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SUMMARY OF SITE CONDITIONS ELECTRO-COATINGS, INC. FACILITY 1401 AND 1421 PARK AVENUE EMERYVILLE, CALIFORNIA

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In 1963, Electro-Coatings, Incorporated (ECI) purchased the assets of a metal plating facility at 1401 and 1421 Park Avenue, Emeryville, California (Figure 1). Prior to 1963, Industrial Hard Chrome Plating Corporation operated the metal plating facility at the 1401 and 1421 Park Avenue address.

This report is in response to the 26 August 1994 letter from the Regional Water Quality Control Board (RWQCB). This letter outlines six specific items for inclusion in a technical report:

- Discussion of investigations completed to date and inclusion of all relevant data.
- 2. Work plan to define the vertical and lateral extent of chlorinated solvents and chromium in soil and groundwater.
- 3. Proposed interim remedial measures to prevent the migration of the plumes to adjacent sites.
- 4. Documentation of the removal of the underground storage tanks.
- 5. Proposed remedial action to be implemented at the site to prevent migration and impact of the plumes.
- 6. Determination of potential health risks posed by the presence of the contaminant plumes to on-site workers and to occupants of adjacent sites.

1.1 OBJECTIVES

The objective of this report is to identify reasonable remediation actions based on current site conditions, risk to human health and the environment, and current and future use of the property including the soil and groundwater. While this report is in response to requests from the RWQCB, ECI has an ongoing program to identify and evaluate reasonable and appropriate remediation alternatives.

1.2 STRUCTURE OF THIS REPORT

This report is organized to address each of the above items described in the 26 August 1994 letter from the RWQCB:

- Section 2 describes the site and history of operations, summarizes environmental actions to date, and briefly describes the regional setting.
- Section 3 describes potential sources of chemicals to soil and groundwater, data collected to date, and interprets current environmental conditions at the site.
- Section 4 discusses exposure pathways and potential risk, but this section is not a formal risk assessment.
- Section 5 recommends a plan of action to mitigate further contribution of chemicals to soil and groundwater.

2.1 SITE DESCRIPTION

The 1.0 acre Electro-Coatings, Inc. (ECI) facility consists of two parcels of property. Four buildings are located on the premises. The facility began operation in 1952 under the ownership of Industrial Hard Chrome Plating Corporation. In 1963 the business was purchased by ECI. One of the parcels is owned by ECI and one is leased. The buildings abuts property lines along its northern and eastern borders and is surrounded by parking and paved areas to the south and west.

The facility is bounded to the north by Park Avenue. To the east is a dirt alley with buried railroad tracks which is reportedly scheduled to be improved in order to extend Holden Street from Park Avenue to 40th Street. A lumber yard and parking area are south and immediately adjacent to the facility. About 140 feet further south is the Southern Pacific Railroad right of way which is being reconstructed as an extension of 40th Street. A clothing outlet and a crematorium are immediately adjacent to the west of the facility. About 80 feet further west is Horton Street.

2.2 HISTORY OF SITE OPERATIONS

Chrome plating operations began at the site in 1952 by Industrial Hard Chrome Plating Corporation. Electroless nickel plating began in the late 1950s. In 1963, ECI purchased the assets of the facility. ECI ceased hard chrome plating in 1989 and now only performs nickel plating. The remaining nickel plating operation is slated for relocation sometime in the near future. All plating operations have and continue to be performed in vats constructed above the ground surface. All nickel plating operations have been performed on the 1421 Park Avenue parcel and all chrome plating operations have been performed on the 1401 Park Avenue parcel.

Trichloroethylene (TCE) was used at the facility to vapor degrease metal parts prior to nickel plating. Trichloroethylene (TCE) was in use prior to purchase by ECI in 1963. TCE use was discontinued in 1973 and 1,1,1-trichloroethane (TCA) was substituted in the vapor degreaser. Vapor degreasing was discontinued in 1992. Today, metal degreasing is performed using a liquid alkaline soak process. All metal degreasing operations were performed in the southwest portion of the facility on the 1421 Park Avenue parcel.

2.3 HISTORY OF ENVIRONMENTAL INVESTIGATIONS

In 1977 the California Regional Water Quality Control Board issued a Cleanup and Abatement Order (No. 77-011) which required ECI to investigate groundwater contamination emanating from the site. In response, ECI conducted a well survey and installed seven monitoring wells (wells MW-1, MW-2, MW-3A, MW-3B, MW-3C, MW-4 and MW-5) in 1977. The seven wells were sampled and analyzed for total chromium and hexavalent chromium in August 1977.

In 1978 three additional monitor wells (MW-6, MW-7 and MW-8) were installed. In 1980 five more monitor wells (MW-9, MW-10, MW-11, MW-12 and MW-13) were installed. Soil samples were also collected for analysis from the borings for these wells. All 15 wells were sampled in 1981, some more than once, and analyzed for total and hexavalent chromium.

In 1982 five additional monitor wells (MW-14, MW-15, MW-16, MW-17 and MW-18) were installed and one soil boring (B1) was conducted. Soil samples were also collected from the boring for monitor well 14. No data are available from the reported boring B1. In 1983 four additional wells (MW-18A, MW-19, MW-20 and MW-21) were installed and two soil borings, 22 and 23 were performed. In 1985, eight more soil borings were drilled and sampled, borings 24 through 31. Also, in 1985, 15 of the 24 wells were sampled and analyzed for total and hexavalent chromium and halogenated volatile compounds. It was during this round of sampling that TCE was encountered in the groundwater.

In 1991, 21 of the 24 monitor wells were samples. Three could not be located. Groundwater was analyzed for total chromium, hexavalent chromium and VOCs. Again

in 1991, and in 1994, selected wells were sample and analyzed for total chromium, hexavalent chromium, and VOCs.

Water sample analyses from all sampling events are summarized in Tables 3 through 8. Soil sample analyses are summarized in Table 2.

2.4 MUNICIPAL SETTING

The ECI facility is located in a highly industrialized area north of Interstate 580-Highway 24 and just east of Interstate 80. The facility is roughly 2,500 feet to the east of San Francisco Bay. The area was developed in early 1900s and various areas became industrialized beginning in the 1920s. The area is currently in transition.

On 7 September 1994, the City of Emeryville Zoning Ordinance Revision Committee conducted a public workshop as part of the City's effort to update the land use policies and design guidelines for the creation of a Revitalization Strategy/Zoning Overlay District for five blocks along Park Avenue. The ECI facility at 1401 and 1421 Park Avenue are included in this district.

Since 7 September, ECI has met with the City's Community Development staff, including the Redevelopment Director, Mr. Kofi Bonner. ECI has submitted copies of all existing technical reports describing environmental conditions to the City. The City has expressed the intent to preserve the character of this district while allowing new uses by non-industrial commercial businesses. Many buildings in the area have served their useful economic life for industry and are now in transition to other uses. The City wants to find ways to address the environmental issues so that development can occur without posing unacceptable risk to humans. Mr. Bonner is willing to meet with the County of Alameda Department of Health Services and the RWQCB and the property owners to discuss possible ways to address the environmental issues of this site as it is converted to non-industrial use.

2.5 Hydrogeologic Setting

The site is generally underlain by clays and silty clays to roughly 15 feet. Between about 15 to 30 feet, the sediments become coarser, consisting of layers of sand and gravel with varying silt and clay content. This coarser zone comprises the water bearing unit and shallow aquifer. While variations exist at each boring location, the lithology and hydrogeology of the site can be generally described by the boring logs and cross-sections (Appendix C). Monitor well construction details are summarized in Table 1.

The shallow aquifer is semi-confined to confined, with a potentiometric surface roughly 6 to 9 feet below ground surface, and a hydraulic gradient to the west and northwest. An aquifer test performed in the aquifer yielded permeability of 28 feet³/day/foot² (units can also be expressed as feet/day, which is not indicative of groundwater velocity). This permeability is typical for sand and gravel aquifers.

Three monitor wells (3A, 18A, 20) are constructed below the shallow aquifer. Underlying the shallow aquifer is an extensive blue clay with small lenses of sand and gravel. These three monitor wells are screened in deeper water bearing units and data from the deeper wells are not used to interpret conditions in the shallow aquifer. The potentiometric surface in the deeper wells 3A and 18A reveal a slight vertically downward hydraulic head, whereas, monitor well 20 shows an upward vertical head of about 3 feet above the shallow potentiometric surface. This upward head indicates that contaminants are not likely to migrate below the shallow aquifer under natural conditions.

3.1 POTENTIAL SOURCES

There are three identified potential sources of contaminants to soil and/or groundwater. The following generally describes each potential source. Other potential sources of compounds to soil at the facility include general operations and maintenance, but based upon the data, these activities are not considered at this time to be significant. Background or upgradient activities by others is the only other known general source of contaminants to groundwater beneath the ECI facility. The remainder of this section describes historical environmental data and site conditions as described by the most recent set of data.

3.1.1 CHROMIUM WASTE STORAGE AREA

The southeast corner of the facility directly behind the 1401 Park Avenue building is identified as the likely source of contaminants to soil and groundwater. It is labeled as "chromium waste storage area" on Figures 2 through 8. A concrete vault or tank was located there and it was reported leaking sometime in either 1974 or 1977, the record is unclear. In 1977, the storage area was completely reconstructed with double lined concrete and steel tank. The southeast corner of the facility directly behind the 1401 Park Avenue building is identified as the likely source of contaminant to soil and groundwater. It is labeled "chromium waste storage area" on Figures 2 through 8. The groundwater quality data (described below) indicate that elevated chromium concentrations probably emanated from this area. The chromium waste storage area is no longer used, nor available for use.

3.1.2 VAPOR DEGREASER AND SOLVENT ABOVE GROUND STORAGE TANK

Metal parts for nickel plating were degreased in an operation performed in the southwest corner of the facility. The vapor degreaser was located inside the southwest corner of the 1421 Park Avenue building and an above ground tank located outside the building was used to gravity feed the solvent to the degreaser. Until 1973, TCE was the solvent used in the vapor degreasing operation, at which time 1,1,1-TCA was substituted as the degreasing chemical. In 1992, vapor degreasing was discontinued at the facility. Although no leaks or spills are reported in the record, elevated levels of TCE, above background, may have emanated from this area, as this is the only known potential on-site source.

3.1.3 GASOLINE UNDERGROUND STORAGE TANK

A 500 gallon gasoline underground storage tank (UST) is reportedly located to the west of the building at 1421 Park Avenue. No investigation has been performed to date to either locate or assess the integrity of this UST.

3.1.4 BACKGROUND GROUNDWATER QUALITY

While this report does not detail current or historical industrial activities in the area upgradient from the ECI facility, the long term industrialization of this area is known. Groundwater quality in the region has been affected as a consequence of industrialization of the area that began in the early part of this century. Groundwater background, or upgradient, to the ECI facility is to the east, southeast, and northeast and is described by monitor wells MW-1, 2, 8, and 21 (see Figure 2 for locations).

These wells show that background groundwater contains detectable concentrations of chromium and various chlorinated solvents. Although no statistical evaluation of upgradient water quality has been performed, we estimate that upgradient shallow groundwater quality can be generally described as containing up to:

1 μg/l 1,1-DCE

800 μg/l trans 1,2-DCE

• 2 μg/l 1,1-DCA

• 100 (MW-8) to 2,200 (MW-21) μg/l TCE

• 110 μg/l TCA

- 250 (MW-21) μg/l PCE
- 70 μg/l chromium

It is noted that none of the ECI wells have been analyzed for petroleum products and there is no reported use of perchloroethylene (PCE) at the facility.

3.2 SOIL QUALITY

Soil samples have been analyzed from 15 soil borings shown in Figure 3. Table 2 summarizes the analytical results. Most of the borings were located south of the ECI building, some near the former chromium waste storage area. Borings 9 through 14 are associated with monitor wells with the same identification. Soil has been analyzed for total and hexavalent chromium, but not for solvents nor petroleum products. The data show that background levels of total chromium ranges from about 30 to 60 mg/kg; this is common for soils derived from the Franciscan complex of rocks comprising the San Francisco Bay Area. Hexavalent chromium is not considered an ambient form of chromium and was not detected in soil samples interpreted as representing background conditions.

Some of the soil borings extending 9 feet or more below ground surface (bgs) show an increase in total chromium with depth, notably borings 13, 27, 30, and 31. Because the potentiometric surface of the shallow groundwater is between 6 and 9 feet bgs, this increase probably results from chromium migrating in groundwater.

Soil borings 24 and 25 reveal levels of chromium greater than 1,000 mg/kg in near surface soils with background levels at depth. Because borings 24 and 25 are located east of the facility and off the property, it is not known if this chromium is attributable to ECI. The soil data show that chromium concentrations decrease to background levels by 9 feet bgs in boring 24, and by 6 feet bgs in boring 25, and that background concentrations are maintained to total sampling depth of 11.5 feet bgs. These data are not indicative of a source of chromium to groundwater.

In contrast, soil boring 22, located near the chromium waste storage area, shows that chromium concentrations attenuate to 482 mg/kg at 8.5 feet bgs, which is above background levels. The highest level of chromium in soil below 5 feet bgs, was contained in soil boring 23 at 980 mg/kg at 8 feet bgs. These data are indicative of a source area, which has been consistently described as located in the southeast corner of the facility. Other borings in this area also reveal chromium above background levels in some samples.

3.3 GROUNDWATER QUALITY

Groundwater samples have been collected at various times from 24 monitor wells installed by ECI on and off-site. Tables 3 through 8 summarize 17 years of analytical results beginning with monitor well installation in 1977 and continuing through 1994. Groundwater, when sampled, has been consistently analyzed for total chromium and hexavalent chromium. In 1977 and 1981, selected wells were also analyzed for the metals arsenic, iron, nickel, and zinc, and also pH. In 1985, 1991, and 1994 groundwater from most of the wells (fewer wells in 1994) was analyzed for purgeable halocarbons, with results tabulated for the more common chlorinated compounds 1,1-DCE, trans 1,2-DCE, 1,1-DCA, TCE, TCA, methylene chloride, and vinyl chloride. The analytical results for chromium and TCE are interpreted graphically in Figures 4 through 8.

3.3.1 CHROMIUM IN GROUNDWATER

Figure 4 plots the total chromium concentrations at each monitoring well location for the years 1981, 1985, 1991, and 1994. Figures 5 and 6 depict the distribution of total chromium in shallow groundwater in 1981 and 1991. Several wells recorded highest concentrations in 1985, but as suggested in an earlier technical summary report, it is possible that samples were not filtered in the field. Nonetheless, several important conditions can be interpreted from the data and figures:

• The magnitude, or highest concentrations of the chromium plume, is decreasing over time, from 880,000 μg/l at MW-5 in 1981 to 510,000 μg/l at MW-13 in 1991.

- The size of the chromium plume is increasing over time to the west to northwest and parallel with the direction of the groundwater gradient.
- The center, or most concentrated portion of the chromium plume, is migrating at a rate slower than the less concentrated portions of the plume.
- Significant contributions of chromium to groundwater stopped sometime ago, possibly before 1981) because the center, or most concentrated portions of the plume, is about 100 feet downgradient of the probable chromium source area in 1981 and 200 feet in 1991.
- The data support the source of chromium to groundwater as located near the southeast corner of the ECI building, based on a 10 feet/year migration of the plume center (comparing 1981 and 1991 figures).
- The plume and some background wells exceed drinking water criteria based upon the California maximum contaminant level (MCL) for chromium in drinking water level is 50 μg/l.
- Groundwater below the shallow aquifer contains detectable, but much lower, levels of chromium. It appears that the 1,300 μg/l chromium detected in deep well 20 in 1983 may have been a consequence of drilling because groundwater from this well was non detect for chromium in 1985; MW-20 was installed in 1983.

3.3.2 OTHER METALS IN GROUNDWATER

Detectable levels of arsenic, iron, nickel, and zinc were found in some of the wells sampled and analyzed for these metals. Most of the detected concentrations were near the detection limit. The MCL for arsenic is $50 \mu g/l$ and for nickel is $100 \mu g/l$. Primary MCLs have not been promulgated for iron and zinc, secondary drinking water standards are $300 \mu g/l$ and $5{,}000 \mu g/l$, respectively. From the data, only MW-5 had iron and nickel above standards, and only MW-11 had arsenic above standards.

3.3.3 CHLORINATED HYDROCARBONS IN GROUNDWATER

Purgeable halocarbons, which includes chlorinated solvents, were analyzed in selected wells in 1985, 1991, and 1994. Tables 7 and 8 summarize these data. Figure 7 plots TCE concentrations at each location for the years 1985, 1991, and 1994. Figures 8 and 9 depict the distribution of TCE in shallow groundwater in 1985 and 1991.

ECI reports using only the solvents TCE and 1,1,1-TCA at the facility. These solvents were used to degrease metal parts prior to plating. It is recognized that vinyl chloride and some forms of DCE and DCA are degradation products of TCE and TCA, and that TCE (and possibly TCA) is a degradation product of PCE. Because all of these compounds have been detected in ECI wells that monitor upgradient groundwater (see section 3.1.4), ECI is not the only source of these compounds to groundwater. While PCE is also detected in ECI wells, it is highest in background wells MW-1 (221 μg/l) in the northeast corner and MW-21 (380 μg/l) beyond the southern property line. Because PCE has no documented use at this facility and because background wells contain the highest concentrations, ECI is not interpreted to be a source of PCE.

