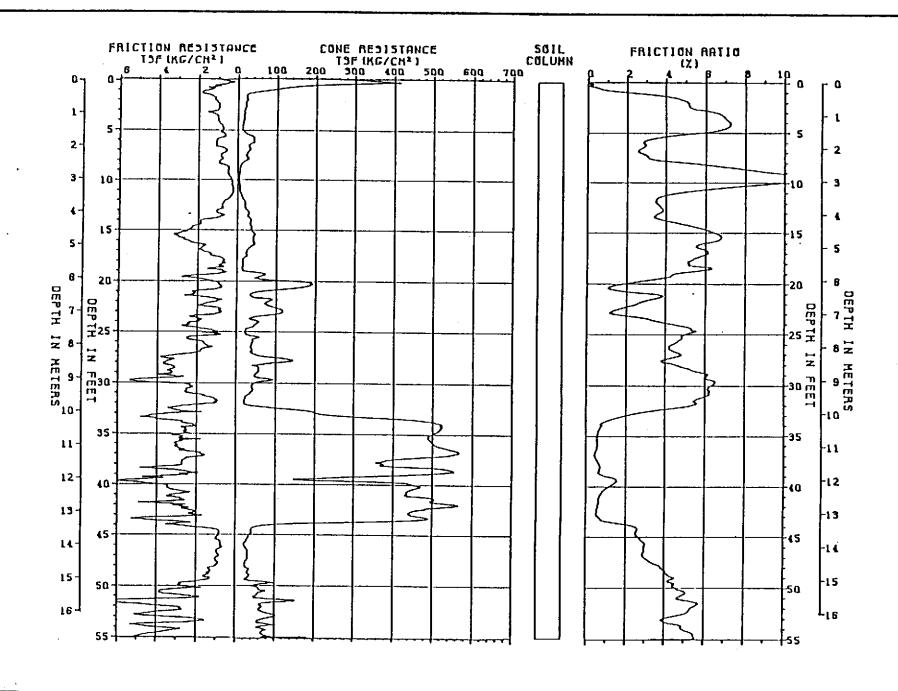
The minimum depth interval at which data are obtained from each sensor, typically 2.0cm, should be specified. Likewise, the scales at which measured and interpreted data are to be reported should be specified. Typically the basic data should be shown as continuous plots of tip bearing, sleeve friction, and pore pressure if obtained, in TSF, kg/cm², or other as desired, versus depth in feet or meters. Inclination is usually expressed simply as a single value indicating maximum inclination observed during the test.

Standard data processing should include generation of a continuous plot of friction ratio, and pore pressure ratio if obtained, versus depth at specified scales, as shown, for example, in Figure 3. The offset distances representative of physical dimensions of the instrument and used in the calculation of these ratios should be noted in the report, as should any filtering or averaging of the data.

Other basic information that should usually be included in a report includes tabulation of depth and measured and calculated values, typically at a one foot interval, as well as interpreted information such as Soil Behavior Type, Equivalent SPT N₁ values, Cyclic Strength, and Equivalent Drained Friction Angle or Undrained Strength, and Equivalent Relative Density. An example of such a tabulation is given in Figure 4. The contract specification should always require description of the method used to develop such interpreted data.

3.4 Report

Final report contents should be specified. That report, in summary, would include documentation of time and location of each test, equipment used, depths achieved, refusal criteria invoked, (target depth achieved, total capacity exceeded, rod buckling initiated, see Table 2) documentation of calibration and saturation procedures and field checks of calibrations, maximum test inclination, plots of the measured and interpreted values. An example of a

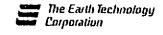


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INSTRUMENT NUMBER: F15CKE070

