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Alan McKay

Peter Kaldveer and Associates, Inc.
Geotechnical Consultants

425 ROLAND WAY, OAKLAND, CALIFORNIA 94621, 415/568-4001

Peter Kaldveer, PE.
President
Richard Short, PE.
Executive Vice President
Ronald Bajuniemi, PE.
Vice President Engineering

Patrick Stevens, PE.
Michael McRae, PE
Dawn Rinaldi, PE

August 15, 1986
K405-35, 08759

Blymyer and Sons
1829 Clement Avenue
Alameda, California 94501-1396

Attention: Mr. Chris Falbo

Shellmound St.

RE: SOIL AND WATER TESTING
PIE TRUCK TERMINAL
EMERYVILLE, CALIFORNIA
/ Alameda

Gentlemen:

In this letter report we present the results of our soil and water testing program at the existing PIE truck terminal. The terminal is bounded by Shellmound Street and the Eastshore Freeway, Interstate Highway 80, to the east and west, respectively and due south of Powell Street in Emeryville, California, as shown on the Site Plan, Figure 1. The purpose of our testing program was to determine the quantities, if any, of diesel fuel and other associated petroleum products in the subsurface soils and free groundwater.

SITE INVESTIGATION

A. Subsurface Investigation

A subsurface investigation was performed using a truck-mounted 8-inch diameter hollow stem auger to investigate and sample the subsurface soils. Seven exploratory borings were drilled on August 11, 1986, to depths where the groundwater level was encountered, or a maximum depth of 15 feet. All drilling equipment was steam cleaned prior to drilling.

The approximate location of the seven borings are shown on the Site Plan, Figure 1. Logs of the borings and details regarding the field investigation are included in Appendix A.

B. Soil and Groundwater Sampling

Three soil samples were obtained from each boring above or near the vicinity of the free groundwater level encountered at the time of drilling, with the exception of Boring 2 where two soil samples were obtained. The soil samples were contained in cleaned, 6-inch long 2.5-inch O.D. brass liners with teflon paper capping. The sampler and brass liners were

cleaned with trisodium phosphate wash water and rinsed in distilled water prior to each sampling. The water samples were placed in 40 ml glass vials.

The twenty soil and seven water samples were stored on ice until delivery to the Brown and Caldwell chemical laboratory for testing.

SITE CONDITIONS

A. Surface

The existing structures at the PIE Truck Terminal consisted of an office building and connected dock-high freight terminal to the north and an L-shaped service building at the south end of the site. At the time of the field investigation, several excavations were located immediately northwest and northeast of the "leg" of the service building. These excavations were made during the removal of underground diesel and oil tanks and extended to depths of approximately 9 to 10 feet. Water was observed in these excavations and a film of oil-like products was observed on the groundwater surface. In addition, a shallow excavation approximately 2 feet deep was present in the former fuel island location north of the "leg" of the service building. A reinforced concrete slab surrounded the northeast, north and northwest sides of the service building. The remainder of the site was covered with approximately 3 inches of asphaltic concrete.

B. Subsurface

The subsurface soils underlying the asphaltic concrete and concrete slabs generally consisted of fill materials comprised of silty and sandy clays and sands with variable amounts of fines with a few lenses of gravels. Small amounts of debris such as wood, brick and wire were encountered within the fill materials. However, in Boring 2, the fill materials extended to a depth of about 10 feet and was underlain by native sands.

Detailed descriptions of the soils encountered in each of the exploratory borings are presented in Appendix A. The attached boring logs and related information depict subsurface conditions only at the specific locations shown on the Site Plan and on the particular date designated on the logs. Also, the passage of time may result in changes in the subsurface conditions due to environmental changes. The locations of the borings were approximately determined by pacing and should be considered accurate only to the degree implied by the method used.

C. Groundwater

Free groundwater was encountered in all the borings at depths of 7 to 14 feet at the time of drilling. All borings were backfilled immediately after

drilling, which was not a sufficient time to establish an equilibrium groundwater level. Fluctuations in groundwater level could occur due to change in seasons, variations in rainfall, tides and other factors.

SOIL AND GROUNDWATER TEST RESULTS

The complete results of the chemical laboratory tests run on the twenty soil samples and seven water samples are presented in the attached Appendix B. The chemical testing performed on each soil and water samples consisted of a Nonhalogenated Volatile Organics test, EPA Test Method 8015. This test was run in compliance with the EPA Manual SW-846, "Test Methods for Evaluating Solid Wastes".