The data and figures do show concentrations of TCE and TCA and their related degradation products DCE and DCA and vinyl chloride, above background levels downgradient from the southwest corner of the facility. This area housed the vapor degreaser with an outside above ground storage tank used to gravity feed TCE and later TCA. Several important conditions can be interpreted from the data and figures:

- The size of the TCE plume is increasing with time to the west to northwest and parallel with the direction of the groundwater gradient.
- The magnitude, or highest concentrations of the TCE plume, appears to be increasing over time, however, this may be due to the center of plume not intercepted by wells in 1985.
- The data support the source of TCE and TCA to groundwater as located near the southwest corner of the facility.
- The California maximum contaminant level (MCL) for TCE, TCA, and PCE in drinking water is 5 µg/l, 200 µg/l, and 5 µg/l, respectively. The

plume and most background wells exceed drinking water criteria for TCE and PCE. Only wells MW-10 and MW-16 exceed the MCL for TCA.

Groundwater below the shallow aquifer has only been analyzed from well
 18A, which contained low levels of TCE and methylene chloride in 1985.

3.4 SUMMARY OF CURRENT ENVIRONMENTAL CONDITIONS

The most recent comprehensive set of data describing groundwater is from 1991. These data are graphically depicted for total chromium in Figure 6 and for TCE in Figure 9. Both figures show that the most concentrated portions of the plumes are still located almost entirely on the ECI property. The figures also show the plume to extend to the west and northwest, parallel with groundwater flow direction Background levels for both total chromium and TCE appear to be reached about 100 to 200 feet beyond MW-6. Based on the limited 1994 data, chromium and TCE concentrations appear to be decreasing. The plume does not extend into any areas known to be a source of drinking water supply.

Data describing soil were collected from 1980 to 1985. Because operations at the facility have remained similar or have decreased over the last several years, the extent of soil with chromium and TCE is not expected to be much different than described by the existing data.

The purpose of this section is to generally describe the potential for exposure to contaminants at the ECI facility, notably chromium and TCE. Routes of human exposure to contaminated soil include dermal absorption, inhalation of particulates, and incidental ingestion. Routes of human exposure to contaminated groundwater include dermal absorption, inhalation of volatiles and water droplets, and ingestion of contaminated water supplies. Some of the following discussion is based on the *Site Inspection Prioritization* report dated 20 September 1993 and generated by Bechtel for the U.S. Environmental Protection Agency for the purpose of assessing "the relative threat associated with actual or potential releases of hazardous substances at the site."

EXPOSURE TO SOIL

Currently, the site is covered by either buildings or pavement; and the surrounding area is similarly developed. The known or inferred locations of contaminated soil are either beneath buildings, beneath pavement, or beneath several feet of uncontaminated soil. Under current conditions, exposure of on-site workers or workers at adjacent sites to contaminated subsurface soils is unlikely.

The one exception is the chromium detected in shallow soil to the east along the buried railroad tracks (soil borings 24 and 25). Because of the reported planned construction in this area, ECI has notified Catellus Development of the presence of elevated levels of chromium found in soil borings in 1985.

EXPOSURE TO GROUNDWATER

The water bearing unit at the site begins at about 15 feet below ground surface. The only routes of exposure to this groundwater would be via supply wells or excavation. Surveys of groundwater supply wells have been performed by previous consultants to ECI and by Bechtel, the contractor for the EPA in 1993. None of these surveys revealed groundwater supply wells that would be potentially impacted by the plume emanating from the ECI

facility. Under current conditions, exposure of on-site workers or workers at adjacent sites to shallow groundwater is unlikely.

POTENTIAL FUTURE EXPOSURE SCENARIOS

There are potential future scenarios that may result in exposure to contaminated media. If subsurface soils containing concentrations of contaminants above background levels are excavated, potential exposure pathways may include dermal adsorption, inhalation of particulates, and incidental ingestion. If excavation continues into the groundwater, then potential exposures may include inhalation and dermal adsorption. Use of the groundwater as source of drinking water is unlikely because the EBMUD supplies drinking water to the Emeryville area by importing surface water from the Central Valley of California, over 100 miles to the east.

This sections provides recommendations for action in response to known or inferred conditions regarding chromium and TCE in soil and groundwater at the site. These recommendations should be further discussed and developed, and then prioritized based on perceived need, regulatory criteria and request, cost effectiveness, financial abilities of ECI, future use, and potential effects on the redevelopment plan now underway by the City of Emeryville. This section is intended to serve as an outline for a work plan to be further developed following discussions with the regulatory agencies.

5.1 KNOWN AND POTENTIAL SOURCE AREAS

There are three known or potential sources that should be considered for investigation and remediation. They are the chromium waste storage area, the vapor degreasing operation and TCE above ground storage tank, and the gasoline UST.

- The chromium waste storage area should be further investigated. Soil containing elevated levels of chromium should be removed. Contaminated soil can as appropriate, be subject to building integrity serve as a secondary and long term source of compounds to groundwater long after the primary source has been removed. A cleanup level for chromium in soil should be developed one te extent is known.
- The vapor degreasing area should be investigated. Soil with elevated levels of TCE, or other contaminants, should be remediated, as appropriate subject to building integrity. Contaminated soil can serve as a secondary and long term source of compounds to groundwater long after the primary source has been removed. A cleanup level for TCE and other chlorinated hydrocarbons should be developed once the extent is known.

• The gasoline UST should be located and records describing the UST should be located and reviewed. As appropriate, and presuming the UST is not slated for future use, the UST should be removed or abandoned in place by filling with concrete. Soil samples and UST closure should be performed consistent with the RWQCB and County guidelines.

5.2 OTHER AREAS WITH CONTAMINANTS IN SOIL

Only one other area exists with known concentrations of chromium above background that is not associated with a known or potential source area. This is the shallow soil east of the facility adjacent to the buried railroad tracks. While it is not clear if ECI activities are responsible for this chromium, ECI is aware of its existence. As described above, ECI has recently notified Catellus Development of the presence of elevated levels of chromium. We recommend that ECI confirm this notification in writing and receive notification that this issue is being appropriately addressed if and when road construction is contemplated.

5.3 GROUNDWATER

One complete round of groundwater samples should be collected from all monitor wells except 3C. MW-3C is screened in a zone already screened by immediately adjacent well MW-3B. Groundwater from these wells should be analyzed for total chromium and purgeable halocarbons. The samples for chromium should be filtered in the field to exclude artifacts introduced by suspended sediment. In addition, an attempt should be made to determine which previous groundwater samples were submitted filtered or unfiltered for analysis. The results of the recommended groundwater sampling should be evaluated along with previous data for the purpose of developing an annual groundwater sampling program. Annual sampling is considered adequate based on a 17 year history of data and the reasonably predictable behavior of the groundwater plume.

Groundwater remediation is not recommended at this time because:

Groundwater is not a source of drinking water downgradient of the site.

- There are no actual beneficial uses being impacted by the groundwater plume.
- EBMUD supplies all water to Emeryville, and exposure to, and use of, the groundwater is unlikely.
- There is a regional groundwater contamination problem that should be approached comprehensively with costs allocated accordingly.
- The only identifiable probable use of the groundwater is as discharge to the bay. Concentrations of compounds at this point of discharge are not yet predicted, but based on current data and rates of contaminant attenuation, the discharged concentrations are likely to be much lower than those detected at the ECI facility.

Bechtel Environmental, Inc., Site Inspection Prioritization, September 20, 1993.

- American Environmental Management Corporation, Groundwater Monitoring Report for Electro-Coatings, Inc., Emeryville, California, January 27, 1992, pp 1-4, 12-17, 19-22.
- Kleinfelder, Inc., Data Summary Report, Electro-Coatings Facility, 1421 Park Avenue, Emeryville, California, April 25, 1991, Tables 1,3,5,7, Plates 2-9.
- Poling, Kathleen U., Electro-Coatings, Inc., Letter to Susan Hugo, county of Alameda, Department of Environmental Health Hazardous Materials Division, August 31, 1994.

Table I WELL SUMMARY

Well	Date	Depth	Depth	TOC	Water	Screen	Depth of	Well Dia./	
No.	of	of Well	To Water	Elevation	Elevation	Interval	Well Seal	Slot Size	Remarks
	Installation	(feet)	5 November 1991		5 November 1991	(feet)	(feet)	(inch)	from 5 November 1991 survey
			(feet)	(feet)	(feet)				
				AUGALIA-O	7////				
1	08/18/77	29	6.4	15.19	8.79	21-29	14	4/na	New locking well cap, removed tubing from well
2	08/18/77	21	na	na	na	14-21	13	1.5/na	Could not focate
3A	08/15/77	(65)	7.75	16.1	8.35	57-61	55	1.5/na	7" well cover installed
3B	08/15/77	18	7.1	16.3	9.2	16-18	15	1.5/na	7" well cover installed
3C	08/15/77	15 🗭	6.825	16.21	9.38	11-14	10	1.5/na	7" well cover installed
4	08/15/77	20.5	6.6	14.29	7.69	16-20	14.5	1.5/na	7 " well cover installed
5	08/15/77	15	7.55	15.87	8.32	11-15	10	1.5/na	7" well cover installed
6	02/21/78	18	3.675	9.24	5.56	13-17	11	1.5/na	7" well cover installed, found tubing in well
3700	02/21/78	18	na	na	na.	10-13	9	1.5/na	Could not locate a
8	03/13/78	22	6.575	16.42	9.84	16-22	14	na/na	7" well cover installed, extend casing, found tubing in well
9	12/12/80	24.5	7.1	16.03	8.93	17.5-24.5	15.5	4/na	12" well cover installed, new locking well cap- soil boring
10	12/18/80	24.5	7.15	15.1	7.95	17.5-24.5	17	4/na	12" well cover installed, new locking well cap - soil boring
-11	12/19/80	29~	6.7	15.94	9.24	16-29	14.5	6/na	New locking well cap - soil boring
12	12/22/80	28.5	6.85	16.04	9.19	17.5-28.5	14.5	4/na	New locking well cap
13	04/13/82	A Comment	7.125	15.37	8.24	10.5-15.5	9.5	6/ца	12" well cover installed, new locking well cap - soil boring
14	04/13/82	25	7.075	15.49	8.41	15-25	na	4/0.01	New locking well cap - soil boring
15	04/13/82	25	8.35	17.26	8.91	15-25	na.	4/0.01	New locking well cap
16	04/13/82	22	4.8	12.08	7.28	12-22	na	4/0.01	New locking well cap
17	04/13/82	25	5.075	12.76	7.68	10-20	na	4/0.01	New locking well cap
18	04/13/82	25	5.37\$	13.57	8.19	15-25	na	4/0.01	New locking well cap, new well cover lid
18A	1983	51.5	6.5	13.36	6.86	35-50	35	4/0.02	Repair casing, new locking well cap
19	06/10/83	25	na	na	na	10-25	6	4/0.02	Could not locate
20	1983	(53)	3.55	14.93	11.38	31-51	28	4/0.02	Repair casing, new locking cap, grout inside casing
21	06/08/83	0.5	na	na	na	10-25	7	4/0.02	Could not locate

Notes: na = not available

		OMIUM IN SOIL		,
Boring	Approximate	Sample	Total	Hexavalent
Number	Sample Date	Depth (feet)	Chromium (mg/Kg)	Chromium (mg/Kg)
9	12/12/80	6-7	31	
10	12/16/80	3.5-4.5 5-7.5	29 52	:
	4			
11	12/18/80	3 5.5	39 35	
13	12/22/80	3	48	
		8	305	•
14	04/13/82	5	33	•
		10	48.8 40.2	
		15 20	46	
22	0.6 /0.8 /0.7	4-4.5	5,200	-
22	06/08/83	8-8.5	482	
23	06/83	2-2.5	38.4	-
		7.5-8	980	
24	01/09/85	2.0-2.5	6,700	< 0.2
		4.0-4.5	727	-
	1	6. 9-6. 5 9.0-9.5	432 41.5	91
	1	11.0-11.5	41.5	
25	01/05/85	2.0-2.5	2,030	< 0.2
	-0.00,000,000	4.0-4.5	503	< 0.2
		6.0-6.5	40.9	•
		9.0-9.5 11.0-11.5	44.9	<u>:</u>
26	01/09/85	4.0-4.5	48.2	< 0.2
***	V1103103	6.0-6.5	39.9	
		9.0-9.5	45.1	
		11.0-11.5	66.1	
27	01/09/85	2.0-2.5	95.3	*/
		4.0-4.5 6.0- 6. 5	78.2 102	
		9.0-9.5	250	1.6
		11.0-11.5	51.7	
28	01/09/85	2.0-2.5	52.2	
	SUPPLIES HE FACT	4.0-4.5	434	< 0.2
		6.0-6.5 9.0-9.5	4934 49.5	•
		11.0-11.5	24.1	
29	01/09/85	2.0-2.4	55.6	0.4
20	7.55.55.55	4.0-4.5	46	2000
		6.0-6.5	36.9	
		9.0-9.5	47.6	
30	01/09/85	2.0-2.5	45	
		4.0-4.5	48.5 36.1	*
	1	6.0 -6.5 9. 0-9. 5	57.9	
		11.0-11.5	110	44
31	01/09/85	2.0-2.5	60.8	-
	CM004362	4.0-4.5	45.2	*
		6.0-6.5	37	< 0.2
	1	9.0-9.5	73.7	**

Notes: Soil borings 9 through 14 correspond to monitor well locations. - no data

Table 3
TOTAL CHROMIUM IN SHALLOW GROUNDWATER

Well	Aug-77	Jan-81	Jul-81	Sep-81	Oct-81	Nov-81	Dec-81	Jun-83	Feb-85	Jun-91	Oct-91	Jul-94
Number	(ug/l)	(ug/l)	(ug/l)	(ug/l)	(ug/l)	(ug/l)	(ug/l)	(ug/l)	(ug/l)	(ug/l)	(ug/l)	(ug/l)
1	200	-	-	<1	1	2.5	32		<20		<50	*
2	60	-		<1	4	1.1	2	_		-	-	
3B	60	•	-	<1	480	2,000	190				110,000	
3C	18,000 7,100	-	55	30,000	28,000	22,000	17,000	-	7,250	∌	2,300	-
4	90,000		520	57,000	61,000	56,000	55,000	- 1	59,000	17,000	22,000	
5	360,000		-	-	880,000	610,000	280,000	-	480,000	390,000	260,000	
6				630	80	790	630		3,330		31,000	-
7	-	-	-	<1	<1	-	3	-	-			
8	•		-	<1	2	2,5	70	-	<20	•	<50	-
9	-	258,000			-	-	-	-	892,000	-	140,000	-
10	-	17,000		- E	-		-		746,000	_	490,000	-
11	-	129,000*	340	-	-	-		×	2,440	-	470	_
12	1	32,000	-		-	-	-	-	240,000	38,000	44,000	_
13	1	381,000		-	-	-	7-1		676,000		510,000	230,000
14			•	-	-	=	-	=	654,000	-	320,000	-
15		-	-	¥		_	-	-	<20	30	<50	-
16	-	-		-	-	-	-	-	460,000		240,000	120,000
17	-		-	-	-	-			90,000		250,000	190,000
18		-	-	-	(€)	-	-		60,500	-	31,000	-
19	-							<20	20		-	-
21	(·	_	-	-	-			20	40		-	

Notes: October 1991 data also includes November 1991 data

- = no data

^{*} Data represents the average of 8 sequential samples collected hourly during a pumping test.

< 20 = less than detection limit

Table 4
HEXAVALENT CHROMIUM IN SHALLOW GROUNDWATER

Well	Aug-77	Jan-81	Jul-81	Sep-81	Oct-81	Nov-81	Dec-81	Jun-83	Feb-85	Jun-91	Oct-91	Jul-94
Number	(ug/l)	(ug/l)	(ug/l)	(ug/l)	(ug/l)	(ug/l)	(ug/l)	(ug/l)	(ug/l)	(ug/l)	(ug/l)	(ug/l)
											W	20 1/((4-1)-1
1	-		-		-			-	<20	-	50	- 7
2	-		-		-	-		-	-			
3B	-	-	-	-		-	_			-	100,000	-
3C	12,000		(- N			*	(40)	-	6,300	-	1,600	_
	6,700											
4	67,000	-	-	-	-			-	59,000	17,800	22,000	6,300
5	295,000	-		-	2,240		*	-	480,000	-	250,000	454,000
6	-	-	_	-	((*)	-	-	-	3,300	-	25,000	4,800
7	-	-			1/4/					-		-
- 8		-	-	-	-	-	-	-	<20	_	<10	-
9	-	185,000	~		-			-	877,000	-	130,000	-
10		14,000		-	-		-	(#3)	740,000	-	450,000	_
11		115,500*	34	-	-	_	-		2,410	_	410	_
12		12,000			-	_		to.	240,000	29,700	39,000	
13	-	325,000	-			_	-	-	676,000	(#	430,000	130,000
14	-		-		(*)		4		632,000		310,000	
15	-	•	-	-	-	-	*	-	<20	-	<10	<10
16		-		(*)		-	-	- 31	460,000		290,000	320,000
17	-	-	-	•	_	-	-	-	38,200	-	300,000	200,000
18					-		-	-	55,000	-	24,000	
19	6 .	-	-	(●))	-	-	-	<20	20	-	-	-
21	-	-	_		_		-	<20	<20		-	

Notes: October 1991 data also includes November 1991 data

* Data represents the average of 8 sequential samples collected hourly during a pumping test.

< 20 = less than detection limit

= no data

Table 5 TOTAL CHROMIUM IN DEEP GROUNDWATER

Well Number	Aug-77	Jan-81	Jul-81	Sep-81	Oct-81	Nov-81	Dec-81	Jun-83	Aug-83	Feb-85	Oct-91	Jul-94
Number	(ug/l)											

3A	50	-	J. #1	<1	<1	230	14	-		770	130	_
18A	-	-	-		-			20	-	<20	<50	_
20	*		2 .6 3	5 4 3	*	W	- (1,300 1,300	90	<20	<50	-

Notes: October 1991 data also includes November 1991 data

* Data represents the average of 8 sequential samples collected hourly during a pumping test.