DATE: 15 AUGUST 1984



CONE PENETROMETER TEST

FIGURE 3 - TYPICAL SOUNDING LOG

DEPIH CONE	FRICTION TSF	FRIC RATIO	SOIL BEHAVIOR TYPES	EGUIV RELATIVE GENSITY		EQUIV SPT	EGUIV	SU1= (C-T)/NC KSF	SU2= FS+A KSF
									- ,
1 0 96,78	1. 31	2. 69	SANDY SILT-CLAYEY SILT	80-100	35-39	٥1	65		
2.0 24.16	1.19	5. 14	CLAY	UNDEF	UNDEF	20	27	3, 43	2, 38
3.0 16.20 4.0 14.10	1.03	6. 69	CLAY	UNDEF	UNDEF	20	27	2. 29	2, 06
5.0 12.49	1.02 0.77	7, 26 5, 30	CLAY	UNDEF	UNDEF	19	26	1. 98	2. 04
6.0 39.88	1. 10	2. 88	CLAY	UNDEF	UNDEF	13	20	1.61	1. 54
7.0 30.92	0. 33	2. Fit	CLAYEV SILT-SILTY CLAY CLAYEV SILT-SILTY CLAY	60-80	27-31	30	25	5, 65	2, 20
6.0 22.51	0. 87	5. 6L	CLAY SICI-SICIT CLAY	40-80 UNDEF	27-31	15	-21	4. 36	1, 10
9.0 4.84		10. 12	ORGANIC MATERIALS	UNDEF	UNDEF	21	28	3. 15	1. 74
10.0 2.72	0. 30	7. 44	ORGANIC MATERIALS	UNDEF	UNDEF	10	17	0, 62	0.94
11 0 5.56	o. 22	4. 33	CLAY	UNDEF	UNDEF	7 5	14	0.30	0. 60
12 0 15, 79	0. 63	3. 58	SILTY CLAY TO CLAY	UNDEF	UNDEF	11	12 16	0, 70 2, 30	0. 44
13, 0 25, 48	1.06	3, 55	SILTY CLAY TO CLAY	UNDEF	UNDEF	15	53	2. 30 3. 53	1. 26 2. 12
14 0 35, 50	1. 90	4.77	CLAY	UNDEF	UNDEF	33	40	4. 95	3. 80
15.0 37.99	2. 51	6. 57	CLAY	UNDEF	UNDEF	52	59	5. 30	5. 02
16.0 40.46	2. 52	5, 74	CLAY	UNDEF	UNDEF	41	48	5. 63	3.04
17. 0 23. 95	1. 76	6. Cá	CLAY	UNDEF	UNDEF	33	40	3. 28	3, 52
18 0 13, 19	9. 72	5. 17	CLAY	UNDEF	UNDEF	13	20	1. 73	1. 44
19 0 17, 56		4. 35	CLAY	UNDEF	UNDEF	14	Žί	2. 35	1. 40
20.0 166.74	1. 20	1. 59	SAND TO SILTY SAND	60-80	39-42	58	60		4. 40
21.0 56.83		3. 75	CLAYEY SILT-SILTY CLAY	86-10 0	27-31	40	47	7. 94	4, 54
22 0 79.69		2.41	SANDY SILT-CLAYEY SILT	60-80	31-35	40	43		• .
23, 0 118, 44 24 0 57 83	ů. 76	1. 63	SILTY SAND TO SANDY SILT	40~80	35-39	50	52		
25 0 22,54		4. 56	SILTY CLAY TO CLAY	UNDEF	UNDEF	53	60	8.04	3. 74
		4. 94	CLAY	UNDEF	UNDEF	20	27	3. 01	1.80
24.0 41.60 27.0 36.08		4. 27	SILTY CLAY TO CLAY	UNDEF	UNDEF	35	42	5. 72	3, 36
27.0 36.08 28 0 108 95		4.81	CLAY	UNDEF	UNDEF	33	40	4. 92	3. 76
29.0 54.61		4.64	*SANDY CLAY TO CLAY	UNDEF	UNDEF	100	100		
30 0 47,37		6.11	CLAY	UNDEF	UNDEF	72	74	7. 33	6, 42
31.0 34,83		6.31	CLAY	UNDEF	UNDEF	61	스팅	6. 31	8.08
32 0 25.19		6. 03	CLAY		UNDEF	∓ Ģ	47	4. 71	4, 94
33.0 208, 18		5. 16	CLAY		UNDEF	82	83	2. 61	1. 96
34. 0 525. IS		1. 91 0. 76	SILTY SAND TO SANDY SILT		39-42	82	83		
33. 0 505, 40		0. 57	GRAVELLY SAND TO SAND		> 45	100	100		
36 0 503, 32		0.54	CRAVELLY SAND TO SAND		>45	100	100		
37.0 552.88		0. 44	GRAVELLY SAND TO SAND GRAVELLY SAND TO SAND		>45	100	100		
38. 0 376, 67		0. 72	GRAVELLY SAND TO SAND		245	100	100		
39. 0 450, 74		1. 11	+GRAVELLY SAND TO SAND		42-45	82	62		
40. 0 452, 16		1 15	+GRAVELLY SAND TO SAND		42-45	100	100		
41. 0 430. 45		0.66	GRAVELLY SAND TO SAND		42-45 >45	100	100		
42.0 566,79		0. 58	GRAVELLY SAND TO SAND				100		
43.0 447.61		0, 65	GRAVELLY SAND TO SAND				100		
44. O 90. 37		2. 59	SANDY SILT-CLAYEY SILT		35-39	51	52		
45. 0 35, 35		2, 73	CLAYEY SILT-SILTY CLAY		27-31	16	22	4. 61	• • •
46.0 22.19		2. 97	CLAYEY SILT-SILTY CLAY		27-31	14	19	2. 77	1.62 1.52
47 0 31.95		3. 02	CLAYEY SILT-SILTY CLAY		27-31	ië	22	4. 16	1. 62
46.0 24.63		3. 87	SILTY CLAY TO CLAY		UNDEF	19	25	3. 11	1. 92
49.0 3a.57			SILTY CLAY TO CLAY	UNDEF	UNDEF	33	40	4. 80	3. 22
50.0 41,55 51.0 52.64			SILTY CLAY TO CLAY	UNDEF	UNDEF	40	47	5. 51	5. 74
52.0 67 86		4.63	SILTY CLAY TO CLAY	UNDEF	UNDEF	50	57	7. 11	4. 76
33 0 102, 77			SANDY CLAY TO CLAY		UNDEF	80	85	-	
54.0 54.72		3. 86	ASANDY CLAY TO CLAY		UNDEF	56	63		
55 0 100, 50			CLAY		N11DEE	80	60	7. 35	8. 70
	J. 14 3	5. 51	+SANDY CLAY TO CLAY	UNDEF	UNDEF	100	100		