Our services were performed in accordance with generally accepted soil and environmental engineering principles and practices.

If you have any questions, please call.

Very truly yours,

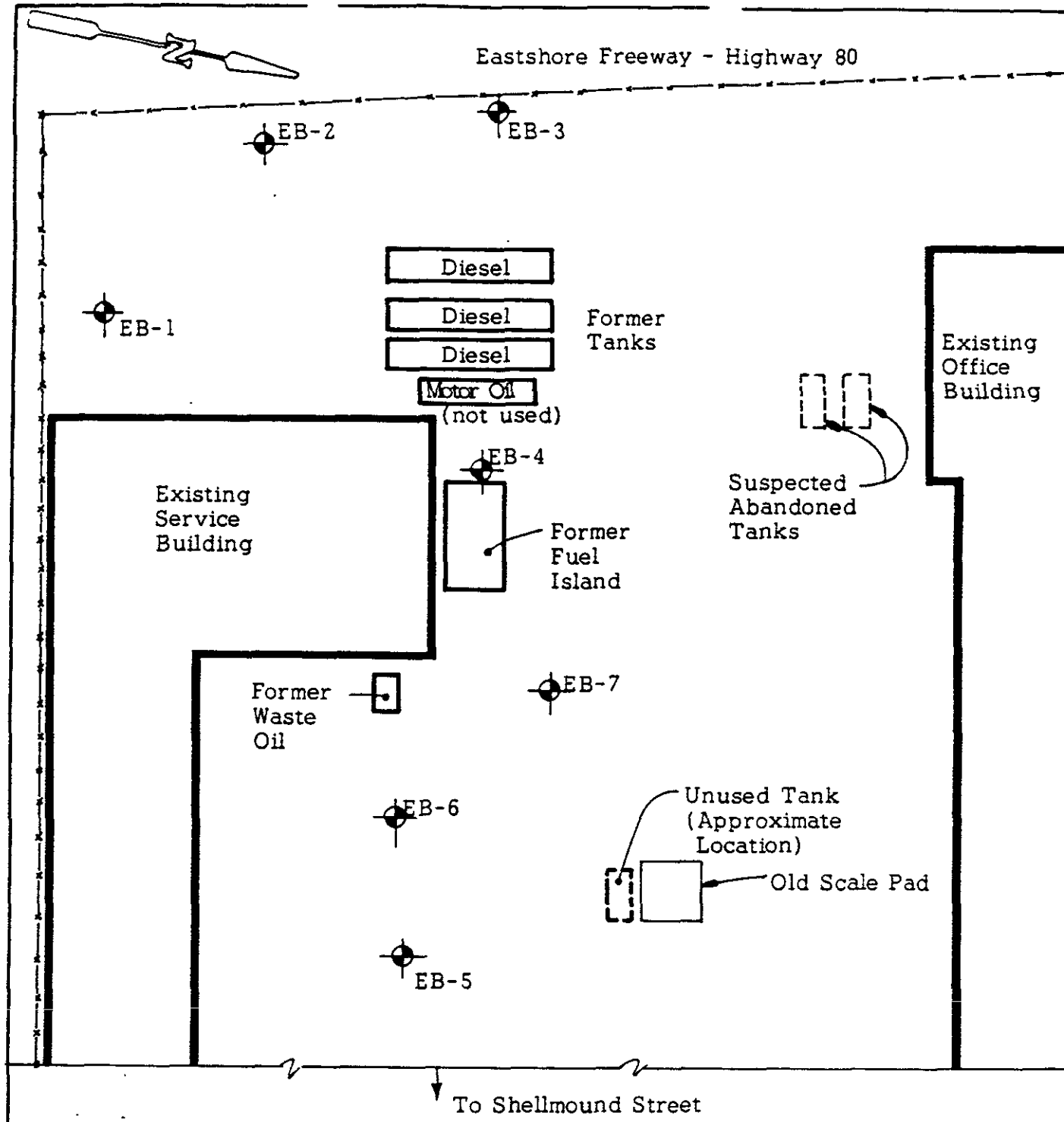
PETER KALDVEER AND ASSOC., INC.