< 20 = less than detection limit

no data

Table 6
HEXAVALENT CHROMIUM IN DEEP GROUNDWATER

Well Number	Aug-77 (ug/l)	Jan-81 (ug/l)	Jul-81 (ug/l)	Sep-81 (ug/l)	Oct-81 (ug/l)	Nov-81 (ug/l)	Dec-81 (ug/l)	Jun-83 (ug/l)	Aug-83 (ug/l)	Feb-85 (ug/l)	Oct-91 (ug/l)	Jul-94 (ug/l)
3A	-	-	_	-		•			-	80	<500	_
18A	-	-		-	-	-	-	<20	-	<20	<10	-
20	-	-	-	-	-	=	-	1200	40	<20	14	-
								530				

Notes: October 1991 data also includes November 1991 data

* Data represents the average of 8 sequential samples collected hourly during a pumping test.

< 20 = less than detection limit

- no data

	Table 7											
		PURGEAB	LE HALOC	ARBONS I	N SHALLO	W GROUT	(DWATE)	R				
Well Number	Date	1,1-DCE (ug/l)	Trans 1,2-DCE (ug/l)	1,1-DCA (ug/l)	TCE (ug/i)	TCA (ug/l)	PCE (ug/l)	Methylen Chloride (ug/l)	Vinyl Chloride (ug/l)			
1	3/21/85 11/15/91	<0.5 0.5	<0.5 4.8	<0.5 1.6	33 11	<0.5 <0.5	21 0.6	<0.5 <0.5	<0.5 <1			
2	NS		-		-		*	•				
3B	10/29/91	13	45	1.2	650	<0.5	6.8	<0.5	6.4			
3C	6/11/85 10/29/91	<0.5 61	23 46	<0.5 5.4	150 180	2.4 34	1. 7 1. 7	<0.5 <0.5	<0.5 18			
4	11/4/91 7/28/94	<5 -	260	<5 -	2,100 6,500	<5	31	<5	10			
5	11/4/91	4.2	120	42	410	1.3	8.9	<0.5	54			
6	6/11/85 11/5/91 7/28/94	<5 29	54 78		220 420 790	3.9 6.4	<5 5.9		<5 19			
8	6/10/85 6/11/85 11/5/91	<1 1 0.8	19 32 23	1	46 93 38	<1 <0.5 <0.5	18 35 35	<5	3 4.9			
9	6/13/85 10/30/91	<5 <0.5	31 13		700 200	<5 <0.5	26 11	<50 <0.5				
10	6/12/85 6/12/85 11/7/91	<50 <50 3,800		<50	5,100 12,000 14,000	<50 <50 6,500	81 <50 <50	<500	-			
11	6/12/85 11/15/91	<0.5	3.4	<0.5	19 10	1.3	5.3 1.5					
12	11/11/91	3.3	9	1.3	130	4.6	10	<1	<2			
13	11/8/91 7/28/94	6.8	89	15	630 770	<5	8.9	<5	20			
14	3/21/85 11/11/91				580 4,300	<0.5 17	26 13	_				
15	6/13/85 11/12/91	<50 <5				<50 <5	<50 <5					
16	3/21/85 11/19/91 7/28/94	1,200			19,000	<0.5 1300	42 <5	< 5				
17	6/13/85 11/19/91 7/28/94	L 54	54	7.8	460		18 8.9	<5				
18	6/12/85 6/12/85 11/19/91	<50	<50	<50	340	66	32 <50	<500)			
19	3/21/85					<0.5	23		<0.5			
21	6/13/85	<50	800	<50	2,200	110	<50	380	<50			
_			_									

Notes:

⁼ No data

<5 = Not detected above reported detection limit

Table 8 PURGEABLE HALOCARBONS IN DEEP GROUNDWATER

Well Number	Date	1,1-DCE (ug/l)	Trans 1,2-DCE (ug/l)	1,1-DCA (ug/l)	TCE (ug/l)	TCA (ug/l)		Methylen Chloride (ug/l)	
3A	10/29/91	<0.5	<0.5	<0.5	<0.5	<0.5	<0.5	<0.5	<1
18A	6/13/85 11/19/91	<0.5 <0.5		<0.5 <0.5	10 <0.5	<0.5 <0.5	<0.5 <0.5	(2.4 <0.5	
20	11/15/91	<0.5	<0.5	<0.5	<0.5	<0.5	<0.5	<0.5	<1

Notes:

= No data

<5 = Not detected above reported detection limit</p>

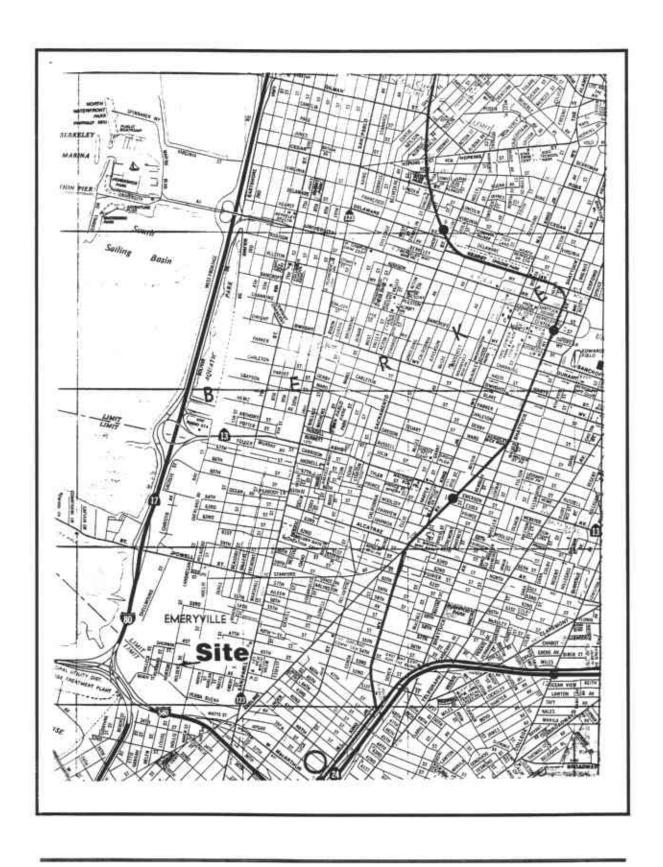
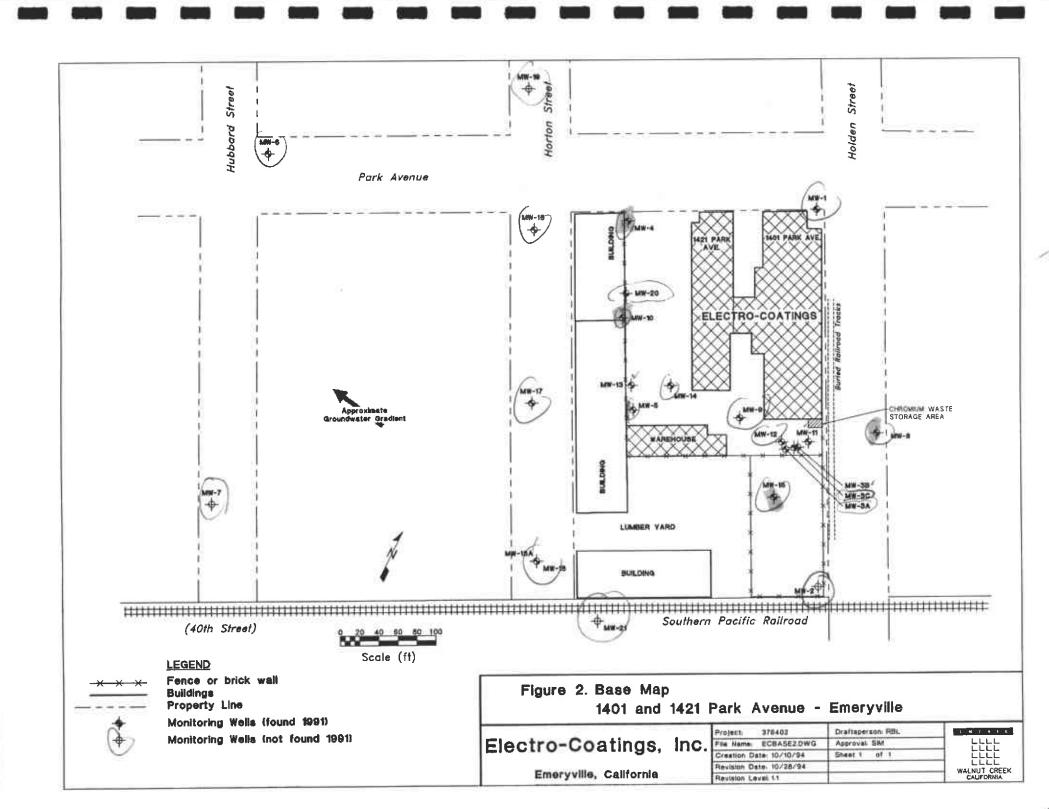
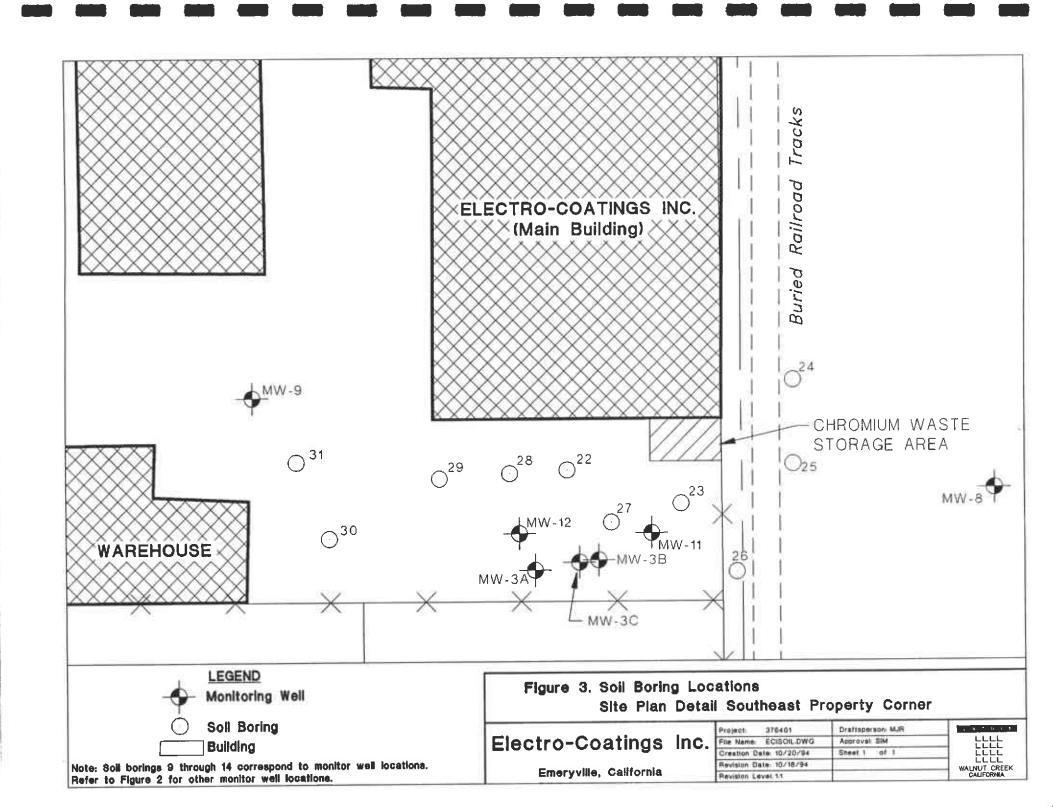
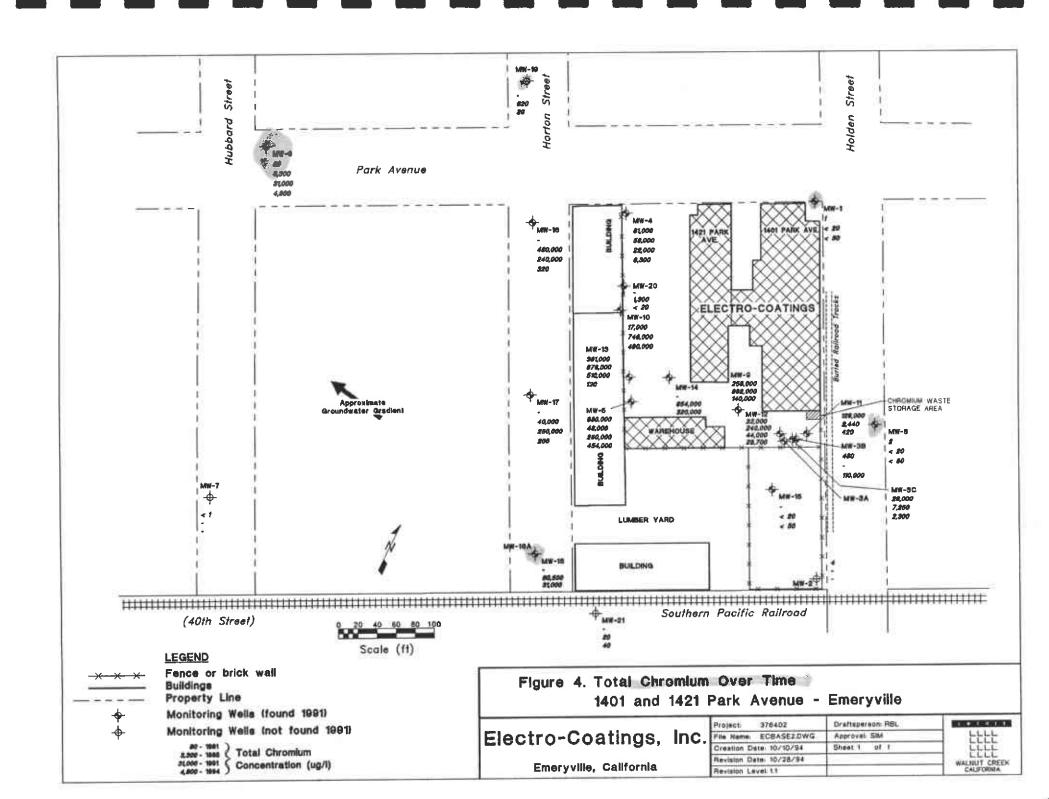
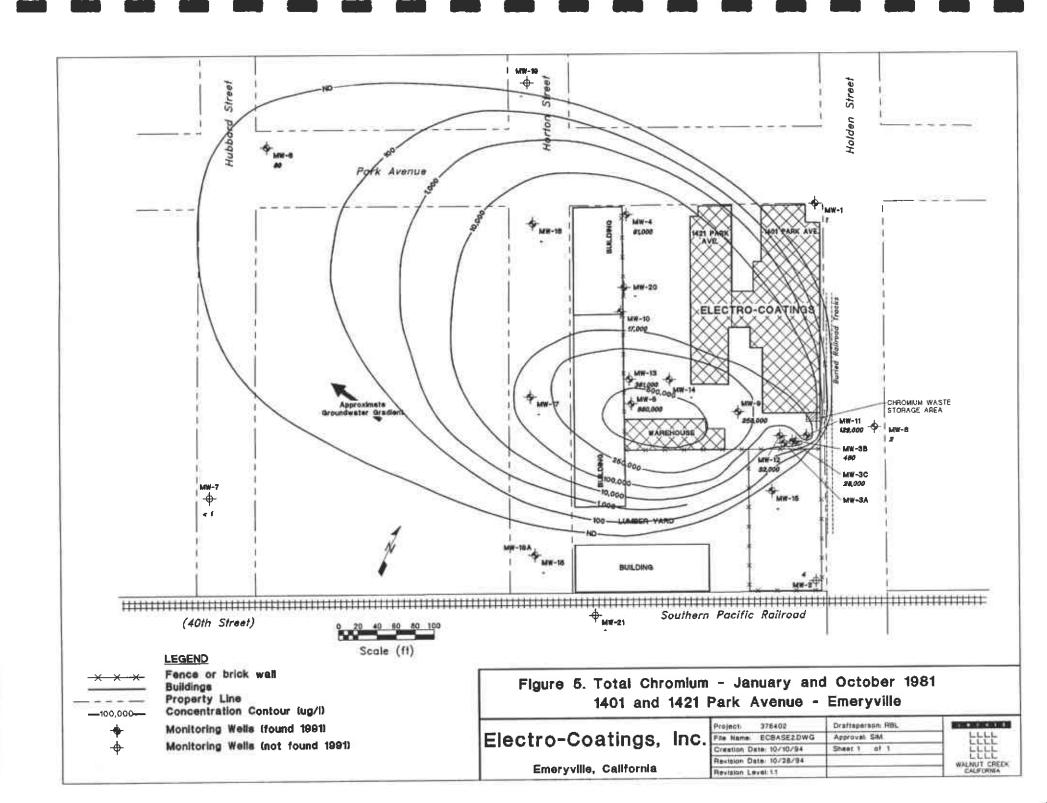


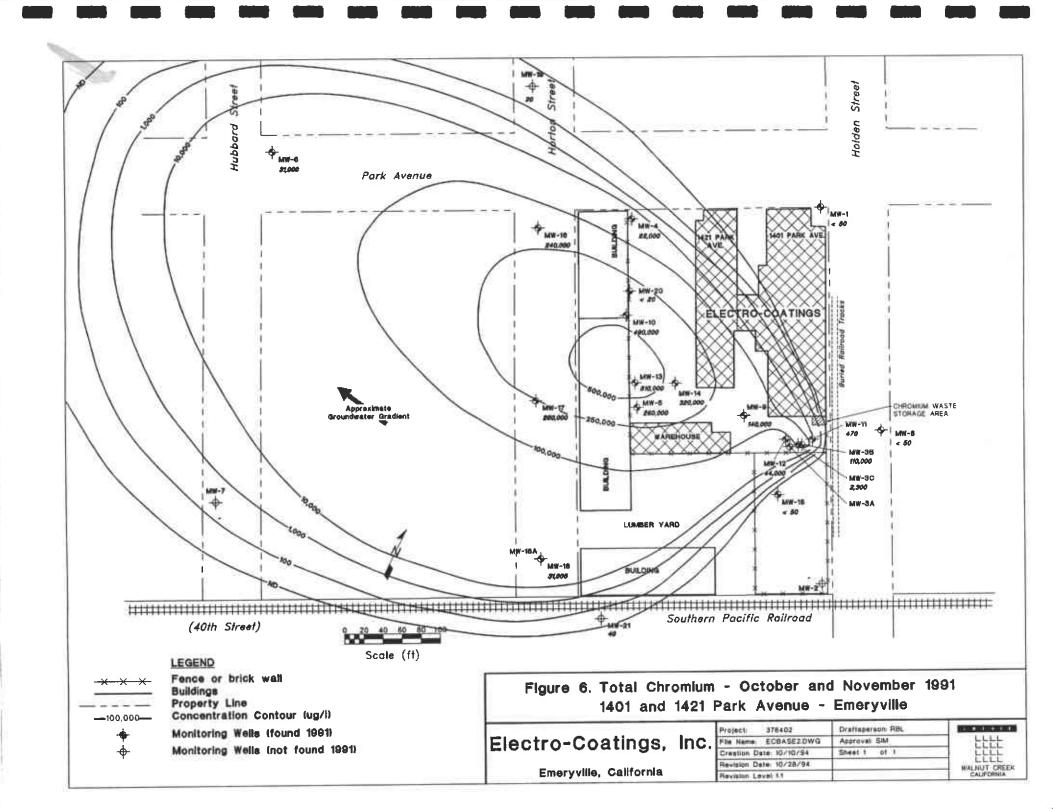
Figure 1. Site of Electro-Coatings, Inc.