""" INDICATES "OVERCONSOLIDATED OR CEMENTED MATERIAL"

TYPICAL DATA TABULATION FIGURE 4

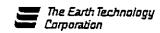


TABLE 2

CPT Refusal

Type	Cause	Indications	Remedial Actions		
Reaction force exceeded	Cumulative rod friction and/or penetration of very hard layer. Most common lim-	Deadweight liftoff, earth anchor yield.	Use heavier equipment (maximum available ~20T trucks).		
	itation encoun- tered in CPT.		Use larger and/or deeper earth anchors		
			Use more efficient friction reducers (enlarged instruments).		
			Drill through hard layer, if known to be thin, and resound		
Push rod buckling	Inadequate lateral support (Qc<10 TSF) to push rods, column buckling. Occasional occurrence in recent, swamp or backwater organic deposits, or silty hydraulic fill.	Bowing of push rods, excessive push rod springback after hard push. Snapping sound and loss of electrical signal.	Set casing through soft layer with CPT equipment, if layer extent is limited (maximum about 25 ft.).		
Rapid change in inclination (>1°/inch)	Deviation due to bedding, gravels, cobbles, rubble or sloping bedrock. Also caused by not leveling CPT equipment before test.	Inclinometer read- ings. Snapping sound and loss of electrical signal.	Use inclinometers. Terminate test and resound.		

summary report table is given in Figure 5. Ideally, the report would include a brief review of the data by the consultant with particular emphasis on any apparent peculiarities or abnormal conditions.

In summary, it can be noted that the ultimate quality of the CPT service received can better be ensured by knowing and working with the consultant/contractor than by contractual specification alone.

TEST			REFUSAL*	МАХІМИН	INSTRUMENT**	ZERO LOAD ERROR (g/fs)	ONE PT.
NUMBER	DATE	DEPTH	CRITERIA	INCLINATION	NO. TYPE	TSF	CHOID EARON
1	10 Aug. 84	84.5	c ·	80	15 ECF P-72	6.5/0.07	+2.0%
2	10 Aug. 84	77.0	С	80	15 ECF P-72	2.0/0.04	+0.5%
3	10 Aug. 84	86.0	С	5 ⁰	15 ECF P-72	0.5/0.02	+0.1%
4	11 Aug. 84	82.5	C	6°	15 ECF P-73	4.0/0.04	+3%
5	10 Aug. 84	51.5	С	19 ⁰	15 ECF P-72	1.0/0.02	-0.03%
6	10 Aug. 84	92.0	c	80	515 ECF P-72	2.0/0.04	±0%
7.	10 Aug. 84	102.0	T	230	515 ECF P-72	1.0/0.02	+0.3%
8	11 Aug. 84	101.5	Ŧ	110	515 ECF P-73	2.0/0.02	-0.6%
9	11 Aug. 84	100.0	T	70	515 ECF P-73	1.0/0.01	+0.3%
. 10	11 Aug. 84	102.5	T	130	515 ECF P-73	3.0/0.03	-0.4%

*C = Reach 20 T Capacity
T = Target Depth Achievers

B = Rod Buckling

**15-15T Load Cell, 15 sq. cm. end area 515-5T Load Cell, 15 sq. cm. end area 510-5T Load Cell, 10 sq. cm. end area

E = Electric

C = Cone

F = Friction

P = Piezometer, Tip

PS= Piezometer, Side

R = Resistivity

T = Temperature

S = Seismic

TYPICAL SUMMARY REPORT TABULATION



4.0 INTERPRETATION OF CPT DATA

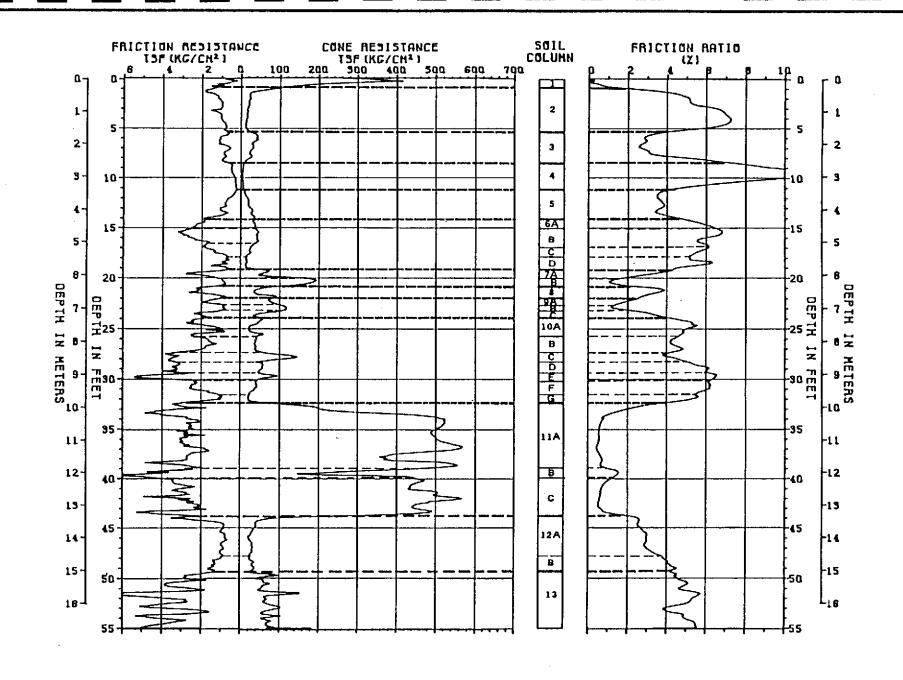
Interpretation of CPT data and application of the results to soil mechanics or foundation design problems is the topic of intense research worldwide. There is no single source of information that provides step by step instructions for CPT data interpretation applicable to all cases. Further, as the CPT provides only a few channels of information while many variables can influence soil behavior, there will never be a single interpretive procedure that meets all project requirements. However, through proper use of the CPT data much valuable insight into site characteristics can be gained.