Dawn Rinaldi

Dawn Rinaldi, P.E.

Peter Kaldveer

DR/PK:jb
Copies: Addressee (3)



LEGEND

EB-7 — Approximate Location of Exploratory Boring

APPROXIMATE SCALE (Feet)

0 50 100

Base: Untitled, undated Parcel Plan provided by Blymer & Sons.

PETER KALDVEER
AND ASSOCIATES, INC.
Geotechnical Consultants

SITE PLAN

SOIL AND GROUNDWATER TESTING
PIE TRUCK TERMINAL
Emeryville, California

PROJECT NO	DATE	Figure
K405-35	August 1986	1

APPENDIX A - FIELD INVESTIGATION

The field investigation consisted of a subsurface exploration program using a truck-mounted, continuous hollow stem flight auger. Seven 8-inch diameter exploratory borings were drilled on August 11, 1986, to a maximum depth of 15 feet. The locations of the exploratory borings are shown on the Site Plan, Figure 1. The soils encountered in the borings were continuously logged in the field by our representative. The soils are described in accordance with the Unified Soil Classification System (ASTM D-2487). The logs of the borings as well as a key for the classification of the soil (Figure A-1) are included as part of this appendix.

The soil samples were obtained from the exploratory borings at selected depths appropriate to this soil and water testing program. Samples were obtained using a 3-inch O.D. Modified California sampler. All samples were transmitted on ice to the Brown and Caldwell Analytical Laboratory for appropriate testing. The sampler type is indicated in the "Sampler" column of the boring logs as designated below:

☒ Modified California

The attached boring logs and related information show our interpretation of the subsurface conditions at the dates and locations indicated, and it is not warranted that they are representative of subsurface conditions at other locations and times.

PRIMARY DIVISIONS			GROUP SYMBOL	SECONDARY DIVISIONS
COARSE GRAINED SOILS — MORE THAN HALF OF MATERIAL IS LARGER THAN NO 200 SIEVE SIZE	GRAVELS — MORE THAN HALF OF COARSE FRACTION IS LARGER THAN NO 4 SIEVE	CLEAN GRAVELS (LESS THAN 5% FINES)	GW	Well graded gravels, gravel-sand mixtures little or no fines.
			GP	Poorly graded gravels or gravel-sand mixtures, little or no fines.
		GRAVEL WITH FINES	GM	Silty gravels gravel-sand-silt mixtures non-plastic fines
			GC	Clayey gravels gravel-sand-clay mixtures, plastic fines
	SANDS — MORE THAN HALF OF COARSE FRACTION IS SMALLER THAN NO 4 SIEVE	CLEAN SANDS (LESS THAN 5% FINES)	SW	Well graded sands gravelly sands little or no fines
			SP	Poorly graded sands or gravelly sands, little or no fines
		SANDS WITH FINES	SM	Silty sands sand-silt mixtures non-plastic fines
			SC	Clayey sands, sand-clay mixtures, plastic fines
FINE GRAINED SOILS — MORE THAN HALF OF MATERIAL IS SMALLER THAN NO 200 SIEVE SIZE	SILTS AND CLAYS — LIQUID LIMIT IS LESS THAN 50%		ML	Inorganic silts and very fine sands, rock flour silty or clayey fine sands or clayey silts with slight plasticity
			CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays.
			OL	Organic silts and organic silty clays of low plasticity.
	SILTS AND CLAYS — LIQUID LIMIT IS GREATER THAN 50%		MH	Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts
			CH	Inorganic clays of high plasticity, fat clays.
			OH	Organic clays of medium to high plasticity, organic silts
HIGHLY ORGANIC SOILS			Pt	Peat and other highly organic soils

DEFINITION OF TERMS

	U.S. STANDARD SERIES SIEVE				CLEAR SQUARE SIEVE OPENINGS			
	200	40	10	4	3/4"	3"	12"	
SILTS AND CLAYS	SAND			GRAVEL		COBBLES	BOULDERS	
	FINE	MEDIUM	COARSE	FINE	COARSE			