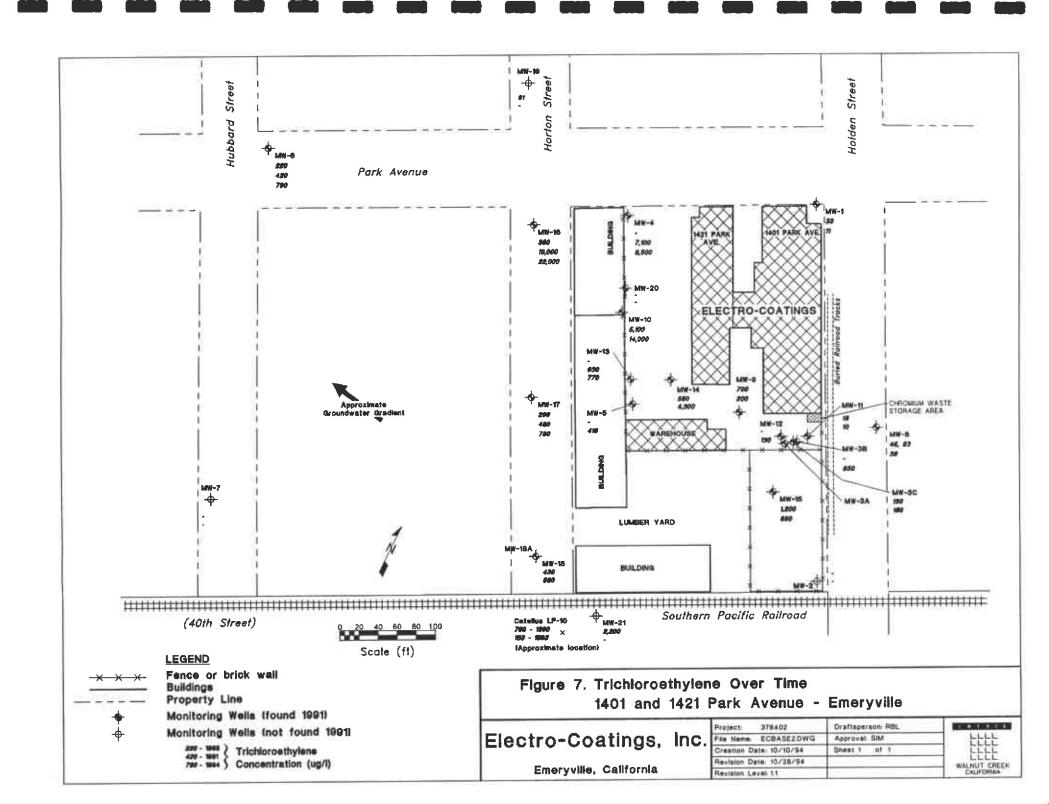


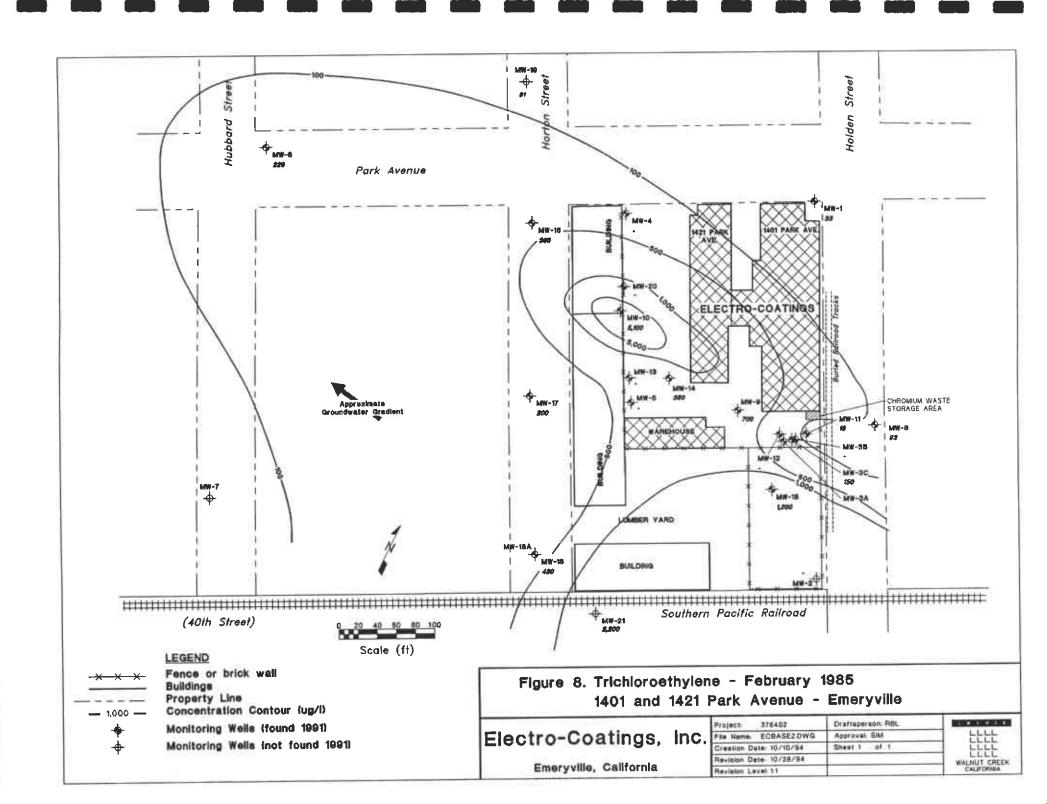


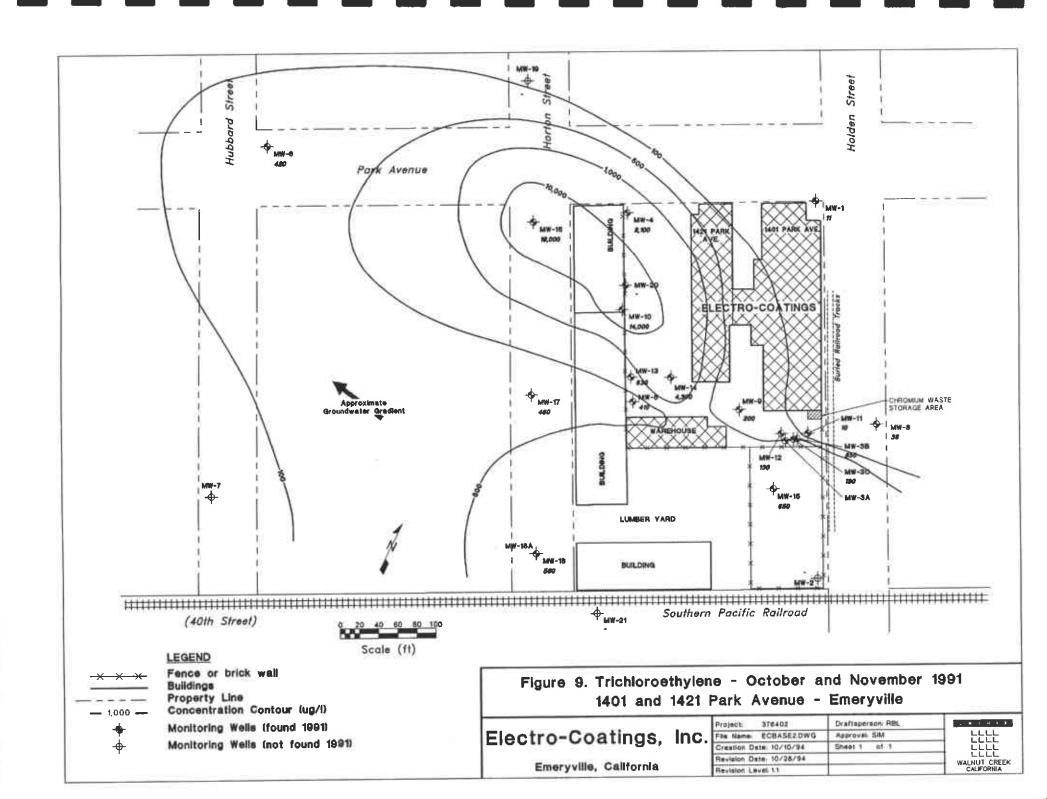


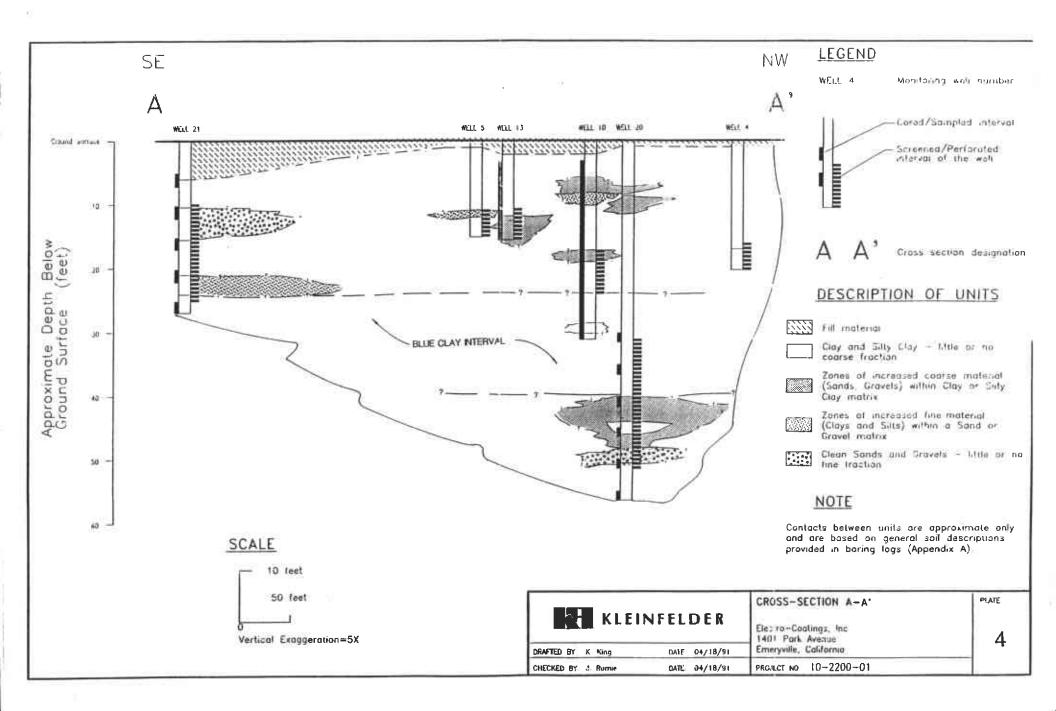


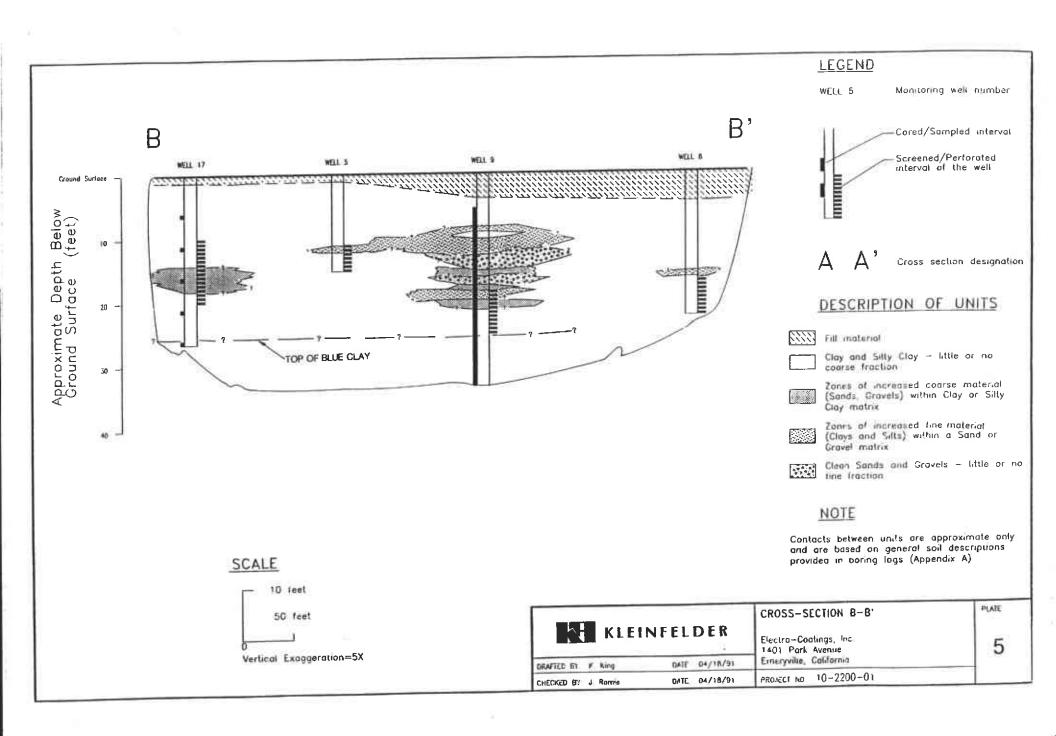


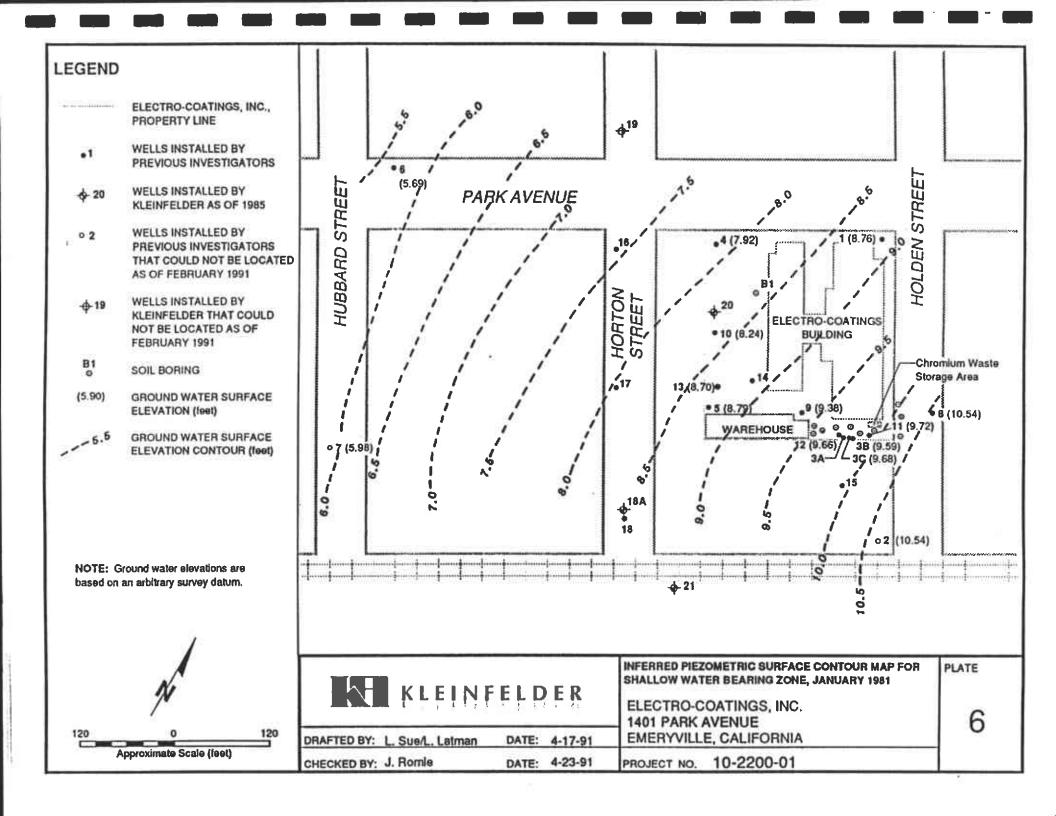


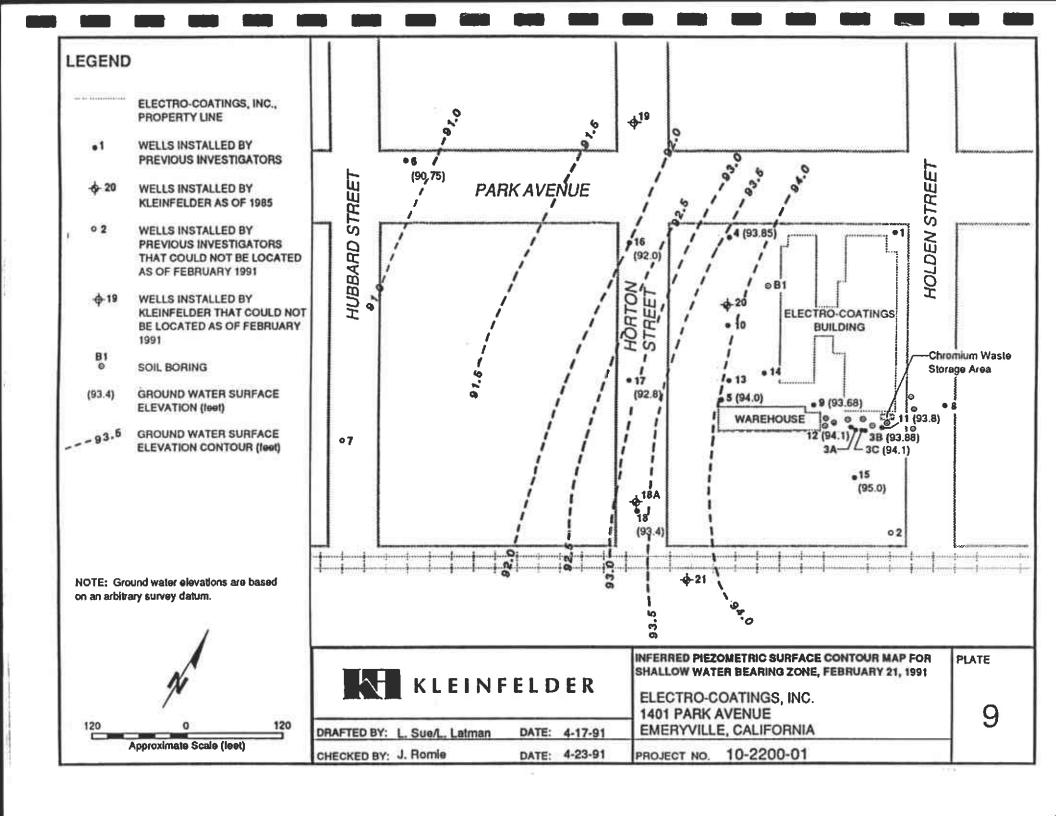












Project	Eme	CTRO COATINGS INC. eryville, California	LOG C)F	WELL		NO.	1
Tate Drille Type of Bo Hammer W	ring: <u>6"</u>	gust 18, 1977 Auger	Remarks:					
Depth, F1. Samples	Blows/Ft.	MATERIAL I	(See Legend Sh		sampler si	res	WEL INSTALL DETA	L ATION
	Su	rface Elevation:	(a) au				100	
10-	Upon comple of dril:	calcareous, with noo S I L T Y C L A Y Stiff, moist to wet trace of sand and so Gravelly layer Becomes medium stidark brown and gravely S I L T Y C L A Y Stiff, wet, green and	(CL) (CL)	n, vel	(FILL)		Athermonium months and a gravel GROUT SEAL CHARLES GROUT G	CAP CASING BENTONITE SEAL PROTECTIVE CAP
-		BOTTOM OF HOL	LE @ 30'					
Proj. No.	13895B	WOODWARD-	CLYDE CONSULT	ANTS		1	Figure	2

					=							
Proj			Emery	RO COATINGS		LOG	OF	WELL	. [NO.	2	
Date	Drille	d:	Aug	ust 18, 1977		Remarks:_						
·pe	of Bai	ring:_	6" .	Auger		· · · · · · · · · · · · · · · · · · ·				 		
Hamm						(See Legend	Sheet fo	r sompler size	es o	nd homi	ner we	ights)
Depth, Ft.	Samples	Blows/Ft.		MA	ATERIAL D	ESCRIPTION	ON			INSTA	ELL LLAT: AILS	10%
			Sur	oce Elevation	· ;						4	
15-			-V- 9-8-77	SILTY mottled d SILTY Stiff, mo Tecomes SILTY GRAVEI CLAYEY mottled g SILTY grayish brown Trace t CLAYEY CL	CLAY ark brown a CLAY ark brown a CLAY sist, black grayish by CLAY sist, mottle ac of fine; (GM): Dens SILT gray and brown CLAY at trace of af water to some gray coist, brown ILTY coist, gray- acets	(CL): Sti. black & g: (CL) (CL): Sti. fine sand vel (CL): Sti fine sand (CL): Sti fine sand (CL): Sti (CL): Sti fine sand (CL): Sti (CL): Sti	d gray avel brown mp, elay lend small	nses		2' FALL-IN SAND FILTER GROUT SEAL SEAL		
25-				вотт	OM OF HOI	LE 0 23'		-				
30-					·							
II Drai	No	138	G S R		WOODWARD-	CLADE CONC	111 T A NITS	2	1	Figu	20	3

5 1

Project:	ELECTRO COATINGS INC. Emeryville, California	LOG OF WELL	NO. 3A
Date Drilled:	August 15, 1977	Remarks:	
Type of Boring:	4 7/8" \$ Rotary		
Hammer Weight		(See Legend Sheet for sampler size:	
Samples Blows/Ft.	MATERIAL I	DESCRIPTION	WELL INSTALLATION DETAILS
	Surface Elevation:		<u> </u>
	6" ASPHALT CONCRETE		
5	SANDY CLAY F Stiff, moist, brown, CLAYEY SAND Dense, moist, bluish GRAVEL LAYER: Dense, gr SILTY CLAY Very stiff, moist, b T Becomes gray, with 9-8-77 T Becomes bluish gr. SILTY CLAY Stiff, moist, blue- SILTY CLAY Very stiff, moist,	with bricks & blue clay F I L L (SC-SP) n green to brown (ay, with clay binder (CL-CH) (FILL) plack n trace of gravel ay (CL) gray	GROUT SEAL GROUT SEAL ING PROTECTIVE CAP-
15-	brown, with rust st		14" PVC CASING
25— 30— Proj. No. 1	T moist, blue, with No gravel	L-ML): Medium stiff, very trace of fine gravel -CLYDE CONSULTANTS	Figure 4

Proj	ject:		ELECTRO COATINGS INC. Emeryville, California	LOG	OF	WELL	NO. 3A
Depth, Ft.	Samples	Blows/F1,	MATERIAL D				WELL INSTALLATION DETAILS
35— -			}Sand lense: black a	and blue		-	or seal
40-						-	GROUT
45			VERY SILTY C Stiff, moist, gray, thin sand lenses		CL-ML)		TTE SEAL
55-			SILTY CLAY (Stiff, moist, brown	CL)			ER BENTONIS
60-			SILTY SAND & Dense, wet, reddish gravel to ± 1" diame SILTY CLAY Stiff, moist, reddis	(CL) h brown	E L	(GM-SM)	SAND FILTER -
65- Proj	j. No.	138	958 WOODWARD-CI	YDE CONSUI	LTANTS		Figure 5

			 j
Project:	ELECTRO COATINGS INC. Emeryville, California	LOG OF WELL	NO. 3B
Date Drilled:_	August 15, 1977	Remarks:	
Type of Boring:	6" Auger		
Hammer Weight	l;	(See Legend Sheet for sampler sizes	and hammer weights)
			WELL
Depth, Ft. Samples Blows/Ft.	MATERIAL	SECOND TION	INSTALLATION
Depth, Sample	MATERIAL L	DESCRIPTION	DETAILS
S B			
	Surface Elevation:		
	6" ASPHALT CONCRETE		1 1/464 1 1
1 1 11	SANDY CLAY FILL:Stiff,	moist, brown, with bricks	1 (3):1
1 4 11	CLAYEY SAND	(SC-SP): Medium dense,	
	very moist, blue-gre	en to brown	/小小 8
	3'-0" Gravel lense,	(FILL)	
	after Gravel lense, drilling	(FIBB)	SEAL
5-	SILTY CLAY (CL): Very s	tiff, moist, black	SEAL NG NG ROTECTIVE
			ROUT SE
	9-8-77 TBecomes gray, with	organic material	SROUT
]] []		ue-gray, with trade of	
1 - 1	fine gravel	ļ.	
1 - 1			15."
10-	SILTY CLAY	(CL)	1 1141.1
1 10	Medium stiff, moist,	gray-brown,	
1 - 1	trace of fine gravel		
4			FILTER FILTER
1 1		LAY (CL)	4 & CJEJ T
	Very stiff, moist, r	nottled gray and brown	aw Sand
1 7 11			R
15-	\Trace of water at	time of drilling	
1 4 11			d d
	SAND AND GR	A V E L (SW-GW)	
	Donse, wet, dark bro	own, with trace	
	of clay		l s
	Sand and gravel		
20			
	BOTTOM OF HOL	F 0 20'	PERFORATIONS
		51. 6 20	」 と と
1 7 11] <u>6</u> .
-		 	
	i		4
25		j	
		ì]
]	7
		1	4
			4
		ł	1
30-	1	İ	
			<u> </u>
Proj. No. 1	3895B WOODWARD	-CLYDE CONSULTANTS	Figure 6

					-							
Pro				Emery	TRO COATINGS INC.	LOG	OF.	WELL	į	NO.	3C	