4.1 Stratigraphy

The first step in CPT interpretation is to get to know the site. Review the geology and then quickly review the CPT data to develop an overview of conditions. Re-examine what information is likely to be critical to the project. Then begin detailed assessment of the individual CPT sounding logs.

The continuous CPT data logs are the primary and most valuable source of information obtained from the test. Data conversions such as interpreted soil properties are always supplemental. Review and mark each channel of each log separately, looking for changes in magnitude and characteristic of the log, such as smoothness or frequency of response. Prepare a stratigraphic sequence based upon compilation of all the changes in magnitude and characteristic evidenced in each log. An example of such a stratigraphic delineation is given in Figure 6. Check for material similarity between different strata by examining where the strata fall on a cross plot of end bearing and friction ratio. Then check for continuity of end bearing magnitude between similar strata found at different depths.

Horizontal continuity of strata is usually of importance to the site characterization process even if only to allow extrapolation of laboratory data across the site. A detailed horizontal profile can be developed from the



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DATE: 15 AUGUST 1984



CONE PENETROMETER TEST

STRATIGRAPHIC DELINEATION

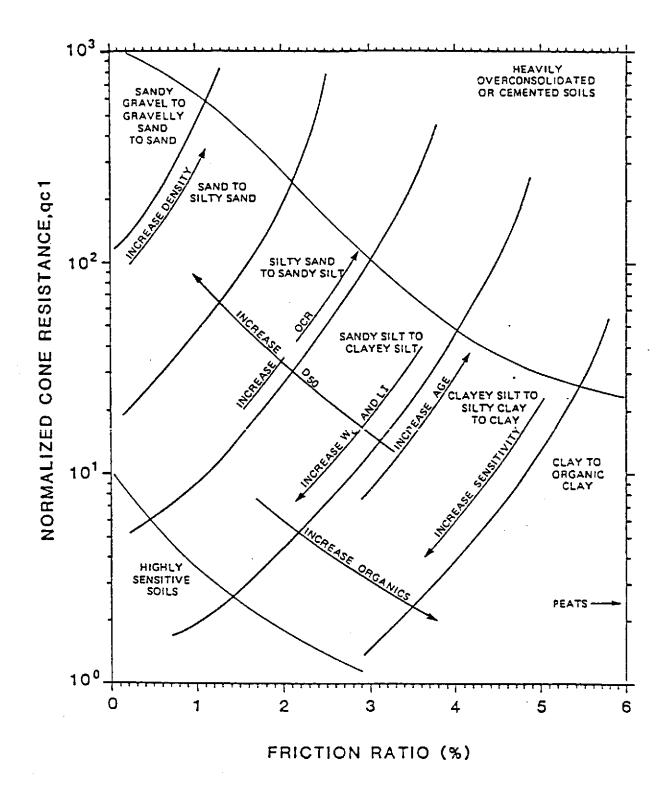
individual logs laid side by side or from a computer-drawn profile series. Establish layer continuity based upon the similarity of individual strata identified within each CPT coupled with an understanding of the likely geologic processes that have affected those strata. Compare strata with those revealed by borings. Note that borings typically provide a very simplified picture of actual conditions and disagreements between borings and CPTs invariably are the result of a poor boring log.

4.2 Soil Properties

After an appropriate layering model is defined, utilize the Soil Behavior Type classification and properties identification charts shown in Figures 7 through 9, or other similar information, to describe each strata. Preliminary engineering analyses should be performed utilizing those initial properties assessments. The analyses will identify problems or critical areas requiring further attention. Select a laboratory testing program to provide site specific correlation for the critical areas and areas of interest as needed.