GRAIN SIZES

SANDS AND GRAVELS	BLOWS/FOOT†
VERY LOOSE	0 - 4
LOOSE	4 - 10
MEDIUM DENSE	10 - 30
DENSE	30 - 50
VERY DENSE	OVER 50

RELATIVE DENSITY

SILTS AND CLAYS	STRENGTH‡	BLOWS/FOOT†
VERY SOFT	0 - 1/4	0 - 2
SOFT	1/4 - 1/2	2 - 4
FIRM	1/2 - 1	4 - 8
STIFF	1 - 2	8 - 16
VERY STIFF	2 - 4	16 - 32
HARD	OVER 4	OVER 32

CONSISTENCY

†Number of blows of 140 pound hammer falling 30 inches to drive a 2 inch O.D. (1-3/8 inch I.D.) split spoon (ASTM D-1586)

‡Unconfined compressive strength in tons/sq. ft. as determined by laboratory testing or approximated by the standard penetration test (ASTM D-1586) pocket penetrometer, torvane or visual observation

PETER KALDVEER
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KEY TO EXPLORATORY BORING LOGS
Unified Soil Classification System (ASTM D-2487)

SOIL AND GROUNDWATER TESTING
PIE TRUCK TERMINAL
Emeryville, California

PROJECT NO	DATE	Figure
K405-35	August 1986	A-1

DRILL RIG Hollow Stem Auger	SURFACE ELEVATION --	LOGGED BY DYR
DEPTH TO GROUNDWATER 8½' (see note 2)	BORING DIAMETER 8 Inches	DATE DRILLED 8/11/86

DESCRIPTION AND CLASSIFICATION				DEPTH (FEET)	SAMPLER	PENETRATION RESISTANCE (BLOWS/FT)	WATER CONTENT (%)	DRY DENSITY (PCF)	UNCONFINED COMPRESSIVE STRENGTH (KSF)
DESCRIPTION AND REMARKS	COLOR	CONSIST.	SOIL TYPE						
3½" AC									
SAND (fine grained), some clay and silt	tan		SM-SC	1 2 3					
CLAY, silty with sand, glass, nail, wood (FILL)	grey black		CL	4 5 6 7 8 9					
Bottom of Boring = 9½ Feet				10 11 12 13 14 15 16 17 18 19 20					

Notes:
1. The stratification line represents the approximate boundary between soil types and the transition may be gradual.
2. Groundwater level was measured at time of drilling.

PETER KALDVEER AND ASSOCIATES, INC.
Geotechnical Consultants

EXPLORATORY BORING LOG

SOIL AND GROUNDWATER TESTING
PIE TRUCK TERMINAL
Emeryville, California

PROJECT NO	DATE	BORING NO
K405-35	August 1986	1

DRILL RIG Hollow Stem Auger	SURFACE ELEVATION --	LOGGED BY DYR
DEPTH TO GROUNDWATER 10½' (see note 2)	BORING DIAMETER 8 Inches	DATE DRILLED 8/11/86

DESCRIPTION AND CLASSIFICATION				DEPTH (FEET)	SAMPLER	PENETRATION RESISTANCE (BLOWS/FT)	WATER CONTENT (%)	DRY DENSITY (PCF)	UNCONFINED COMPRESSIVE STRENGTH (KSF)
DESCRIPTION AND REMARKS	COLOR	CONSIST.	SOIL TYPE						
3" AC									
SAND, with some silt and clay, gravels	olive		SM	1–5					
SAND, with clay some gravels pieces of concrete (FILL)	grey		SC	5–8	X				
(grading with less silt and without gravels and clay) (shells, organics)			SM	8–12	X		▽		
Bottom of Boring = 12½ Feet				13–20					

Notes:
1. The stratification line represents the approximate boundary between soil types and the transition may be gradual.
2. Groundwater level was measured at time of drilling.

PETER KALDVEER AND ASSOCIATES, INC.
Geotechnical Consultants

EXPLORATORY BORING LOG

SOIL AND GROUNDWATER TESTING
PIE TRUCK TERMINAL
Emeryville, California

PROJECT NO	DATE	BORING NO
K405-35	August 1986	2

DRILL RIG Hollow Stem Auger	SURFACE ELEVATION --	LOGGED BY DYR
DEPTH TO GROUNDWATER 14' (see note 2)	BORING DIAMETER 8 Inches	DATE DRILLED 8/11/86