Date	Drill	ed	:	August	t 15, 1977	Remarks:						
10010	of Re	orin	·	4 7/8	" ¢ Rotary	.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,						
Hamn						(See Lenend	Sheet fo	or sampler size	s 0	nd hom	mer wei	ahts)
		7							T		ELL	
□ □	<u>=</u>	1	<u>E</u>						1		LLATI	ON
Ę	Samples	1	×		MATERIAL I	DESCRIPTI	ON				AILS	•
Depth,	So		Blows/Ft.						\vdash		· · ·	
}	!	Щ.		Surt	ace Elevation:				7			\
		\top	1		6" ASPHALT CONCRETE				+		1-3	/
_	1 1	1		<u> </u>	SANDY CLAY FILL: Stiff,	maint be	···	th bricks	4	13	153/	CAP.
7	1 1	ı	ļ					CII DITCKS		1	141	
1 -	1 1	-	}		CLAYEY SAND		-	')	7	/		Z CE
] -	- i				Dense, moist, blue	reen to br	own		٦.	14 5	\િ છ	TI
1 .]	1	Ī		CLAYEY GRAVEL: Dense,	damp, brow	'n,		48	SEAL	17 18	E E
			4	<u> </u>	angular to 14" in d			(FILL)		7 '	11 5	PROTECTIVE
5-	7		Ì	after drilli	ing Silii Cl	-	,-CH)	ľ]	ROUT	PVC CASING	Δ.
-	1 1			∇	Very stiff, mois	t, black		İ	4	ğ N	111 6	
el -	4		7	9-8-77	7 T Becomes grayish b	lue, with c	organic	s,	4	_\R_,	; 5	•
]					trace of fine gra				ل	与汉	[2] [•
1 '	1				•				٦	St.		
n -	-				SILTY CLAY (CL): Stif	f, moist, n	nottled	brown	4	PERFORATIONS SAND F	163	$\overline{}$
10-					and blue-gray, with tr	ace of orga	anic ma	terials	4	SAND		ر <u>ک</u> ر
1 10			!							g (]]	SEA
P ⁱ	┪				TY CLAY (CL): Very stif				7	¥ .		P
J, .	4			and	black mottled, with tr	ace of fine	grave	1 & sand	┥	E :		I
	_]				SILTY CLAY (CL): Ve	rv stiff.	noist.		4	V:	<u>l</u> l.	وَم
ri 💮				\ \	mottled gray and br					1	TI I	-CAP BENTONITE
5	7	11		Ì	——— Trace of water			1	٦	1:) "
15-	-	╀							\dashv			-/
B1	1				\				4			
P	1			ļ	BOTTOM OF HO	PF 6 12.		j	ļ			
_ {	7								٦			
1 (4			ļ					┪			
P	1			1				1	4			
_								i				
20-	_								Π			
•	4			}				İ	-{			
_												
P	j			1					1	ı		
.	4			1					▎┤			
25	_								┧ᆜ			
• i ''	-											
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	4									į		
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	7		}]		
30				}					-	1		
• 1									-	1		
Pro	<u>. l .</u> j. N	ـــــــــــــــــــــــــــــــــــــ	135		WOODWARD	-CLYDE CON	SULTAN	TS		Fig	ure	7
- Irru	۱۱ مان	٠.	100	ں ں ر	1,000,,,,,,	32.22 00.		· -		1		

, ,	oring:_	•	Remarks:			
Hammer V	-		(See Legend Sheet fo	r sompler sizes	T	
Depth, Ft. Samples	Blows/Ft.	MATERIAL	DESCRIPTION		INSTA	ELL LLATION AILS
		Surface Elevation:			1 <u>-</u> /	<u> </u>
		3" ASPHALT CONCRETE			1	1
5 - 1		SILTY CLAY Very stiff, moist, after Becomes gray drilling Trace of gravel 9-8-77 Trace of gravel SILTY CLAY Very stiff, moist, with trace of grave	(CL) gray-brown,		SAND FILTER GROUT SEAL	BENTONITE SEAL 15" PVC CASING -
15—		Water inflow, pos VERY CLAYEY Medium dense, mois				
		BOTTOM OF H	OLE @ 20.5'		1	<u> </u>
25-					-	

i

	Project		ELECTRO COATINGS Emeryville, Californ		LOG	OF	WELL	NO.	5
	Date Drill	ed:	August 15, 1977		Remarks:_				
			6" Auger						
_	łammer W	7			(See Legend	Sheet fo	r sampler size:	1	
	Depth, Ft. Samples	Blows/F1.	MAT	ERIAL D	ESCRIPTIO	NC		INST	ELL ALLATION TAILS
	<u> </u>		Surface Elevation:					1	
_			3" ASPHALT CONC	CRETE		•		= 1	沙
	5		Very stiff,	LAY moist, bl cluish lighte of fine stiff, mo (CL): Very ine gravel	(CL-CH) ack at gray, gravel bist, mott y stiff, m and sand ense, wet,	led gra oist, l	ay & brown	SAND GROUT SEAL	RENTOMITE SEAL 14" PVC CASING—CAP
	15		gray-brown,		ce of fine			PERFORATIONS	NEW O
	20-								·
	25-						·		
P	Proj. N	o. 13	395B W	/OODWARD-	CLYDE CONS	SULTAN	TS	Fig	ure 9

1

Pro	ject	:	ELECTRO COATINGS INC. Emeryville, California	LOG OF WE	LL	NO. 6
Date	Drille	ed:	February 21, 1978	Remarks:		
√vpe	of Bo	ring:_	6" Auger			
tmr	ner W			(See Legend Sheet for sampler s	izes	and hammer weights)
Depth, Ft.	Somples	Blows/Ft.	MATERIAL D	ESCRIPTION		WELL INSTALLATION DETAILS
		·	Surface Elevation: 9.09			
5-			Becomes stiff Becomes stiff to vere calcareous fragment Becomes brownish gradient and fine sand SILTY CLAY (CL): Stift	ark gray mottled ry stiff, trace of s ay, trace of organics green with		ONS BENTONITE PVC CASING PROTECTIVE CAP
15-			Becomes yellow with Very silty clay lay Becomes brown, no s Becomes medium stiff interbedded Very silty sand to	er, yellowish brown and or gravel f, brown and gray	1 1 1 1 1	PERFORATIONS
20-			BOTTOM OF HOL	E @ 18'	1	
25-			_			•
Proj	. No.	138	95C WOODWARD-C	LYDE CONSULTANTS	-	Figure

Proj	ect	;	ELECTRO COATINGS INC. Emeryville, California	LOG OF	WELL	. NO. 7
Date	Drille	e d :	February 21, 1978	Remarks:		
pe	of Bo	ring:	6" Auger			
		eight:		(See Legend Sheet for s	ompler sizes	and hammer weights)
Depth, Ft.	Samples	Blows/Ft.	MATERIAL D	ESCRIPTION		WELL INSTALLATION DETAILS
			Surface Elevation: 9.58			
10-		2/24,	Medium stiff to stiff dark gray to black	bluish gray, es, bluish gray (CL-CH): Stiff to v light green (CL): Very stiff, m, some gravel and CC-CL) EL (SC-GC) rown, gravel to 1" (CL) brown, trace of fin	sand	PEA GROUT SEAL GROUT SEAL GROUT SEAL GRAVEL GROUT SEAL CAP PEA GROUT SEAL CAP PEA GROUT SEAL CAP PROTECTIVE CAP
25-			BOTTOM OF HOL	Z 0 18'		