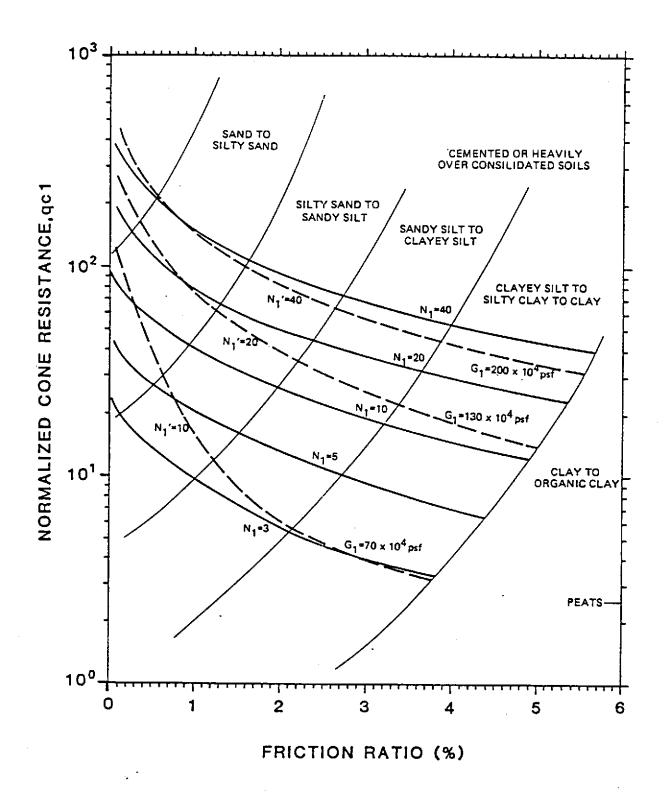
Earth Technology's extensive CPT data base is stored on computer so that the results of laboratory tests can be compiled directly onto the CPT-Soil Behavior Type classification chart. That compilation allows the laboratory results to be applied to any other area of the site having CPT values which fall in that same end bearing and friction ratio zone of the classification chart of Figure 7.

The information contained in the charts of Figures 7 through 9 is most appropriate for geologically young saturated soils. Uncertainties associated with use of the Soil Behavior Type chart are the similar behaviors that can be evidenced by slightly different mixtures of different grain sizes and the effects of underconsolidation or heavy cementation upon the penetrometer readings. Testing in unsaturated or otherwise unusual soil conditions usually requires site— or region—specific correlations until a level of familiarity with those materials is developed.



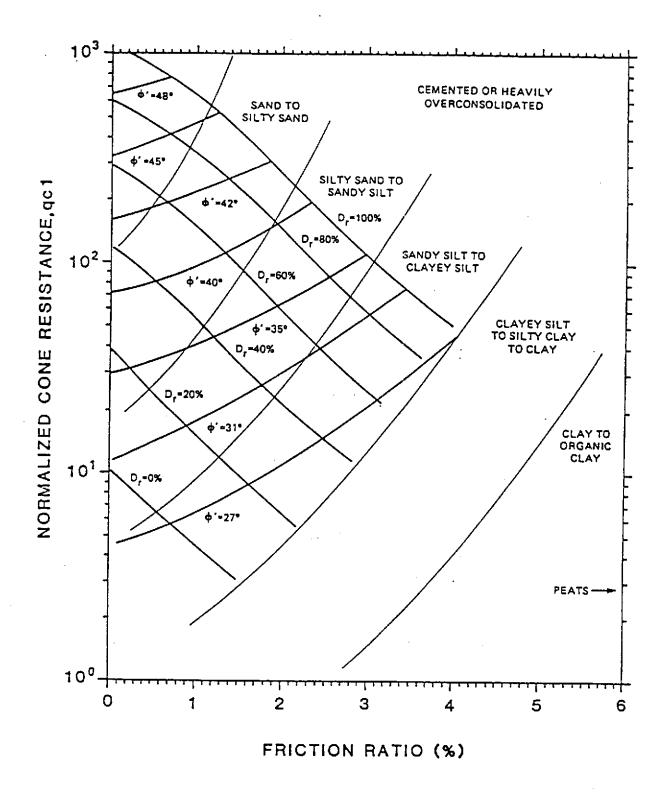
SOIL BEHAVIOR TYPE CLASSIFICATION CHART





EXPANDED SOIL BEHAVIOR TYPE CLASSIFICATION CHART SHOWING OVERBURDEN NORMALIZED EQUIVALENT SPT N VALUES (55% ENERGY) AND FINES-CONTENT ADJUSTED N- VALUES AND MAXIMUM SHEAR MODULI TRENDS





EXPANDED SOIL BEHAVIOR TYPE CLASSIFICATION CHART WITH EQUIVALENT OVERBURDEN NORMALIZED FRICTION ANGLE AND RELATIVE DENSITY TRENDS



The set of classification charts in Figure 7 through 9 have been developed based upon extensive correlation with overburden normalized cone end bearing (9) and friction ratios. Usually it is not necessary to actually carry out such normalization except when precision is important or when data was obtained at shallow (less than 5 feet) or great (greater than 50 feet) depths.

Once soil type and general soil consistency are known, almost any other soil property can at least be estimated. Thus in any project the adequacy of the CPT regional correlation with soil type should be reviewed before attempting any other interpretation. The review of correlation adequacy should focus on prediction of soil behavior and not prediction of arbitrary soil names.

4.3 CPT-SPT

The CPT-Standard Penetration Test (SPT) correlation chart shown in Figure 8 is considered representative of the correlation between CPT and a 55 percent efficient trip hammer SPT used with a liner-sample without liners. Because of the similarity in penetration mechanism between the CPT and SPT, the CPT-SPT correlation need not be modified for regional conditions. However, the correlation can be modified to provide equivalent SPT values appropriate for some other SPT energy efficiency.