DESCRIPTION AND REMARKS	COLOR	CONSIST.	SOIL TYPE	DEPTH (FEET)	SAMPLER	PENETRATION RESISTANCE (BLOWS/FT)	WATER CONTENT (%)	DRY DENSITY (PCF)	UNCONFINED COMPRESSIVE STRENGTH (KSF)
3" AC									
SAND , gravelly, some silt	brown		SM	1 2 3 4 5					
SAND , with some gravel and some clay and silt	grey		SC-SM	6 7					
(grading without clay and less silt)			SM	8 9					
(grading with more silt and clay)			SC-SM	10 11 12 13 14					
(FILL)				15					
Bottom of Boring = 15 Feet				16 17 18 19 20					

Notes:
1. The stratification line represents the approximate boundary between soil types and the transition may be gradual.
2. Groundwater level was measured at time of drilling.

PETER KALDVEER
AND ASSOCIATES, INC.
Geotechnical Consultants

EXPLORATORY BORING LOG

SOIL AND GROUNDWATER TESTING
PIE TRUCK TERMINAL
Emeryville, California

PROJECT NO	DATE	BORING NO
K405-35	August 1986	3

DRILL RIG	Hollow Stem Auger	SURFACE ELEVATION	--	LOGGED BY	DYR
DEPTH TO GROUNDWATER	8' (see note 2)	BORING DIAMETER	8 Inches	DATE DRILLED	8/11/86

DESCRIPTION AND CLASSIFICATION				DEPTH (FEET)	SAMPLER	PENETRATION RESISTANCE (BLOWS/FT)	WATER CONTENT (%)	DRY DENSITY (PCF)	UNCONFINED COMPRESSIVE STRENGTH (KSF)
DESCRIPTION AND REMARKS	COLOR	CONSIST.	SOIL TYPE						
SAND, some silt, trace of clay	tan		SM	1					
				2					
CLAY, sandy	grey black		CL	3					
				4					
				5					
SAND (fine grained), trace of silt	grey		SM	6					
CLAY, sandy	grey black		CL	7					
				8			▽		
				9					
				10					
(FILL)				11					
				12					
Bottom of Boring = 12 Feet				13					
				14					
Notes:				15					
1. The stratification lines represent the approximate boundaries between soil types and the transition may be gradual.				16					
2. Groundwater level was measured after drilling.				17					
				18					
				19					
				20					

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Geotechnical Consultants

EXPLORATORY BORING LOG

SOIL AND GROUNDWATER TESTING
PIE TRUCK TERMINAL
Emeryville, California

PROJECT NO	DATE	BORING NO
K405-35	August 1986	4

DRILL RIG Hollow Stem Auger	SURFACE ELEVATION --	LOGGED BY DYR
DEPTH TO GROUNDWATER 7' (see note 3)	BORING DIAMETER 8 Inches	DATE DRILLED 8/11/86

DESCRIPTION AND REMARKS	COLOR	CONSIST.	SOIL TYPE	DEPTH (FEET)	SAMPLER	PENETRATION RESISTANCE (BLOWS/FT)	WATER CONTENT (%)	DRY DENSITY (PCF)	UNCONFINED COMPRESSIVE STRENGTH (KSF)
3" AC									
SAND, some silt and gravels	tan		SM	1					
SAND, some silt and clay (lenses with gravels)	grey		SM	2 3 4 5	X	24*			
GRAVEL (fine-coarse grained), with clay	grey brown		GC	6 7	X		▽		
CLAY, very sandy (FILL)	dark grey		SC-CL	8 9	X				
Bottom of Boring = 9½ Feet Notes: 1. The stratification lines represent the approximate boundaries between soil types and the transition may be gradual. 2. For an explanation of penetration resistance values marked with an asterisk (*) see first page, Appendix A. 3. Groundwater level was measured at time of drilling.				10 11 12 13 14 15 16 17 18 19 20					

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AND ASSOCIATES, INC.
Geotechnical Consultants

EXPLORATORY BORING LOG

SOIL AND GROUNDWATER TESTING
PIE TRUCK TERMINAL
Emeryville, California

PROJECT NO	DATE	BORING NO
K405-35	August 1986	5

DRILL RIG	Hollow Stem Auger	SURFACE ELEVATION	--	LOGGED BY	DYR
DEPTH TO GROUNDWATER	9' (see note 2)	BORING DIAMETER	8 Inches	DATE DRILLED	8/11/86