	4				···	_			
		ELECTRO COATINGS INC. Emeryville, California	LOG	OF	WEL	L	NO.	8	
Date I	Drilled:_	March 13, 1978	Remarks:						
		: 6" Auger							
Hamm	er Weigh	t:	(See Legend S	haet for	sampler siz	es o	ind hamm	er weigh	its)
Depth, Ft.	Samples Blows/Ft	MATERIAL	_ DESCRIPTION	N			INSTAI	LLATION AILS	Ŋ
		Surface Elevation:				┥.		\mathcal{L}	\
-		4" ASPHALT CONCRETE 5" AGGREGATE BASE					期	<u> </u>	-)- g
4		CLAYEY SAND & GRAVEL FILL	_ 			4		₹ \	Q & D
4		SILTY CLAY (CL-CH) (F black, trace of wo CLAYEY TO SANDY SILT	od	iff, m	oist,	$\frac{1}{2}$	SEAL -	CASING	POOTEOTIVE
7		Loose, wet, black,			\mathcal{A}	4		付 5	ŭ
5-		SILTY CLAY (CL-CH): S	tiff, moist, da	rk gra	У	-	GROUT	₹ SA SA	Qqq
4		Grades to grayish blu	e, trace of lim	e nodu	les	4	5 [1]	扫 "	-
4		SILTY CLAY	14-7		1	4		기 .	L)
4		Stiff to very stiff trace of lime nodu	f, wet, bluish des	green,			14	7	SEAL
	ATI	Trace of fine or							ក
10-	Air	Becomes more si	lty, brown inter	bedded	i	7	4 门	行	BENTONITE
10		with dark gray	••			\exists	GRAVEL	7	Š
1		SILTY CLAY	(CL): Stiff	to ver	y stiff,	1	8 3	\overline{A}	BE
)		moist, reddish bro	own and gray mar	bled	-	\dashv	PEA	•	
_		Trace of fine sa	and, slightly gr	eenish		4	PEA GRA		,
-		With some sand			İ	4		房	
15		with some sand				4	7.1	<u></u>	
4		CLAYEY SAND & FINE G	RAVEL (SC)	.		4	1:4	CONS	
_		Medium dense, wet	, reddish brown						
		VERY SILTY CLAY (CL)						PERFORATIO	
		wet, brown, trace Twith thin clayer			rel	1		H H	
]		and coarse sand		-		1		[4] [7]	
20-		VERY SILTY CLAY (CL)	: Stiff to verv			\dashv		- حرانیا	
		stiff, brown marb				4			
+						+	<u> </u>	4 4	
		PORTON O	F HOLE @ 22'			4			
[-{			r HODE & 55			4			
25									
]	11					7			
1 7		.				1	•		
Į Ţ									
) 1					i				
30-						\dashv			
						-			
Proj.	No. 13	895C WOODWAR	RD-CLYDE CONSUL	TANTS		\neg	Figur	e 2	
						L			

ype of	Bori	12/10/ ng: 45/	ville, California 80 to 12/12/80 8" Pitcher Core Barrel	Log of Boring Remarks: (See Legend Sheet for sampler fyr	pes and hammer wei	ghis)
	- 1	t.recovered Ft.cored		DESCRIPTION	INSTALLAT DETAILS PROTECTIVE	
		<u> </u>	Surface Elevation:			
			8" ASPHALT			
2 -			GRAVEL FILL angular fragments (up to 1"), r	eddish-brown		4" PVC CAP
4 -			CLAY medium hard, very dark gray		4L	
		0 1	lost sample in hole		GROUT SEAL	
7 -		2.15 2.5	CLAY firm to stiff, mottled, trace of grading more sandy ar		85	5 7/8" BOREHOLE
8 -			CLAYEY SAND trace fine gravel, light green		_	
1 1			CLAYEY SILT-SILTY CLAY soft, wet, gray			Š
9 -	ORING	1.5 1.5	GRAVELLY SILTY SAND green, orange mottling		(TE)) }
11 -	CONTINUOUS CORING	1.1 2.5	SILTY SANDY GRAVEL crumbly, moist, gravel is sub of gravel is 1"}, mottled, gree	angular-subrounded (maximum size enish-reddish brown	PPROXIMATE) BENTONITE SEAL (APPROXIMATE)	4" PVC CASING
12 -			core loss		KIMATE)	
13 -		1.7 2.5	to light green tra	vel and clay, mottled, light brown	IISTN	
15-	1	1.0	SANDY SILTY CLAY soft, light brown to light gro	een		
			SANDY GRAVEL		1	

roj	ect:	ELECT Emery	FRO COATINGS INC. ville, California	Log	of 	Boring	No.9
Depth, F1.	Samples	F1.recovered/ F1.cored	MATERIAL		WELL INSTALLATION DETAILS		
	-	ŭ	subrounded gravel, crumbly				4
16	1		more clayey				1 8 8
17		_ -	SANDY CLAY				
" 🚽		2.1 2.5	firm, light brown to light green				
18 -			CLAYEY SAND trace of fine gravel, firm, slightl	y moist, in places s	andy c	lay	PEA GRAVEL
4		\vdash					
19 -			core loss				A THE E
_		1.3 2.5	SANDY CLAY fine sand, firm, slightly moist,	light brown to light	green		AL PERFORATED PVC
20-			fine sand, firm, slightly moist, reddish-brown and brown mot	tling			
21 -			SILTY CLAY moderately stiff, light brown to	o light green			
Z1 - -			core loss				EA GRAVEL. BENTONITE SEAL. INTIMUE SEAS SEAS SEAS SEAS SEAS SEAS SEAS SE
22 -		1 1 5	<u>+</u>				H H N E
	1	1.5 2.5	·				RAV ENT
23	NG -						PEA GRAVEL BENTONIT THE PER CRAVEL BENTONIT THE PER CRAVEL
•	NUOUS CORING		core loss				
24	JUS (
		1.7 2.5	more sandy or	ange mottling mor	e comn	non	
25-	CONT	1.5	trace gravel near base				
26]]		SILTY CLAY firm, common orange mottle	s, blue			
			core loss				Nos Nos
27	4	1.7					
	4	1 <u>.7</u> 2.5	angular fine gravel more prec	dominant with min	or sand	ly zones	DRILL CUTTINGS
28	-		arragered three growth				
	,]						
29	']		SANDY CLAY fine sand, trace gravel, orang	e mottling, bluish-	gray		
30		2.3 2.5					
	4						
3	1 -		extensive orange mottling				
	4	2.2	2				
3	2 -	2.5	grading more sandy				-
-		lo. 149:	Woodw	ard-Clyde Cor	rsulta	ents	Figure 2b

Project:	ELECT Emery	TRO COATINGS INC. ville, California	Log of	Boring	No.9
Depth, Ft. Samples	Ft.recovered/ Ft. cored	MATERIAL	WELL INSTALLATION DETAILS		
33 30	Ft. (84	Bottom of boring @ 3	3.5 feet		DRILL CUTTINGS
Proj. No			d·Clyde Consulta		Figure 2c

Date /pe	Drilled of Bori	Emeryv j : 12/12/ ng 4 5/	/80 to 12/16/80 Re: /8" Pitcher Core Barrel	marks:	Boring		
Hamr Oepith, Fr	Solution Solution	Ft. cored	MATERIAL DES			INSTAL DET	LATION TAILS
ä	Š	Œ		<u></u>		PROTEC	TIVE CAP
			Surface Elevation: 6" ASPHALT			119	
-	4	-	GRAVEL FILL (road ballast)			一员	
3 4 5 - 6		1.5 1.5 1.5 2.6 2.5	SILTY CLAY slightly stiff, moist, black more gravelly crumbly with less silt SANDY CLAY moist, trace orange mottling, dark gra angular gravel (up to 3 cm) near top	Y 12/29/80 1/14/81	Water Lavels	SEAL	BOREHOLE 4" PVC CAP
8	4	1.8 2.5	core loss SILTY SAND			GROUT	8" BORE
10	- ñon	2.5	SILTY CLAY trace coarse sand, firm, olive brown	· · · · · · · · · · · · · · · · · · ·			
	1 -	1.5 2.5	some orange-brown color w	vith coarse sand ar	nd fine gravel,		4" PVC CASING
1:	4	2.0 2.5	more sandy				NAVASANA A"PVCC
T			brown mottling			17	//
Pi	roj. No	. 14929	A Woodward-0	Clyde Consult	tants	rig	U1 6 00

roje	ct:	ELCT Emery	RO COATINGS INC. ville, California	Log	of	Boring	No.10 (Continued)
Depth, Ft.	Samples	Fr. cored	MATERIAL D	ESCRIPTI	ON		WELL INSTALLATION DETAILS
16 -		1.8 2.0					
17 -		2.8 3.0	CLAYEY SAND gravelly, firm, wet, olive-brown				SAND SAND
19 -			SANDY SILTY CLAY firm, moist, orange mottling, green	nish-brown			
21 -	CORING	2.4 3.0	grading more brown				PEA GRAVEL
24 - 25 - 26 -	CONTINUOUS CORING	2.9 3.0	SILTY CLAY trace sand, firm, blue grading more clayey				BENTONITE SEAL
27 - 28 -		2.3 2.5	orange mottling predominant at	base of core			
29 - 30- 31 -		2.2 2.5	SILTY SANDY CLAY firm, orange mottling, blue grading to clayey fine SILTY CLAY firm, reddish-oran	nge mottling, t			DRILL CUTTINGS
32	4			oring @ 31.0			Figure 3b
Proj	. No	. 1492	9A Woodward	l-Clyde Co	nsulta	nts	

ate Dril	lled:	12/16	ville, California /80 to 12/18/80		f Boring	
ype of E	3orin	ig: <u>4 5/</u>	8" Pitcher Core Barrer	(See Legend Sh	eet for sampler ty	pes and hammer weights
<u>.</u> :		Ft. cored	MATERIAL	DESCRIPTION		WELL INSTALLATION DETAILS
Depth,		<u> </u>				PROTECTIVE CAP
			Surface Elevation:			
4			8" ASPHALT			
1 -			CLAY FILL medium soft, moist, black			- day 2 year
2]			becoming firmer with depth			
3						
4						
4 -		2.7	T			
5-		2.5	SILTY CLAY trace sand and fine angular gra dark gray	evels, brown-orange	mottling,	
6 -			core loss	12/29. 1/14/8	/80 Water Levels	1 N N N N N N N N N N N N N N N N N N N
7		1.3	grading more dark gree			GROUT S
8 -	9		core loss			
9 -	CONTINUOUS CORING		SANDY CLAY fine sand, trace angular grave dark green to green-blue	l, minor orange mot	itling, moist,	
10-	LINGON	2.8 3.0	grading to more olive green	increase	d orange mottling	
1 4	CON					
11 -	1		gravel more common			
1.7						一一 1 翼 2
12		2.3	SANDY SILTY CLAY			HEW W
13			trace gravel, orange mottling	, olive green		BENTONITE SEAL
14 -						AND BEA
1 +		2.0				
15-			increased fine gravel and or	ange mottling		SAND
	<u>.</u>	14929	Woodwa	ard-Clyde Cons	ultants	Figure 4a

Project: ELEC Emer	CTRO COATINGS INC. yville, California	Log of Boring	No.11 (Continued)
Somples St. racovered/F1, cored	MATERIAL	DESCRIPTION	WELL INSTALLATION DETAILS
33 - 34	SILTY CLAY trace sand, orange mottling,	some fat clay, bluish-gray	SDN
		Bottom of boring @ 34 feet	DRILL CUTTINGS
Proj. No. 1492		rard-Clyde Consultants	Figure 4c

	ELECTRO COA' Emeryville, Calif	ornia	Log Remarks:		Boring	No.12
ite Drilled	: 12/18/80 to 12/	/19/80				
type of Bori	ng: B Rotary		(See Legen	d Sheet	for sampler typ	pes and hammer weights)
Samples Samples	Ft. cored		RIAL DESCRIPT		WELL INSTALLATION DETAILS PROTECTIVE CAP	
<u></u> _	Surfac	e Elevation:				
1 - 2 - 3 - 4 - 5 - 6 - 7 - 8 - 9 - 9 -	FINE bro	SAND own to gray with angu- ragments of granite (?) ID brown grading to TY CLAY black TY CLAY gray LTY CLAY buff brown to blue-gr	lar chips of gray gravel (very rough drilling)	1	1 } Water Levels —	GROUT SEAL GROUT SEAL A** PVC CAP
10- 11 - 12 - 13 - 14 -		grading less gravel	d angular gravel, tan bro		ultants	SAND Sand Sald Sald Sald Sald Sald Sald Sald Sal
Dun i	No.14929A	1	Woodward-Clyde	Consi		

roject: ELEC	CTRO COATINGS INC. yville, California	Log of Boring	No. 12 (Continued)
Samples St. recovered/ Ft. cored		L DESCRIPTION	WELL INSTALLATION DETAILS
20 - 21 - 22 - 23 - 24 - 25 - 26 - 27 - 28 - 28 - 28 - 28 - 28 - 28 - 28	SAND AND GRAVEL coarse sand, fine gravel, brow	vn to grav	-IN
29	SILTY CLAY trace of fine sand, tan-br	own	FALL-IN
31 -	в	Sottom of boring @ 30 feet .	
		tward-Ctyde Consultants	Figure 5b

ate D	rille	d: 12/2	2/80	Remarks:		
ype o	f Bor	ing : 4 =	/8" Pitcher Core Barrel	(See Lenend Sheet	for sampler types	and hammer weights)
amme	Semples .	ight:_	MATERIAL	DESCRIPTION		WELL INSTALLATION DETAILS PROTECTIVE CAP
•		ŭ j	Surface Elevation:			
			4" ASPHALT		 .	
╡	Ì		SILTY CLAY FILL			
1 -			SILTY CLAY saft, moist			GROUT SEAL
3 +			core ioss			
4 -		2.0 2.5	grading more firm			The Process
6 - 7 - 8 - 8 -	9	2.5 2.5	trace fine gravels SILTY CLAY trace coarse sub rounded gravels stiff, slight moist, light olive	avel (≈20 mm)	Water Level	ENTONITE SEAL
9 -	CONTINUOUS CORING	2.4 2.5	becoming more tan brown,	increasing gravel and sand		NISO CONTRACTOR OF THE PROPERTY OF THE PROPERT
10-	1 E		increased mottling			
] ö		core loss			
11		1.6 2.5	SANDY SILTY CLAY trace gravel, orange mottli	ng around sand grains, tan-b	rown	PEA GRAVEL
13]	-	fine gravels common near core loss SANDY GRAVELLY CLAY			
14	1	2.3 2.5	SANDY SILTY CLAY	nottling common throughou , locally black-orange sand	·t,	

	0	DRY DENSITY {b/ft3	HOISTURE CONTERT & DRY WEIGHT	BLOW COUNT	SAMPLE	USCS	DESCRIPTION
	1 -						6" asphalt.
	2 –						_
	3 –						_
	4 –						_
	5 –			1	00001		Blue-gray clay, stiff, few quartz pebbles.
FEET	6 –			8 13	00002		penores.
DEPTH IN FEET	7 –						_
DEP	8 –						
	9 –	<u> </u>					
	10-			4 8	00003 00004		Blue-gray clay Brown sandy-silty clay, few pebbles
	11-			12			dry
	12-						
	13-	1					·

J. H. KLEINFELDER & ASSOCIATES CENTICHNICAL CONSULTANTS - MATERIALS TESTING

PREPARED DY: RJZ DATE: 5/10/82 5/10/82 DATE: CHECKED BY: DCM

ELECTRO-COATINGS INC. EMERYVILLE, CALIFORNIA LOG OF BORING NO. B-I

PROJECT NO. B-1132-3

PLATE

A-2

	14	DRY DENSITY 16/ft3	MOISTURE CONTENT TORY WEIGHT	BLOW COUNT	SAMPLE	uscs	DESCRIPTION
	15 — 16 —				00005		Tan-green sandy-silty clay, stiff, balling up on auger.
	17 — 18 —						
DEPTH IN FEET	20 - 21 - 22 -			5 6 6	00007. 00008		Gravelly clay with water bearing stringers, very wet, yellow water. Brown sandy clay, fat, moist, stiff.
	23 - 24 - 25 - 26 -			7 8 10	00009		Blue clay. Brown gravelly clay, saturated. Blue silty clay. Bottom of boring at 26½ ft.
	27						Hole abandoned.

^{*} Assumed base of shallow groundwater.

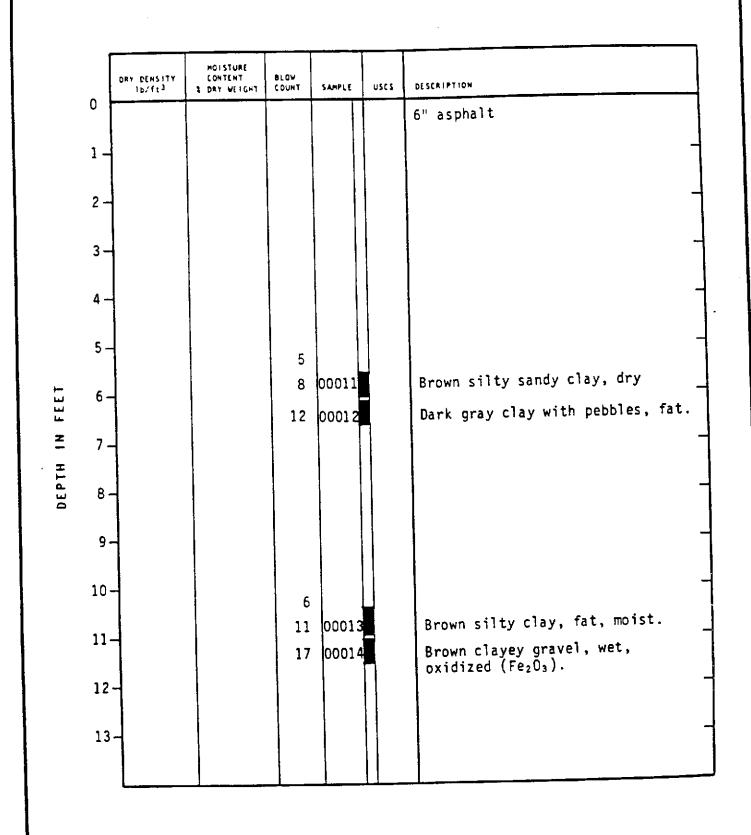
J.H KLEINFELDER & ASSOCIATES EMERYVILLE, CALIFORNIA

CIOTECHNICAL CONSULTANTS - MATERIALS TISTING

PREPARED BY: RJZ DATE: 5/10/82

CHECKED BY: DCM DATE: 5/10/82

PROJECT NO. B-1132-3



J.H. KLEINFELDER & ASSOCIATES GEOTECHNICAL CONSULTANTS . MATERIALS TESTING

ELECTRO-COATINGS INC. EMERYVILLE, CALIFORNIA LOG OF BORING NO. 14

A-3

PLATE

DATE: 5/10/82 PREPARED BY: RJZ 5/10/82 CHECKED BY: DCM DATE:

PROJECT NO. B-1132-3

	14	DRY DENSITY 16/1(3	MOISTURE CONTENT 2 DRY WEIGHT	BLOW COUNT	SAMPLE	uscs	DESCRIPTION
	14						
	15 –			5			_
	16 –			5	00015		Brown silty clay, fairly dry.
				6	00016		Mottled brown clay with extensive oxidation, fat, moist.
	17 –						
	18-						_
	19-	-					-
FEET	20 –			4			
z	21 -			5	00017		Brown clay with pebbles.
				9	00018		Green-gray clay.
DEPTH	22 -	_					
	23 -	4	1		į	1	-
	24 -	_					_
	25 -	_		,			-
				10	00019		
	26	-		12	1		Blue clay
	27						Bottom of boring at $26\frac{1}{2}$ ft. Well construction: 0-15', blank 4" PVC 15-25', perforated 4" PVC

^{*} Assumed base of shallow groundwater.