Depth normalization can be performed using any of the numerous overburden adjustment relations (C_n) developed over the years. The range of published relations and the lack of any data regarding overburden correction in other than clean sand effectively invalidates claims of greater appropriateness of any one method over another (21).

4.4 Cyclic Strength

The CPT-cyclic strength correlation shown in Figure 8 was developed by Earth Technology following the simplified SPT method (24). The predicted strengths

utilizing this CPT approach have been directly verified by field observation of liquefaction at sites at which CPT measurements were taken (23). This CPT approach provides continuous prediction of fines-content-adjusted equivalent SPT values which can directly be utilized in available SPT-cyclic stress ratio correlations (24).

4.5 Modulus

CPT-Shear Modulus correlations shown in Figure 8 were again based upon SPT-Shear Modulus correlations available in the literature and upon actual CPT versus field and laboratory measured shear moduli (28). However, this data base is not extensive and predicted values should be used with some caution. Numerous other correlations can be found in the referenced literature; however, most such correlations are appropriate only for a single soil type.

4.6 Friction Angle and Relative Density

CPT-equivalent drained friction angle and Relative Density correlations shown in Figure 9 have been based upon data available in the referenced literature and adjusted through limited laboratory correlation (13). Although caution should be used in applying such correlations, the general magnitude of the parameters has been found to agree well with the predicted values. Improvements in such correlations have been slow because of the difficulty in obtaining undisturbed samples of natural and lightly cemented sands and the inevitable inaccuracy associated with laboratory calibration tank-type CPT correlations.

4.7 Undrained Strength

Undisturbed undrained strengths ($S_{\rm uu}$) of clay soils can be estimated using any of the bearing capacity-type relations found in the referenced literature. Such equations generally are of the form:

$$S_{uu} = (q_c - \sigma_t)/N_c$$

where q_C is the cone end bearing, σ_C is the total overburden stress and N_C is a bearing capacity factor. Cautions associated with using this formula are primarily related to degree of drainage existing during the test.

The appropriate bearing capacity factor, $N_{\rm C}$, for use in strength calculations had not been assessed for partially drained or partially saturated clays. However, in fully saturated clays it is generally found that $N_{\rm C}$ ranges between about 9 and 16 with the lower values corresponding to the moderately sensitive clays and the higher values corresponding to the moderately overconsolidated clays. Extremes in either direction have resulted in reported $N_{\rm C}$ values as low as 6 and as high as 25. Earth Technology has recently developed a data base relating $N_{\rm C}$ to both material type and consistency, or to zones of the CPT-Soil Behavior Type classification chart.

Direct use of the CPT sleeve friction values (f_s) can be made as an estimate of the large strain or residual strength (S_{ur}) of clays:

$$S_{nr} = Af_{g}$$

where A is a correlation factor typically assumed equal to 1.0. No data is available relating the proper value of A to soil type and consistency.

The ratio of the undisturbed strength calculated using the end bearing value (S_{uu}) to the large strain strength (S_{ur}) calculated from the sleeve friction can be used to define the sensitivity (S_t) of the soil:

$$S_t = S_{uu}/S_{ur}$$

This definition has been found to provide values which relate well to sensitivities obtained from the field vane test.

4.8 Foundation Design

General procedures for shallow and deep foundation design are contained in the references provided in the following section. Examination of the adequacy of each procedure is beyond the scope of this manual. Those references in general will be found to provide adequate description of uses and limitations of each procedure.

5.0 REFERENCES

There are thousands of papers documenting the various aspects of Cone Penetrometer Testing and interpretation. The following abbreviated list is intended to provide a first step in obtaining the detailed information desired.

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APPENDIX B

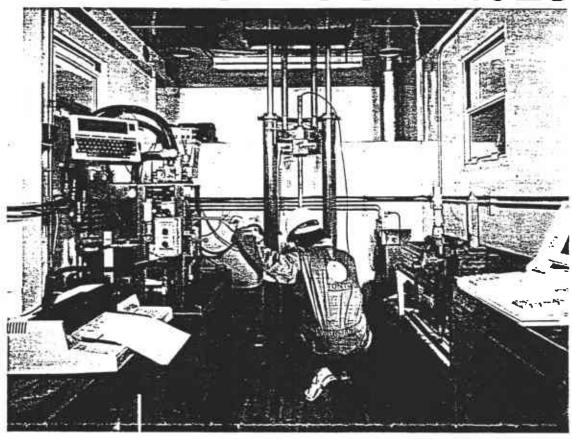
IN-SITU SOIL SAMPLING





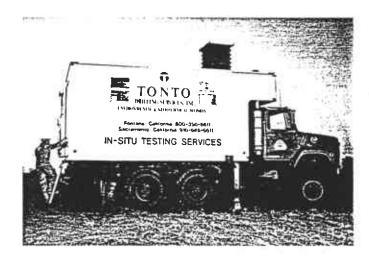
ENVIRONMENTAL AND GEOTECHNICAL DIVISION

INSITU TESTING SERVICES



The cone penetration test is widely accepted as a low cost, non-destructive method of insitu testing for environmental site assessment and geotechnical site investigation. As a logging tool, this technique is unequalled with respect to the delineation of stratigraphy and the nearly continuous, rapid measurement of tip and friction resistance, as well as pore pressure.