DESCRIPTION AND REMARKS	COLOR	CONSIST	SOIL TYPE	DEPTH (FEET)	SAMPLER	PENETRATION RESISTANCE (BLOWS/FT)	WATER CONTENT (%)	DRY DENSITY (PCF)	UNCONFINED COMPRESSIVE STRENGTH (KSF)
3" AC									
SAND with silt, some clay and pea gravels	brown		SM	1					
CLAY, silty, sandy	grey black		CL	2					
				3					
GRAVEL, sandy, some clay	tan		GC	4					
CLAY, sandy, brick, gravels	grey black		CL-SC	5					
SAND, some silt, wood	black		SM	6					
CLAY, sandy (wood, debris)	olive brown		CL	7					
(FILL)				8					
				9			▽		
				10					
Bottom of Boring = 10 Feet				11					
				12					
				13					
				14					
				15					
				16					
				17					
				18					
				19					
				20					

Notes:
1. The stratification lines represent the approximate boundaries between soil types and the transition may be gradual.
2. Groundwater level was measured after drilling.

PETER KALDVEER
AND ASSOCIATES, INC.
Geotechnical Consultants

EXPLORATORY BORING LOG

SOIL AND GROUNDWATER TESTING
PIE TRUCK TERMINAL
Emeryville, California

PROJECT NO	DATE	BORING NO
K405-35	August 1986	6

DRILL RIG Hollow Stem Auger	SURFACE ELEVATION --	LOGGED BY DYR
DEPTH TO GROUNDWATER 10½' (see note 2)	BORING DIAMETER 8 Inches	DATE DRILLED 8/11/86

DESCRIPTION AND REMARKS	COLOR	CONSIST.	SOIL TYPE	DEPTH (FEET)	SAMPLER	PENETRATION RESISTANCE (BLOWS/FT)	WATER CONTENT (%)	DRY DENSITY (PCF)	UNCONFINED COMPRESSIVE STRENGTH (KSF)
3" AC				0					
SAND, some silt and clay, with gravels	tan		SM-SC	1					
				2					
SAND, clayey	grey black		SC	3	X				
				4	X				
				5	X				
				6	X				
				7	X				
				8	X				
CLAY, silty, with sand (FILL)	grey black		CL	9					
				10			▽		
				11					
				12					
Bottom of Boring = 12 Feet				13					
				14					
				15					
				16					
				17					
				18					
				19					
				20					

Notes:
1. The stratification line represents the approximate boundary between soil types and the transition may be gradual.
2. Groundwater level was measured at time of drilling.

PETER KALDVEER AND ASSOCIATES, INC.
Geotechnical Consultants

EXPLORATORY BORING LOG

SOIL AND GROUNDWATER TESTING
PIE TRUCK TERMINAL
Emeryville, California

PROJECT NO	DATE	BORING NO
K405-35	August 1986	7

APPENDIX B

LABORATORY TEST RESULTS

BROWN AND CALDWELL

ANALYTICAL LABORATORIES

LOG NO: E86-08-225

Received: 11 AUG 86
Reported: 15 AUG 86

Ms. Dawn Rinaldi
Peter Kaldveer and Associates
425 Roland Way
Oakland, CA 94621

Project: K405-35

REPORT OF ANALYTICAL RESULTS

LOG NO	SAMPLE DESCRIPTION , SOIL SAMPLES	DATE SAMPLED
08-225-1	EB-1-6'	
08-225-2	EB-1-7.5'	
08-225-3	EB-1-9'	
08-225-4	EB-2-8'	
08-225-5	EB-2-12'	

PARAMETER	08-225-1	08-225-2	08-225-3	08-225-4	08-225-5
Benzene,Toluene,Xylene Isomers					
Benzene, mg/kg	2.3	3.3	5.2	15	24
Toluene, mg/kg	<0.5	18	20	11	5.0
Total Xylene Isomers, mg/kg	1.2	6.2	<1.0	14	14
Total Fuel Hydrocarbons, mg/kg	160	310	290	1200	11000