PLATE

I H. KLEINFELDER & ASSOCIATES
CIOTECHNICAL COMSULTANTS + MATERIALS TESTING

PREPARED BY: RJZ DATE: 5/10/82

CHECKED BY: DCM DATE: 5/10/82

PLATE

ELECTRO-COATINGS INC.
EMERYVILLE, CALIFORNIA

LOG OF BORING NO. 14 (CONT)

A-3.1

		DRY DEHSITY 16/ft3	MOISTURE CONTENT 1 DRY WEIGHT	BLOW COUNT	SAMPLE	uscs	DESCRIPTION
	0						4" asphalt, brown fill, fine grain and white crystalline material.
	1 -						-
	2 –						
	3 ~						
	4 –	1					Black clay
	5 -	-		5			
- - -	6 -			7	00021		4/16/82 Black clay with wood fragments.
<u>.</u> 2	7 -			10	00021	-	Brack Cray wron noos 11-5
ОЕРТИ	8 -						
0	9.						
	10			6			
	11	1			00022		Brown-gray clay with decayed organics, some iron stain, some
	12	4					pebbles, fat.
	13	_					

J.H. KLEINFELDER & ASSOCIATES CHOTECHNICAL CONSULTANTS . MATERIALS TESTING

PREPARED BY: RJZ DATE: 5/10/82 CHECKED BY: DCM DATE: 5/10/82 ELECTRO-COATINGS INC. EMERYVILLE, CALIFORNIA LOG OF BORING NO. 15

PROJECT NO. B-1132-3

PLATE

A-4

	14	DRY DENSITY 16/ft3	MOISTURE CONTENT & DRY WEIGHT	BLOW COUNT	SAMPLE	uscs	DESCRIPTION
	14					ļ	,
	15 –			5			-
	16 –			6 11	00023		Brown sandy-gravelly clay, fat.
	17 –						-
	18 –						Light brown sandy clay, saturated.
	19-						
FEET	20 -			11			_
z	21 -	<u> </u> 		14 27	00024		Brown sandy gravel, saturated.
DEPTH	22 -						_
	23 -						-
	24 -	_					
	25 -			9			-
	26			10 14		*	Brown clayey gravel, blue clay.
	27						Bottom of boring at 26½ ft. Well construction: 0-15', blank 4" PVC 15-25', perforated 4" PVC

^{*} Assumed base of shallow groundwater.

THE KLEINFELDER & ASSOCIATES EMERYVILLE, CALIFORNIA
LOG OF BORING NO. 15 (CONT)

PREPARED BY: RJZ DATE: 5/10/82

CHECKED BY: DCM DATE: 5/10/82

PROJECT NO. B-1132-3

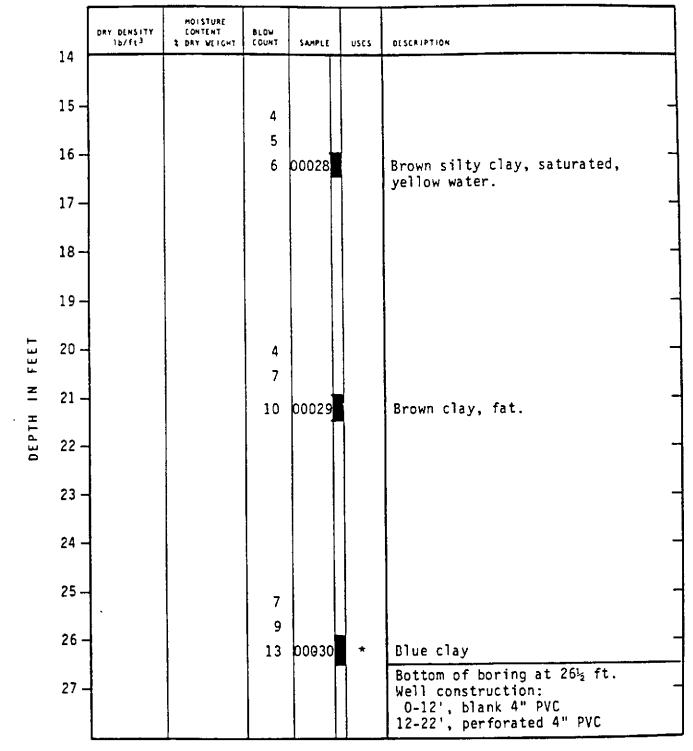
		DRY DENSITY 16/ft ³	HOISTURE CONTENT 3 DRY WEIGHT	BLOW COUNT	SAMPLE	USES	DESCRIPTION	
	0						8" asphalt.	1
	1 -							_
	2 –							-
	3 —							-
	4 —	<u> </u>				<u>~</u>	4/16/82 5/3/82	-
	5 –					=		
-	6 -			3 7				
- L				10	00026		Gray clay with roots, fat.	
DEPTH IN	7 -						·	
DEP	8 -							
	9 -							
	10-	-		4				
	11-			8	- {		Augers wet, brown pebbly clay,	
	12			12	00027		moist.	
	13	1						

J.H. KLEINFELDER & ASSOCIATES LOGIECHNICAL CONSULTANTS • MATERIALS TESTING LOG OF BORING NO. 16

PREPARED BY: RJZ DATE: 5/10/82

CHECKED BY: DCM DATE: 5/10/82

PROJECT NO. B-1132-3



^{*} Assumed base of shallow groundwater.

J.H KLEINFELDER & ASSOCIATES GEOTECHNICAL CONSULTANTS . MATERIALS TESTING



ELECTRO-COATINGS INC.
EMERYVILLE, CALIFORNIA
LOG OF BORING NO. 16 (CONT)

PLATE

A-5.1

PREPARED BY: RJZ DATE: 5/10/82 CHECKED BY: DCM DATE: 5/10/82

PROJECT NO. B-1132-3

	_	DRY DENSITY 10/4t3	MOISTURE CONTENT 2 DRY WEIGHT	BLOW COUNT	SAMPLE	uscs	DESCRIPTION
	0						Asphalt
	1 -						-
	2 –						-
	3 —					₹.	4/16/82
	4					<u>_</u>	Static water level after drilling 10 ft - also on 5/3/82.
	5 -	<u> </u> 					
FEET	6 -			6	00031		Blue-green clay with sand and
z	7 -	<u> </u> -					pebbles, fat
DEPTH	8 -						
	9 -	1					
	10-			4			
	11-	-		5	00032		Brown clay, saturated
	12-						
	13	-					
]] .	

J.H. KLEINFELDER & ASSOCIATES ELECTRO-COATINGS INC.

EMERYVILLE, CALIFORNIA

LOG OF BORING NO. 17

PREPARED BY: RJZ DATE: 5/10/82

CHECKED BY: DCM DATE: 5/10/82

PROJECT NO. B-1132-3

		DRY DENSITY	MOISTURE CONTENT & DRY WEIGHT	BLOW COUNT	SAMPLE	υsεş	DESCRIPTION
	14						
	15 –			6 9			
	16 –			13	00033		Brown gravelly clay, iron stain.
	17 —						
	18 –		i				
	19 –						
FEET	20 –			3 5			
DEPTH IN	21 -			8	00034		Brown silty clay
DEP	22 -	-					
	23 -	1					
	24 -				: : :		
	25 -			6			
	26			13		*	Blue clay Bottom of boring at 26½ ft.
	27						Well construction: 0-10', blank 4" PVC 10-20', perforated 4" PVC
		<u> </u>			!	11	20-25', blank 4" PVC

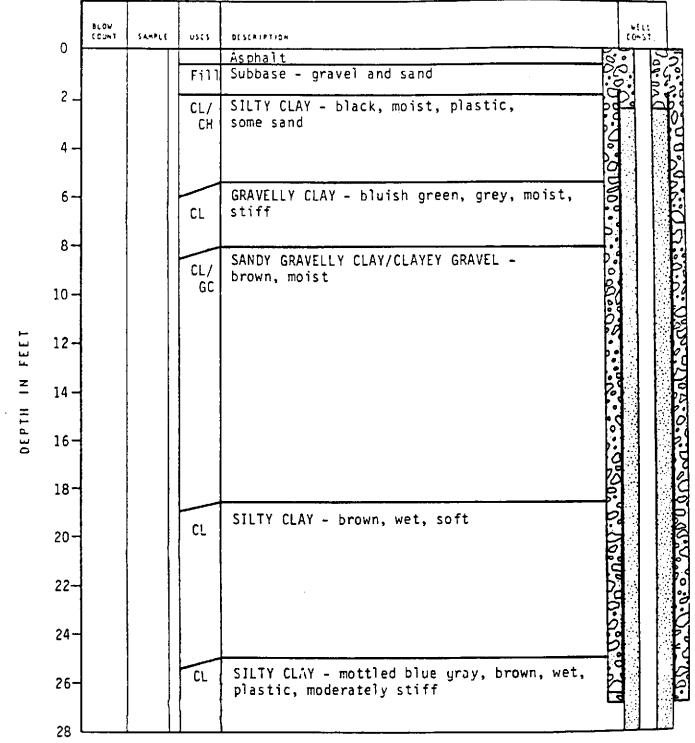
* Assumed base of shallow groundwater.

J.H. KLEINFELDER & ASSOCIATES EMERYVILLE, CALIFORNIA
LOG OF BORING NO. 17(CONT)

PREPARED BY: RJZ DATE: 5/10/82

CHECKED BY: DCM DATE: 5/10/82

PROJECT NO. B-1132-3



(1) 16" diameter hole drilled to 27', 10" casing set and annulus backfilled with bentonite/cement grout that was tremied in place.

JH KLEINFELDER & ASSOCI		ELECTRO-COATINGS INC. EMERYVILLE, CALIFORNIA LOG OF BORING NO. 18 A	7
PREPARED BY: AP DATE:	7/83		_
CHECKED BY: MLS DATE:	7/83	PROJECT NO. B-1132-4	

	ðn.	8104 (0JH)	SAMPLE	usts.	DESCRIPTION	WELL CONST.
	28			CL	Silty clay as before	BB
	30 -	53	1 2		CLAYEY SAND - brown, blue gray, wet, dense,	-88
	32-		2	SC	clay approx. 35%, silt approx. 10%, some fine gravel	
	34 —			ML	SILT - blue gray, mottled brown, wet, moderately stiff, non-plastic, some clay increasing with depth	
	36-	22	3 4			
					grading into	_ =
	38~		5		SILTY CLAY - blue gray, wet, very stiff, some sand and gravel	
FEET	40-			CL		
	42 -	53	5 6			
DEPTH IN	44-			SM	SILTY SAND - flowing sands 3 feet of heave up auger	
٥					, o yeer or meave up organ	
	45					
	48-		:	CL	SILTY CLAY - light olive gray, wet, stiff, slightly plastic to plastic, some high silt content zones	
	50	25	7			
	52-		8		Total depth of boring = 51.5 feet Logged by M. L. Siembieda	
	54-					

TH KLEINFELDER & ASSOCIATES CHOTCHSTON CONSUCTATION MATERIALS STATES

PREPARED BY: ΑP DATE: 7/83 CHECKED BY: MLS DATE: 7/83 ELECTRO-COATINGS INC. EMERYVILLE, CALIFORNIA

PROJECT NO.

LOG OF BORING NO. 18A

B-1132-4

7A

	28	DRY DENSITY lb/ft3	MOISTURE CONTENT & DRY WEIGHT	BLOW COUNT	SAMPLE	USES	DESCRIPTION
	28						
	29 –						-
	30 –			4	00039		Brown clay.
	31-			12			_
	J.			26	00040	ļ	Brown clayey sand, dry.
	32-	<u> </u>					Bottom of boring at 31½ ft.
	33-						Well construction: 0-15', blank 4" PVC 15-25', perforated 4" PVC
FEET	34-	<u> </u>					
<u>z</u>	35-						-
DEPTH	36-						_
	37-	 -					-
	38-	-					-
	39-	-					-
	40	-					-
	41	-					-

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1.H KLEINFELDER & ASSOCIATES GEOTECHNICAL CONSULTANTS + MATERIALS TESTING	ELECTRO-COATINGS INC. EMERYVILLE, CALIFORNIA LOG OF BORING NO. 18 (CONT)	A-7.2
PREPARED BY: RJZ DATE: 5/10/82		
CHECKED BY: DCM DATE: 5/10/82	PROJECT NO. B-1132-3	

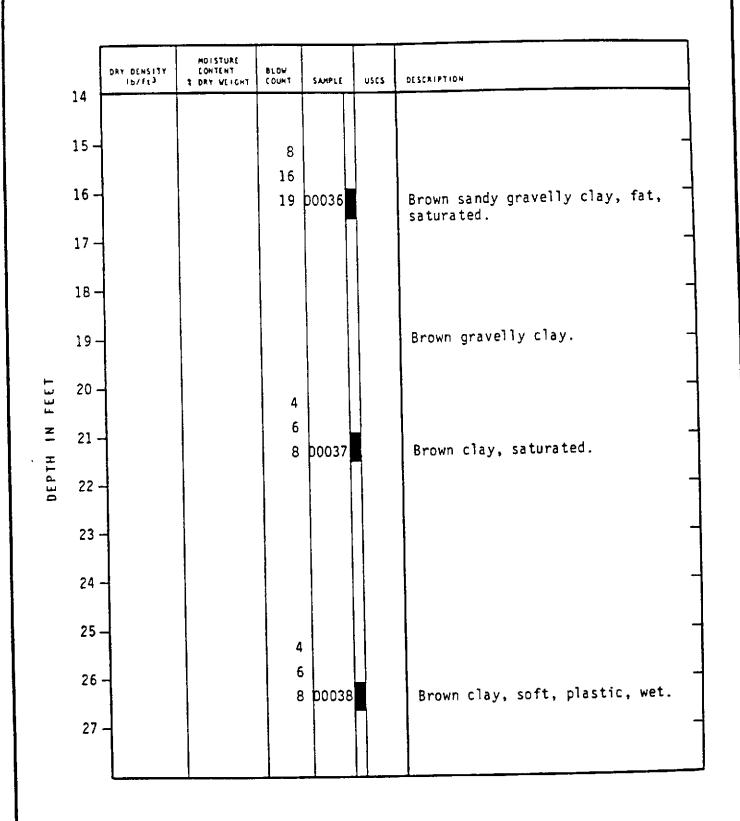
		DRY DENSITY	MOISTURE CONTENT & DRY WEIGHT	SLOW COUNT	SAMPLE	ยรตร	DESCRIPTION
	0						Asphalt
	1 -						†
	2 –						Gray silty clay.
	3					<u>_</u>	4/16/82
	4 –					₹	5/3/82
	5 –			3 7			
FEET	6 -	-		10	*		Black clay.
	7 -	1					Brown clay.
DEPTH IN	8 -						_
	9 -	<u> </u>					
	10-			10			Water on outside of Porter sampler.
	11-	-		14	*		Brown sandy clay, some iron stain, saturated.
	12	-					-
	13						-
						Ш	

I.H. KLEINFELDER & ASSOCIATES | ELECTRO-COATINGS INC. EMERYVILLE, CALIFORNIA | LOG OF BORING NO. 18

PREPARED BY: RJZ DATE: 5/10/82

CHECKED BY: DCM DATE: 5/10/82

PROJECT NO. B-1132-3



J.H. KLEINFELDER & ASSOCIATES CECTECHNICAL CONSULTANTS + MATERIALS TESTING

K

ELECTRO-COATINGS INC. EMERYVILLE, CALIFORNIA LOG OF BORING NO. 18 (CONT) PLATE

A - 7.1

PREPARED BY: RJZ DATE: 5/10/82 CHECKED BY: DCM DATE: 5/10/82

PROJECT NO. B-1132-3

	0	BLOY COUNT	SAMPLE	USCS	DESCRIPTION As phalt	WESS CON17.
	2 –			Fill	Subbase - sand & gravel SILTY CLAY - black, moist, plastic, stiff	
	4 —					
	6	22	1 2		becoming very dark gray	
	8 –					
	10 –	17	3	sc	CLAYEY SAND - brown, saturated, moderately dense, well graded, some fine gravel,	
FEET	12 –			СН	appreciable amount of fines SILTY CLAY - brown, wet, stiff, highly plastic	
DEPTH IN	14 - 16 -	15	4 5	CL	SILTY CLAY - brownish gray, mottled brown, wet, moderately stiff, moderately plastic, trace sand and gravel, silt content approx. 30%	HILLERIE
	18 ~					
2	20 -	13	6 7	6 Sand and gravel increase	sand and gravel increase	
	22 –					
	24 –				grading into SILTY CLAY - bluish gray, gray, wet, plastic	
	26 –	17	8 9	CL	to slightly plastic, stiff	
	28			<u></u>	Bottom of boring @ 26.5 ft. Logged by M. L. Siembieda 6/10/83	<u> </u>

FREPARED BY: MLS DATE: 7/83

ELECTRO-COATINGS INC.
EMERYVILLE, CALIFORNIA

LOG OF BORING NO. 19

PROJECT NO. B-1132-4

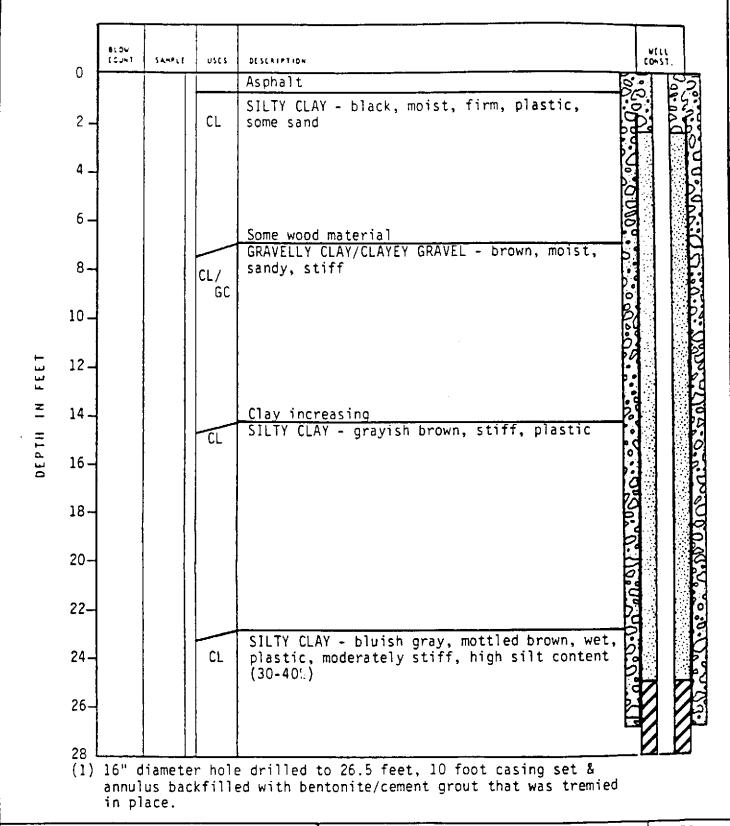


PLATE ELECTRO-COATINGS INC. I H KLEINFELDER & ASSOCIATES EMERYVILLE, CALIFORNIA CLOTECHNICAL CONSULTANTS + MATERIALS TESTING LOG OF BORING NO. 20 7/83 PREPARED DY: ΑP DATE: B-1132-4 CHECKED BY: MLS DATE: 7/83 PROJECT NO.