Our Cone Rigs are mounted on allwheel-drive trucks which are capable of travel at highway speeds. The hydraulic push system has 20-Ton capacity and data is recorded by an on-board computer.



Data is acquired at a rate of 1 meter/ minute with measurement increments of 5 centimeters. Measurements include tip and friction resistance, pore pressure, inclination and temperature which are printed immediately for inspection by the engineer/geologist. A report-ready plot of this data is also available.

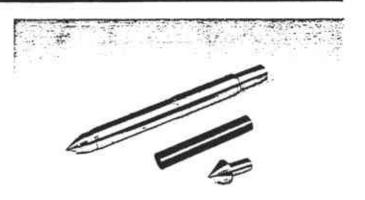
Available groundwater sampling devices include the HydroPunch® and BAT® samplers. These tools are pushed to a target depth and a sample is retrieved from discrete intervals. Target depths defined by analysis of the real time CPT printout and plots greatly increase sampling efficiency. Other sampling techniques are available.

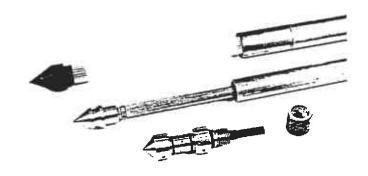
We have the capabilities of retrieving groundwater and soil samples for testing and visual verification of subsurface conditions.

Soil samples suitable for chemical analysis are retrieved with a retractable cone tipped sampler.

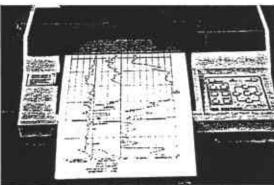
Several alternatives are available for hole abandonment at environmentally sensitive sites.

With our highly trained crews and state-of-the-art equipment we can offer you these services at very competitive rates.











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``Geared-up for the 90's''

APPENDIX C

GROUNDWATER SAMPLING
HYDROPUNCH AND BAT SYSTEM



Groundwater Sampling without Wells

The HydroPunch™ drastically reduces time and money spent on groundwater monitoring site assessments, by collecting samples without wells. Data can be used to determine vertical and horizontal extent of contamination, and to accurately quantify pollutant concentration.

Samples in as little as one hour

The HydroPunch (U.S. Patent No. 4,669,554) is easily used with cone penetrometer or conventional drilling equipment. It collects up to 500 ml of groundwater at the desired depth in unconsolidated soil, and under many conditions can be used to sample multiple waterbearing zones in one operation. The HydroPunch can be visualized as working like a "driven" bailer.

Save 70% or more on site assessment costs

Extremely cost-effective, the HydroPunch has proven in field use to cost as little as 1/10 the price of drilling, casing, and developing a conventional well. The HydroPunch can also help determine optimum location for dedicated wells when they are required. More effective placement can minimize the number of permanent wells needed, providing long-term savings.

High-quality samples for accurate assessments

Samples are unaltered and uncontaminated by drilling fluids or cuttings. All-stainless and Teflon® construction makes the unit chemically inert, preventing contamination. In use, the HydroPunch is driven to the desired depth and then partially withdrawn, opening the inlet and isolating the collection zone from layers above and below. Replaceable inlet screen cartridges keep soil materials from entering the sample chamber.

HydroPunch samples are consistent with requirements for all priority pollutants, unlike indirect site assessment techniques (i.e., soil gas sampling or geophysical monitoring). Samples are not affected by changes in soil type or other complicating

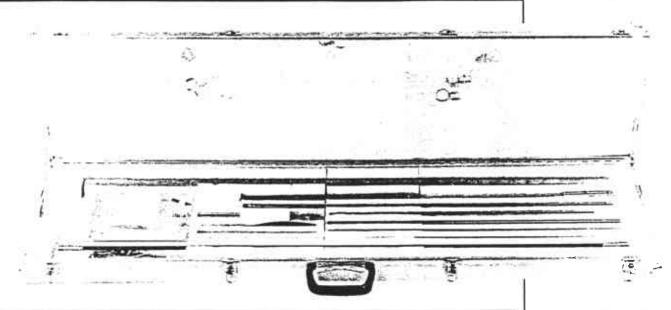
factors. Easy field cleaning expedites repetitive sampling.

Environmentally safe

The HydroPunch can be operated with minimal disturbance to environmentally sensitive areas. There's no need to dispose of well development water, or of contaminated drill cuttings when used with a cone rig. The technique is unobtrusive and won't interfere with normal site operations.

All-stainless and Teflon® construction provides strength, durability, and accurate samples uncontaminated by the testing procedure.

Environmental



Specifications:

The HydroPunchTM is equipped with an "AW" box thread. Any sub-adapter or drive rod used with HydroPunch must have a minimum of 9/16" inside diameter by 4" deep above top of HydroPunch to allow clearance for top check. A number of adapters are available, allowing use of the HydroPunch with different types of drive rods.

The basic kit (shown above) includes one HydroPunch with barbed point in a sturdy, protective carrying case. The kit comes complete with water sample discharge device (w/Teflon® tubing and stopcock), cleaning brush set, extra O-ring and screen sets, extra stainless steel check balls, and all other accessories needed for use.