BROWN AND CALDWELL
ANALYTICAL LABORATORIES

LOG NO: E86-08-225

Received: 11 AUG 86
Reported: 15 AUG 86

Ms. Dawn Rinaldi
Peter Kaldveer and Associates
425 Roland Way
Oakland, CA 94621

Project: K405-35

REPORT OF ANALYTICAL RESULTS

LOG NO	SAMPLE DESCRIPTION , SOIL SAMPLES	DATE SAMPLED
08-225-6	EB-3-8.5'	
08-225-7	EB-3-10'	
08-225-8	EB-3-12'	
08-225-9	EB-4-5.5'	
08-225-10	EB-4-7.5'	

PARAMETER	08-225-6	08-225-7	08-225-8	08-225-9	08-225-10
Benzene,Toluene,Xylene Isomers					
Benzene, mg/kg	140	2.0	<0.5	18	<0.5
Toluene, mg/kg	97	0.8	<0.5	7.5	<0.5
Total Xylene Isomers, mg/kg	99	1.9	1.3	41	<1.0
Total Fuel Hydrocarbons, mg/kg	25000	1900	1700	4600	1.8

BROWN AND CALDWELL
ANALYTICAL LABORATORIES

LOG NO: E86-08-225

Received: 11 AUG 86
Reported: 15 AUG 86

Ms. Dawn Rinaldi
Peter Kaldveer and Associates
425 Roland Way
Oakland, CA 94621

Project: K405-35

REPORT OF ANALYTICAL RESULTS

LOG NO	SAMPLE DESCRIPTION , SOIL SAMPLES	DATE SAMPLED
08-225-11	EB-4-8'	
08-225-12	EB-4-9'	
08-225-13	EB-5-4'	
08-225-14	EB-5-5.5'	
08-225-15	EB-5-7'	

PARAMETER	08-225-11	08-225-12	08-225-13	08-225-14	08-225-15
Benzene,Toluene,Xylene Isomers					
Benzene, mg/kg	47	0.5	14	34	65
Toluene, mg/kg	26	0.7	7.2	9.3	15
Total Xylene Isomers, mg/kg	16	2.2	17	8.5	19
Total Fuel Hydrocarbons, mg/kg	8800	930	7200	8600	17000

BROWN AND CALDWELL
ANALYTICAL LABORATORIES

LOG NO: E86-08-225

Received: 11 AUG 86
Reported: 15 AUG 86

Ms. Dawn Rinaldi
Peter Kaldveer and Associates
425 Roland Way
Oakland, CA 94621

Project: K405-35

REPORT OF ANALYTICAL RESULTS

LOG NO	SAMPLE DESCRIPTION , SOIL SAMPLES	DATE SAMPLED
08-225-16	EB-6-4'	
08-225-17	EB-6-6'	
08-225-18	EB-7-4'	
08-225-19	EB-7-6'	
08-225-20	EB-7-8'	

PARAMETER	08-225-16	08-225-17	08-225-18	08-225-19	08-225-20
Benzene,Toluene,Xylene Isomers					
Benzene, mg/kg	1.6	1.7	<0.5	25	<0.5
Toluene, mg/kg	0.6	0.6	<0.5	11	<0.5
Total Xylene Isomers, mg/kg	<1.0	<1.0	<1.0	14	<1.0
Total Fuel Hydrocarbons, mg/kg	230	250	3.8	2500	210

BROWN AND CALDWELL
ANALYTICAL LABORATORIES

LOG NO: E86-08-225

Received: 11 AUG 86
Reported: 15 AUG 86

Ms. Dawn Rinaldi
Peter Kaldveer and Associates
425 Roland Way
Oakland, CA 94621

Project: K405-35

REPORT OF ANALYTICAL RESULTS

LOG NO	SAMPLE DESCRIPTION , GROUND WATER SAMPLES	DATE SAMPLED
08-225-21	EB-1	
08-225-22	EB-2	
08-225-23	EB-3	
08-225-24	EB-4	
08-225-25	EB-5	

PARAMETER	08-225-21	08-225-22	08-225-23	08-225-24	08-225-25
Benzene,Toluene,Xylene Isomers					
Benzene, mg/L	2.0	3.1	0.2	2.4	0.6
Toluene, mg/L	1.3	0.4	<0.1	0.1	<0.1
Total Xylene Isomers, mg/L	<0.2	<0.2	<0.2	0.4	<0.2
Total Fuel Hydrocarbons, mg/L	8.3	6.9	4.6	7.8	41