		BLOW				witt
	28	COUNT	SAMPLE	USCS	DESCRIPTION	C0451
	30 32	25	1 2	CL	Silty clay as before	17.1
	34-		3		Silt decreases to 20-10% highly plastic	
	36 38	21 3	4	Silt decreases to 20-10%, highly plastic		
F E T	40-	35	5	SC	CLAYEY SANDS - brown, saturated, dense, well	
Z	42-	33	5 6 7		graded, fines approx. 20%, gravel to ½" max., mostly angular	
DEPTH	44-	19	8 9	CL	GRAVELLY CLAY - gray, wet, stiff, plastic, gravel approx. 20% fine, well rounded	
	48-		11 12	SC	CLAYEY SAND- brown, saturated, moderately dense, high clay %, fine grained Grading into	
	50-	19		SP	SAND - brown, saturated, loose, poorly graded, medium grained, little fines	
	52-		12	CL	GRAVELLY CLAY - dark gray, stiff, plastic, silty, gravel approx. 20%	-1-
	54_		13			
	56	30	14		Rottom of horing @ 56.5 feet	

Bottom of boring @ 56.5 feet Logged by M.L. Siembieda

(1) 3.5 feet of slough in hole.

PLATE ELECTRO-COATINGS INC. JH KLEINFELDER & ASSOCIATES EMERYVILLE, CALIFORNIA CIDECHNICAL CONSULTANTS + MATERIALS TESTING 9A LOG OF BORING NO. 20 7/83 PREPARED BY: AΡ DATE: MLS DATE: 7/83 PROJECT NO. B-1132-4 CHECKED BY:

	0	BLOW THUCO	SAMPLE	USCS .	DESCRIPTION	WELL CONST.
				Fill	Railroad Roadbed – sand, gravel and silt, loose, dry	000
	2 –					
	4 _					
	6 -	16	1	СН	SILTY CLAY - gray, moist, firm,	
	8 _	22 2			highly plastic	
_	10 -					
			2	GW	SANDY GRAVEL - brown, saturated, dense,	
FEET	12 _				well graded, little fines, approx. 10%, gravel ½" max.	
. E	14					
DEPTH	16 –		3	CL	SILTY CLAY - greenish gray, wet, plastic,	
	18				stiff, high silt content, approx 35%	
	20 -					
	22	44 4 5	4 5	GC	CLAYEY GRAVEL - brown, saturated, dense,	-
		6			well graded, gravel to 1" max.	
	24 –		6	CL	SILTY CLAY - olive gray, bluish gray, stiff, wet, plastic, few gravel	
	26 –	24	6 7		Bottom of boring @ 26.5 feet	
	28				Logged by M.L. Siembieda 6/8/83	

J.H. KLEINFELDER & ASSOCIATES CONTINUES CONTIN PREPARED BY: ΑP DATE: 7/83 7/83 CHECKED BY: MLS DATE:

ELECTRO-COATINGS INC. EMERYVILLE, CALIFORNIA LOG OF BORING NO. 21

PROJECT NO. B-1132-4

10

	0	BLOW COUNT	SAMPLE	บริตร	DESCRIPTION	WELL CONST.
	2 _	14	1 2	CL	Asphalt SANDY SILTY CLAY (Fill) - blue gray, moist, slightly plastic, firm, sand approx. 35%	
	4 _	18	3 4	GC	CLAYEY GRAVEL (Fill) - black, dark greenish gray, brown, wet, wide range of materials	
	6 -	16	*	CL	SILTY CLAY - very dark gray, moist, stiff, plastic, sand 5-10%	
	8 -	23	5	ML/	- sand increases, mottled color, irridesence - few gravel SILTY CLAY - yellowish brown, moist, slightly	
	10 -	12	7 8	Cr Cr	plastic, firm, some fine sand & gravel Silt decreases - light yellowish gray, brown mottling, moderately plastic	
FEET	12 -				Bottom of boring 11.5 ft. Logged by M.L. Siembieda 6/8/83	
. H	14 –					
DEPTH	16 -					
	18 -					
	20 -					
	22 –					
	24 -					
	26 –					
	28					

J.H. KLEINFELDER & ASSOCIATES CONSULTANTS + MATERIALS TESTING	ELECTRO-COATINGS INC. EMERYVILLE, CALIFORNIA LOG OF BORING NO. 22	1 1
PREPARED BY: AP DATE: 7/83		
CHECKED BY: MLS DATE: 7/83	PROJECT NO. B-1132-4	

	•	BLOV COUNT	SAMPLE	uses	DESCRIPTION	WELL CONST.
	0			F:11	Asphalt	
	2 _	16	2	Fill CL	Subbase - sand and oravel SILTY CLAY - black, moist, stiff, plastic, trace fine sand	
	4 _	15	3 4		- some sandy zones - at 4.5' wood	
	6 -					
	8 -	18	5	CL	SILTY CLAY - greenish, bluish gray, moist, stiff, plastic, trace fine sand, some brown mottling	
	10 -		7			
		19	7 8	ML	CLAYEY SILT - light olive gray, moist, stiff	
FEET	12 _				Bottom of boring @ 11.5 ft. Logged by M.L. Siembieda	
. <u>z</u>	14 -					
DEPTH	16 –					
	18 -					
	20 –					
	22 _					
	24 -					
	26 -					
	28					

PREPARED D' AP DATE: 7/83

CHECKED BY MLS DATE: 7/83

PROJECT NO. B-1132-4

	٥	Blow/ F1.	Sample No.	USCS	DESCRIPTION	CON
	2_	70	S-2.0 A24	GM	SILTY GRAVEL -Brown -dry -serpentine gravels, angular to 2" diameter(fill) -very hard	000000000000000000000000000000000000000
	3_ 4_	17	S-4.0	ML	CLAYEY SILT -Black -medium permeability -moist -low plasticity -stiff -serpentine gravels, angular to l" diameter	00000
	5- 6-	24	S-6.0 A24	СН	CLAY -Dark gray to olive gray -moist -stiff -low permeability -high plasticity	\$0.00°
	8- 9- 0-	34	S-9.0 A24	CL	SILTY CLAY -Blue/green with some brown mottles -moist -some silt -very stiff -low permeability	00000000000000000000000000000000000000
	1-	25	S-11.0 A24		-medium plasticity -some root holes NFWE TD of Boring 11.5' Logged by Mark Klaver 1/9/85) 0 C
	3-				Logged by Mark Klaver 179765	
٦	5-					

JH KLEINFELDER & ASSOCIATES CEOTECHNICAL CONSULTANTS - MATERIALS TESTING PROJECT NO. B-1132-5

ELECTRO - COATINGS EMERYVILLE, CALIFORNIA LOG OF BORING NO. 24

0	Blow/ Ft.	Sample No.	USCS	DESCRIPTION	CONS
0_	-			Concrete	0.0
1_			CL	SILTY CLAY -Black	0.00
2_	44	S-2.0		-moist -hard -trace subrounded gravels to 2" dia (fill)	0.0
3_		A25		<pre>-trace fine sand (oxidized mottles) -low plasticity -medium permeability</pre>	-00
4_				CLAY -Black -moist	00.0
5-	42	S-4.0 A25	CL	-hard -low permeability -high plasticity	0.00
6-				CLAY -Dark gray -moist	0 0 0
	20	S-6.0 A25	CL	-firm -Trace gravels subrounded to 1.5" diameter	00
7-				-low permeability -medium plasticity	
8-				SILTY CLAY	
9-				-Blue/green w light brown mottling -moist	7.0
	26	S-9.0 A25		-very firm -low permeability	20
10-	1			-medium plasticity	
11-					0.00
	18	5-11.0 A25		TD of boring 11.5'	377
12-	1			NFWE Logged by Mark Klaver 1/9/85	
13-	-			Logged by Mark Kraver 173703	
14-	1				
15					
, 5					

J.H. KLEINFELDER & ASSOCIATES CEDIECHMICAL CONSULTANTS + MATERIALS TESTING PROJECT NO. B-1132-5

ELECTRO - COATINGS EMERYVILLE, CALIFORNIA LOG OF BORING NO. 25

0_	Blow/ F1.	Sample No.	USCS	DESCRIPTION	CONS
1_				Abandoned clay gas pipe sand backfill -No sample recovery	0.00000
3_ 4_	17	5-4.0	CL	SILTY CLAY -Black -low permeability -moist -medium plasticity -firm -trace gravels angular to l" diameter	0.0000000000000000000000000000000000000
5- 6- 7-	21	A26 S-6.0 A26	СН	CLAY -Gray -moist -firm -trace angular gravel to }" diameter -low permeability -high plasticity	
9- 10-	28	A-9.0 A26	CL	CLAY -Tan w/ gray mottling -moist -stiff -some silt stringers -low permeability -medium plasticity	
11-	24	S-11.0 A26		TD of boring 11.5' Logged by Mark Klaver 1/9/85	10:0
14-					
15-					

JH KLEINFELDER & ASSOCIATES GEOTECHNICAL CONSULTANTS + MATERIALS TESTING PROJECT NO. B-1132-5

ELECTRO - COATINGS EMERYVILLE, CALIFORNIA LOG OF BORING NO. 26

0_	Blon/ Ft.	Sample No.	USCS	DESCRIPTION	CON
1_	13	5-2.0 A27	ML	CLAYEY SANDY SILT -Black -medium permeability -dry -soft/loose -some wood fragments -some fine sand	000000000000000000000000000000000000000
3- 4- 5-	30	5-4.0 A27	SM	SILTY SAND -Brown -moist -dense -fine to medium sand -some wood fragments -trace angular gravel to ½" diameter	000000
6-	46	S-6.0 A27	СН	CLAY -Dark gray -moist -hard -some root holes -redwood fragments (minor)-high plasticity	\$ 02.00
8- 9- 10-	34	S-9.0 A27	CL	CLAYEY SILT -Gray w/tan mottling -moist -stiff -some fine sand -root holes -low permeability -medium plasticity	
11-	34	S-11.0 A27		GRAVELLEY CLAYEY SILT -Gray w/orange and black mottling -moist -stiff -medium plasticity -medium permeability	
12-				NFWE TD of boring 11.5' Logged by Mark Klaver 1/9/85	
14-					
15-	1				

I H KLEINFELDER & ASSOCIATES CEDIECHNICAL CONSULTANTS . MATERIALS TESTING

B-1132-5

ELECTRO - COATINGS EMERYVILLE, CALIFORNIA LOG OF BORING NO. 27 PLATE

PROJECT NO.

0	Blow/ Ft.	Sample No.	USCS	DESCRIPTION	CON
1_	19	S-2.0	SM	SILTY SAND -Black w/green-gray sand -medium dense -moist -high permeability -low plasticity	0.5.0000000000000000000000000000000000
3_ 4_ 5_	-	S-4.0 A28	SC	CLAYEY SAND -Upper 6" green sand -loose *Sampler refusal @ 4.5' hard rock	0.00000
6	18	5-6.0 A28	СН	CLAY -Black -moist -firm -low permeability -high plasticity	
7- 8- 9-	19	S - 9. O	ML	CLAYEY SILT -Gray w/some orange mottling -green sand stringer 9.0-9.3' -moist -firm -medium permeability -low plasticity	
10-	וו	A28 S - 11.0 A28		SILTY CLAY -Gray -moist to wet -soft -low permeability - medium plasticity	
12-				TD of Boring 11.5' Logged by Mark Klaver 1/9/85	
14-				·	
15-					

1 H KLEINFELDER & ASSOCIATES
GEOTECHNICAL CONSULTANTS - MATERIALS TESTING

ELECTRO - COATINGS EMERYVILLE, CALIFORNIA LOG OF BORING NO. 28 PLATE

PROJECT NO. B-1132-5

0	Blow/ Ft.	Sample No.	USCS	DESCRIPTION	CON
1_	18	S-2.0 A29	SP	SAND -Green w/tan mottles -moist -medium dense -fine to medium sand -high permeability	0.50000
3_ 4_ 5_	19	5-4.0	CL	SILTY CLAY -Gray -moist -firm -trace fine sand -low permeability -medium plasticity	000000
6- 7-	15	5-6.0 AZ9	Сн	CLAY -Black -moist -firm -trace fine sand -low permeability -high plasticity	1000 Co.
8- 9-	24	S - 9. 0	ML	CLAYEY SILT -Gray w/ orange mottling -moist -firm -medium permeability-low plasticity	المحمد المحمد
10-	23	5-11.0 A29	GP	SANDY GRAVEL -Varicolored gravels w/ brown sand -angular gravels to ½"diameter -medium to coarse sand -medium dense -high permeability	
12-				TD of boring 11.5' Logged by Mark Klaver 1/9/85	
13- 14-					
15-			 		

1 H. KLEINFELDER & ASSOCIATES GEOTECHNICAL CONSULTANTS + MATERIALS TESTING

B-1132-5

ELECTRO - COATINGS EMERYVILLE, CALIFORNIA LOG OF BORING NO. 29 PLATE

PROJECT NO.

0_	Blow/ F1.	Sample No.	USCS	DESCRIPTION	CON
1_	17	5-2.0	SP	SILTY SAND -Brown w/ green sand lenses -moist -loose -fine to medium sand -trace fine gravel -medium permeability	000000
3_	19	A30 5-4.0	ML	SANDY SILT -Greenish gray -moist -loose -medium permeability	000000
5-	12	A30 S-6.0	СН	CLAY -Black -moist -soft -low permeability -high plasticity	S.000
7- 8- 9-	21	A30 S-9.0 A30	CL	SILTY CLAY -Gray w/orange mottles -moist -slightly stiff -trace fine sand -low permeability -medium plasticity	
10-	18	S -11 .0 A30	ML	CLAYEY SILT -Gray -low permeability -moist -low plasticity -firm -trace root holes	
12- 13-				TD of Boring 11.5' Logged by Mark Klaver 1/9/85	
14-					
15-					

1 H KLEINFELDER & ASSOCIATES CEOTECHNICAL CONSULTANTS + MATERIALS TESTING

B-1132-5

ELECTRO - COATINGS EMERYVILLE, CALIFORNIA LOG OF BORING NO. 30 PLATE

PROJECT NO.

٥	Blow/ Ft.	Sample No.	USCS	DESCRIPTION
2_	31	S-2.0	SW	Concrete 4" GRAVELLEY CLAYEY SAND -Black w/green sand stringers -moist -dense -high permeability
) — 1 — 5 —	18	5 - 4 .0 A31	ML	SANDY SILT -Black w/green sand stringers -moist -loose to medium dense -medium - high permeability
	18	S-6.0	CL	CLAY -Black -medium plasticity -moist -firm -trace fine sand -low permeability
	26	s-9.0	ML	SANDY SILT -Light gray -slightly wet -firm -trace fine sand and fine gravels -medium permeability
	15	A31 S-11.0 A31	CL	GRAVELLEY SILTY CLAY -Gray w/orange and black mottles -wet -firm -medium to high permeability -gravel lens at 10.5'-11.0'
				TD of Boring 11.5' Logged by Mark Klaver 1/9/85
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H. KLEINFELDER & ASSOCIATES CEOTECHNICAL CONSULTANTS + MATERIALS TESTING

ELECTRO - COATINGS EMERYVILLE, CALIFORNIA LOG OF BORING NO. 31 PLATE

PROJECT NO. B-1132-5