Maximum diameter: 1.75" Length: Closed—64.50" Open—76.50"

Weight (HydroPunch only): 24 lbs. Shipping weight: 44 lbs.

Sample volume: 500 ml (nominal)

Guidelines for use:

General applications

The HydroPunch is a groundwater sampling tool designed to be pushed or driven to the desired depth for sample collection. It is manufactured for durable performance, with rugged construction of stainless steel and Teflon*. Following a few basic guidelines will maximize the usable life of your HydroPunch.

In general, the HydroPunch can be pushed or driven into position in the same types of formations suitable for a standard 2" split barrel (spoon) soil sampler. Suitable geologic materials include unconsolidated clays, silts, sands, and fine gravels. Driving a split barrel sampler immediately above the desired HydroPunch sampling zone is helpful. This provides an estimate of soil permeability, and predicts the formation's resistance to driving the HydroPunch.

Hydrologic considerations

The HydroPunch fills using the aquifer's hydrostatic pressure, similar to the way a bailer fills; thus the formation thickness and yield determine the fill rate. The sample inlet area of the HydroPunch must be in hydraulic contact with a water-bearing zone to collect a sample. Because the sample chamber is above the inlet, the HydroPunch point must be driven to a minimum of 5 ft. below the static water level so that hydrostatic pressure is high enough to assure normal fill times and adequate sample volumes.

Complete HydroPunch kit in heavy-duty carrying case



Typical application using HydroPunch with cone penetrometer equipment.



I Floating Layer and Ground Water Sampler

HydroPunch II enables drill rig operators to locate, measure, and sample ground water and floating layers of gasoline and other hydrocarbons... rapidly and economically, without drilling wells.

Quality samples in an hour or less—at much lower cost

HydroPunch II is a breakthrough in site assessment and hydrocarbon detection technology. Sampling is so rapid, you can have reliable results in hours, not days or weeks. Unlike indirect survey methods, it delivers actual samples of ground water and floating hydrocarbons—not indirect readings requiring interpretation.

Ground water collected with the HydroPunch II is consistent with monitoring requirements for all priority pollutants. Floating layer samples accurately identify and estimate the thickness of lighter-than-water hydrocarbons (i.e. gasoline, fuel oil, solvents).

Better yet, HydroPunch II collects these samples at as little as 1/10 the cost of drilling, casing, and developing conventional monitoring wells. Replaceable screens make field cleaning for multiple grabs fast and easy. There's minimal environmental disturbance—with no permanent installation or well development water.

How the HydroPunch II works

The sampling tool is driven to the desired depth in unconsolidated formations. Preliminary grabs or other information help determine the approximate sampling depth. An auger or split barrel sampler is often used to provide a "pilot hole" to the area just above the sampling zone.

For floating layer applications

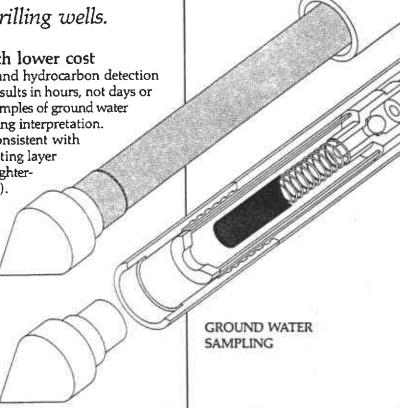
After inserting the polypropylene screen and attaching the point, the HydroPunch II is fixed to the casing, lowered through the pilot hole, and driven to the proper depth. The tool is then withdrawn approximately 48", leaving the point in the ground and exposing the screen so that ground water and floating product can enter.

A 1" O.D. Teflon[®] bailer is lowered through the hollow interiors of the drive casing and body of the HydroPunch II to collect the sample.

For ground water applications

Insert the ball check valves and stainless steel screen, then attach the point. The tool is driven to the proper zone (at least 5 foot submergence for ground water sampling), then withdrawn approximately 18" to expose the inlet screen. The interior fills with water. When the HydroPunch II is recovered, the check balls keep the sample from draining. The point remains in the ground.

Discharge to sample containers is easy with the supplied stopcock. Throughout the process, the sample contacts only stainless steel and Teflon.



HYDROCARBON

SAMPLING

Specifications

Dimensions:

Length: 55.5" overall (closed)

O.D.: 2" (nominal)

Weight: 24 lbs. (approximate-varies

with configuration)

Sample volume: 1 liter (ground water) unlimited (floating layer)

Materials:

Body and fittings: 304 stainless steel Drive shoes: hardened carbon steel (std.)

stainless steel (optional) Adapters: AW drill rod (ground water)

- carbon steel EW casing (floating layer)

— carbon steel

Check balls: Teflon® or stainless steel

Screens: 5.25" long 125-micron 120 mesh stainless steel (ground water) 48" long x 1.375" O.D. polypropylene (floating layer)

Replaceable points: lead-free carbon steel

Basic kit:

Body, drive shoes, AW and EW adapters, check ball assemblies, cleaning brush set, 10 replaceable points, 5 stainless steel screens, 5 polypropylene screens, in a protective carrying case

Replaceable supplies: Points (10/cs) Stainless steel screens (10/cs) Polypropylene screens (10/cs.)



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